

BOARD OF APPEALS

April 15, 2026

County Administration Building, 100 W. Washington St., Meeting Room 2000, Hagerstown, at 6:00 p.m.

AGENDA

AP2026-010: An appeal was filed by Arcola Towers for a special exception to establish a commercial communication tower on property owned by James & Elanie Miller and located at 10944 White Hall Road, Smithsburg, Zoned Agricultural Rural District.

Annual Report for 2025

Annual Election for Chair and Vice-Chair for the Board of Appeals

Pursuant to the Maryland Open Meetings Law, notice is hereby given that the deliberations of the Board of Zoning Appeals are open to the public. Furthermore, the Board, at its discretion, may render a decision as to some or all of the cases at the hearing described above or at a subsequent hearing, the date and time of which will be announced prior to the conclusion of the public hearing. Individuals requiring special accommodations are requested to contact Katie Rathvon at 240-313-2464 Voice, 240-313-2130 Voice/TDD no later than April 6, 2026. Any person desiring a stenographic transcript shall be responsible for supplying a competent stenographer.

The Board of Appeals reserves the right to vary the order in which the cases are called. Please take note of the Amended Rules of Procedure (Adopted July 5, 2006), Public Hearing, Section 4(d) which states:

Applicants shall have ten (10) minutes in which to present their request and may, upon request to and permission of the Board, receive an additional twenty (20) minutes for their presentation. Following the Applicant's case in chief, other individuals may receive three (3) minutes to testify, except in the circumstance where an individual is representing a group, in which case said individual shall be given eight (8) minutes to testify.

Those Applicants requesting the additional twenty (20) minutes shall have their case automatically moved to the end of the docket.

For extraordinary cause, the Board may extend any time period set forth herein, or otherwise modify or suspend these Rules, to uphold the spirit of the Ordinance and to do substantial justice.

Tracie Felker, Chairman

Board of Zoning Appeals



WASHINGTON COUNTY BOARD OF ZONING APPEALS

747 Northern Avenue | Hagerstown, MD 21742-2723 | P:240.313.2430 | F:240.313.2431 | Hearing Impaired: 7-1-1

ZONING APPEAL

Property Owner: James & Elanie Miller
12290 Scott Road
Waynesboro PA 17268
Appellant: Arcola Towers
112 West Washington Street
Suite 201
Middleburg VA 20117
Property Location: 10944 White Hall Road
Smithsburg, MD 21783
Description Of Appeal: Special exception to establish a commercial communication tower
Docket No: AP2026-010
Tax ID No: 18013932
Zoning: A(R)
RB Overlay: No
Zoning Overlay:
Filed Date: 03/09/2026
Hearing Date: 04/15/2026

Appellant's Legal Interest In Above Property: Owner: No Contract to Rent/Lease: No
Lessee: Yes Contract to Purchase: No
Other:

Previous Petition/Appeal Docket No(s):

Applicable Ordinance Sections: Washington County Zoning Ordinance Section 5A.2 Special Exception, Section 3.3 (1) Table of Land Uses, R. Utilities

Reason For Hardship:

If Appeal of Ruling, Date Of Ruling:

Ruling Official/Agency:

Existing Use: Residential/Agricultural Proposed Use: Commercial Communication Tower

Previous Use Ceased For At Least 6 Months: Date Ceased:

Area Devoted To Non-Conforming Use - Existing: Proposed:

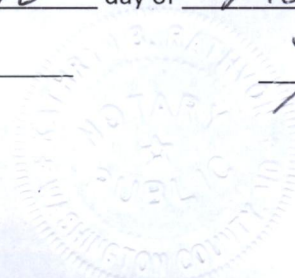
I hereby affirm that all of the statements and information contained in or filed with this appeal are true and correct.

[Signature]
Appellant Signature

State Of Maryland, Washington County to-wit:

Sworn and subscribed before me this 10 day of March, 2026.

Nov 7, 2029
My Commission Expires [Signature] Notary Public





WASHINGTON COUNTY BOARD OF ZONING APPEALS

747 Northern Avenue | Hagerstown, MD 21742-2723 | P:240.313.2430 | F:240.313.2431 | Hearing Impaired: 7-1-1

AFFIDAVIT IN COMPLIANCE WITH SECTION 25.51(C)

Docket No: AP2026-010

State of Maryland Washington County, To Wit:

On 3/9/2026, before me the subscriber, a Notary of the public of the State and County aforesaid, personally appeared Arcola Towers and made oath in due form of law as follows:

Arcola Towers will post the zoning notice sign(s) given to me by the Zoning Administrator in accordance with Section 25.51(c) of the Washington County Zoning Ordinance for the above captioned Board of Appeals case, scheduled for public hearing on 04/15/2026, and that said sign(s) will be erected on the subject property in accordance with the required distances and positioning as set out in the attached posting instructions.

Sign(s) will be posted on 03/31/2026 and will remain until after the above hearing date.

Arcola Towers Agent

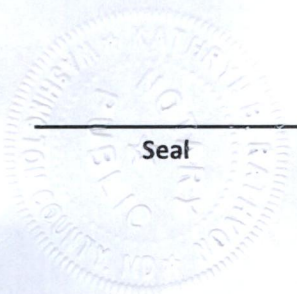
Sworn and subscribed before me the day and year first above written.

Notary Public

Nov. 7, 2029

My Commission Expires

Seal





BOARD OF ZONING APPEALS

ATTENTION!

Posting Instructions

The premises MUST be posted in accordance with the following rules:

1. The sign must be posted a minimum of fourteen (14) days prior to the public hearing. Section 25.51(c) Property upon which the application or appeal is concerned shall be posted conspicuously by a zoning notice no less in size than twenty-two (22) inches by twenty-eight (28) inches at least fourteen (14) days before the date of the hearing.
2. The sign must be placed on the property within ten (10) feet of the property line which abuts the most traveled public road.
3. The sign must be posted in a conspicuous manner not over six (6) feet above the ground level, and affixed to a sturdy frame where it will be clearly visible and legible to the public.
4. The sign shall be maintained at all times by the applicant until after the public hearing. If a new sign is needed or required, please contact the Plan Review Department at 240-313-2460.
5. An affidavit certifying the property will be posted for the minimum of fourteen (14) days prior to the public hearing date.

Proper posting of the sign will be spot checked by the Zoning Inspector. IF SIGN IS NOT IN COMPLIANCE, IT MAY RESULT IN RESCHEDULING OF THE HEARING.





BOARD OF ZONING APPEALS

80 West Baltimore Street | Hagerstown, MD 21740 | P: 240.313.2460 | F: 240.313.2461 | Hearing Impaired: 7-1-1

WWW.WASHCO-MD.NET

Appeal for Special Exception

Appeal is hereby made for a special exception under the Washington County Zoning Ordinance as follows:

Location 10944 White Hall Road, Smithsburg, MD 21783

Appellant's present legal interest in above property: (Check One)

Owner (Including Joint Ownership) Lessee Contract to rent/lease
 Contract to Purchase Other _____

Use Proposed: 187' monopole-style wireless telecommunications facility with a 2 ft. lightning rod for Verizon Wireless.

Zoning Ordinance section and subsection(s) providing for proposed use: Section 4.22 Commercial

Communications Towers

If filing functionally similar to a principal permitted use or special exception use, please list the use and describe the use similarities:

Provide Detailed Explanation on Separate Sheet

Has any previous petition or appeal involving this property been made to the Board?

Yes No

If yes, give docket number(s): _____

Additional comments, if any: Please see attached narrative.

I hereby certify that I have, to the best of my knowledge, accurately supplied the information required for the above referenced appeal.

Jonathan L Yates
Signature of Appellant

105 Broad St. Third Floor, Charleston, SC 29401
Address of Appellant

jly@hellmanyates.com
Email of Appellant

843-414-9754
Phone Number of Appellant

This appeal form is to be used to assist the customer in gathering the information necessary to submit an application. However, the application shall be processed in person.

HELLMAN YATES

JONATHAN L. YATES
DIRECT VOICE 843 414-9754
JLY@HELLMANYATES.COM

HELLMAN & YATES, PA
105 BROAD STREET, THIRD FLOOR
CHARLESTON, SOUTH CAROLINA 29401
V 843 266-9099
F 843 266-9188

March 4, 2026

VIA HAND DELIVERY

Katie Rathvon
Zoning Coordinator
80 West Baltimore Street
Hagerstown, MD 21740

Re: Proposed 187-foot monopole-style communications facility with a 2 ft. lightning rod (for a total height of 189 ft.) located at 10944 White Hall Road, Smithsburg, MD 21783 by Arcola Towers for Verizon Wireless.

Dear Ms. Rathvon,

Enclosed, please find the application of Arcola Towers for a proposed 187-foot monopole-style communications facility with a 2 ft. lightning rod for a total height of 189 feet. The proposed facility is to be located at 10944 White Hall Road, Smithsburg, MD 21783 (Tax Account #18-013932) on the property of James H. and Elaine K. Miller by Arcola Towers for Verizon Wireless. In support of this application, we have taken the liberty of recasting the relevant sections of Washington County Zoning Ordinance, with our answer to the relevant section in bold beneath. As will be evident from a review of the attached, Arcola Towers has not only met, but has exceeded, all the necessary requirements for approval under the Washington County Zoning Ordinance.

Section 4.22 Commercial Communication Towers

The purpose of this section is to regulate the placement, construction, and modification of commercial communications towers as defined in Article 28A (hereinafter "towers") and commercial communications equipment as defined in Article 28A (hereinafter "equipment"). It is the intent of these regulations to minimize the visual impact of towers and equipment, to minimize the number of towers through shared use and co-location, to encourage utilization of technological designs that will either eliminate or reduce the need for new towers to support equipment and to ensure that all towers and equipment are compatible with surrounding land uses while assuring wireless communications service to the citizens of Washington County.

Equipment proposed to be located on an existing tower or antenna support structure as defined in Article 28A shall be allowed in any district provided that the height from grade of the equipment shall not exceed the height from grade of the antenna support structure by more than twenty (20) feet.

The Applicant accepts and acknowledges this provision. This is an application for a new tower. Please see the Site Plan and Drawings by Maryland Professional Engineer Camille Shabshab attached hereto as Exhibit "1" and incorporated herein by reference.

No permit to construct a tower may be issued unless the applicant demonstrates to the Planning Commission, or where applicable, to the Board of Zoning Appeals, need for the tower and that the applicant has exhausted all alternatives to constructing a tower. Applicants are required to prove need by:

- a. demonstrating via statement or other evidence that, in terms of location and construction, there are no existing towers, buildings, elevated tanks or other structures able to provide the antenna platform required.

The Applicant accepts and acknowledges this provision. Arcola Towers was able to confirm that there are no existing towers, buildings, elevated tanks, or other structures able to provide the antenna platform required. Please see the Alternative Candidate Analysis by Ryan Foltz attached hereto as Exhibit "2" and incorporated herein by reference. The closest tower is a 195' self-support American Tower site that is 1.29 miles away to the southeast and too far to cover Verizon's intended coverage area.

- b. providing evidence, including coverage diagrams and technical reports, demonstrating that co-location on existing sites is not technically possible in order to serve the desired need.

Please see the Alternative Candidate Analysis by Ryan Foltz in Exhibit "2".

A. Design requirements

In addition to the applicable requirements for a site plan as specified in Section 4.11, the applicant shall provide the following information as part of the site plan submittal. These provisions shall apply to towers in all districts where permitted as a principal permitted or special exception use:

1. Subject to a minimum setback of a distance equaling the total height of the tower and equipment. The setback shall be measured from the base of the tower to the boundary line of the property owned, leased, or controlled by easement by the applicant.

The proposed monopole-style communications facility will meet the setback requirements of tower height. As shown on sheet Z-1 of Exhibit "1", the proposed setbacks for the 187 ft. tower are: 590.1 feet to the front, 2150.4 feet to the rear, 281.2 feet to the southwest, and 2181.2 feet to the northeast.

2. Subject to a minimum distance requirement of a distance equaling the height of the tower and equipment plus 200 feet from the RT, RS, RU, RM and R V districts or the nearest part of any existing dwelling, school, church, or institution for human care, in any other district.

As shown in Note 7 on Sheet Z-1 of Exhibit "1", the proposed facility will meet the requirement of a 389 ft. distance from these districts, existing dwellings, schools, churches, or institutions for human care. The closest RT zoned property is 469.4 ft. to the west.

3. Subject to a minimum setback from all overhead transmission lines of a distance equaling two times the height of the tower and equipment.

As shown in Note 9 on sheet Z-1 of Exhibit "1", there are no overhead transmission lines within two times the height of the tower.

4. Subject to a height not to exceed 200 feet. Measurement of tower height shall include the tower structure itself, the base pad, and any other equipment attached thereto which extends more than twenty (20) feet over the top of the tower structure itself. The tower height shall be measured from grade.

The proposed monopole-style communications facility will not exceed 200 feet. As shown on Sheet Z-4 of Exhibit "1" and the Design Drawings by Maryland Professional Engineer Robert E. Beacom attached hereto as Exhibit "3" and incorporated herein by reference, the proposed monopole has an elevation of 187 feet with a two-foot lightning rod for a total height of 189 feet.

5. Proposed towers shall meet the following separation requirements from existing towers or towers which have been issued a permit but are not yet constructed.
 - a. Monopole towers shall be separated from all other towers, whether monopole, self-supporting lattice, or guyed, by a minimum of seven hundred and fifty (750) feet.

Please see the Alternative Candidate Analysis in Exhibit "2". There are no existing structures within a 750 ft. radius of the proposed facility. The closest facility is a 195 ft. American Towers self-support tower approximately 1.29 miles away to the southeast and will not cover the intended area for Verizon.

- b. Self-supporting lattice or guyed towers shall be separated from all other self-supporting or guyed towers by a minimum of fifteen hundred (1,500) feet.

The provision does not apply to this application as this is for a monopole-style communications facility.

- c. Self-supporting lattice or guyed towers shall be separated from all monopole towers by a minimum of seven hundred and fifty (750) feet.

The provision does not apply to this application as this is for a monopole-style communications facility.

6. All towers shall be designed for co-location, which shall mean the ability of the structure to allow for the placement of comparable equipment for other carriers. An application for a tower shall be accompanied by an affidavit from the applicant stating that one ten (10) foot space on the proposed tower will be specifically reserved for use by the County, and that other spaces will be made available to other future users, when possible.

Please see the Affidavit from Christian Winkler of Arcola Towers attached hereto as Exhibit "4" and incorporated herein by reference. One ten (10) foot space on the proposed tower will be specifically reserved for use by Washington County, and other spaces will be made available to other future users, when possible. The proposed monopole-style facility has been designed for Verizon and collocation by at least three additional broadband carriers as shown in the Design Drawings in Exhibit "3". In addition, please see the ANSI/ Fall Zone letter by Maryland Professional Engineer Robert E. Beacom attached hereto as Exhibit "5" and incorporated herein by reference. Beacom certifies the fall zone at 187' and thus any collapse would be contained on the Miller property.

7. Fencing shall be provided around the base of the tower and any associated equipment buildings.

Please see Sheets Z-4 and Z-6 of Exhibit "1" for the fencing plan. The 50ft-by-50ft compound will be secured by an 8' chain-link fence topped with three strands of barbed wire for a total height of 9 feet.

8. All sites shall be identified by means of a sign no longer than two square feet affixed to the fence identifying the entity using the site and shall provide the telephone number of a contact person in the event of an emergency.

The Applicant accepts and acknowledges this provision. Please see Sheet Z-7 in Exhibit "1". Only the FCC required site identification and emergency signage will be deployed.

9. Towers not requiring FAA painting or marking shall have an exterior finish which enhances compatibility with adjacent land uses, as approved by the Planning Commission or Board of Zoning Appeals. Towers shall not be lighted unless specifically required by the FAA.

The Applicant accepts and acknowledges this provision. Please see the FAA Determination of No Hazard to Air Navigation and the Ken Patterson Airspace Report attached hereto as Exhibit "6" and incorporated herein by reference. Due to its de minimis height, the FAA will not require illumination.

10. In order to protect the natural skyline, towers should be sited within areas of mature vegetation and should be located down slope from ridge lines, and toward the interior of the parcel whenever possible. Placement should only be considered elsewhere on the property when valid technical dates supplied by the applicant indicates that there is no other suitable location.

The Applicant accepts and acknowledges this provision. Please see the Balloon Test and Photo Simulations attached hereto as Exhibit "7" and incorporated herein by reference.

11. Towers proposed to be located within the Appalachian Trail corridor special planning area as identified in the adopted Comprehensive Plan for the County, any "AO" Antietam Overlay zoning district or "HP" Historic Preservation zoning district shall utilize stealth technology as defined in Article 28A to minimize visual impact.

The Applicant accepts and acknowledges this provision. As shown in Note 6 on sheet Z-1 of Exhibit "1", the proposed facility is not to be located within the Appalachian Trail Corridor Special Planning Area as identified in the adopted Comprehensive Plan for the County, any "AO" Antietam Overlay zoning district or "HP" Historic Preservation zoning district.

12. (a) A Commercial Communication Tower that is out of service for a continuous six (6) month period will be deemed to have been abandoned. The Zoning Administrator may issue a Notice of Abandonment to the Owner of the Tower that is deemed to be abandoned. The Owner shall have the right to respond in writing to the Notice of Abandonment setting forth the reasons for operation difficulty and providing a reasonable timeframe for correction action, within thirty (30) days from the date of the Notice. The Administrator shall withdraw the Notice of Abandonment and notify the Owner that the Notice has been withdrawn if the Owner provides information that demonstrates the Tower has not been abandoned.

The Applicant accepts and acknowledges this provision. Please see the Tower Removal Letter by Ryan Foltz of Arcola Towers attached hereto as Exhibit "8" and incorporated herein by reference.

(b) If the Tower is determined to be abandoned, the Owner of the Tower shall remove the Tower, and all related equipment at the Owner's sole expensed within three (3) months of the Date of Notice of Abandonment. If the Owner fails to remove the Tower and related equipment, the Administrator may pursue legal action to have the Tower removed at the Owner's expense.

The Applicant accepts and acknowledges this provision. Please see the Tower Removal Letter in Exhibit "8".

B. Additional Provisions for Towers Permitted by Special Exception

In addition to the limitations, guides and standards enumerated in Section 25.6, the Board of Zoning Appeals shall consider the following provisions when considering a request for a special exception for a commercial communications tower.

1. In those cases where a proposed tower is part of a grid or network, the applicant shall provide a map indicating the location of any existing or proposed towers in the grid or network within Washington County and within one (1) mile of the County boundary.

Please see the Inventory Letter by Ryan Foltz of Arcola Towers and the RF Justification Letter by Shawn Boykins of Verizon Wireless and proposed coverage studies attached hereto as Exhibit "9" and incorporated herein by reference.

2. The tower shall be compatible with and shall not adversely impact the character and integrity of surrounding properties. Consideration shall be given to the view shed associated with scenic and historic areas and to the use of stealth technology to minimize the visibility of the proposed tower.

Due to its strategic placement on the Miller property, the proposed facility shall not adversely impact the character and integrity of surrounding properties.

3. The applicant shall submit a visual analysis which may include, photo simulation, field mock-up, elevations or other visual or graphic illustrations to determine visual impact. Consideration shall be given to views from public areas as well as from private residences. The analysis shall assess the cumulative impacts of the proposed facility and other existing and foreseeable towers in the area and shall identify and include all feasible mitigation measures.

Please see the Balloon Test and Photo Simulations in Exhibit "7".

4. The Board may include conditions on the site where the tower is to be located if such conditions are necessary to preserve the character and integrity of the area affected by the proposed tower and mitigate any adverse impacts which arise in connection with approval of the special exception.

The Applicant accepts and acknowledges this provision.

In addition, please find the following: Survey by Maryland Professional Land Surveyor Fitzroy Jerry Bertrand attached hereto as Exhibit "10" and incorporated by reference; the Recorded Deed attached hereto as Exhibit "11" and incorporated herein by reference; and the Owners Representative Affidavit attached hereto as Exhibit "12" and incorporated herein by reference.

We would respectfully request to be placed on the agenda of the Washington County Board of Appeals April 15, 2026, scheduled meeting.

If you have any questions or concerns, please do not hesitate to contact me at 843-414-9754, or via email at jly@hellmanyates.com.

Thank you for all your help with this.

With warmest regards, I am

Yours very truly,



Jonathan L. Yates

Exhibit “1”



ARCOLA TOWERS
MD-003 WHITEHALL
10944 WHITE HALL ROAD
SMITHSBURG, MD 21783

187' MONOPOLE

entrex
 communication services, inc.
 6100 EXECUTIVE BLVD.
 SUITE 430
 ROCKVILLE, MD 20852
 PHONE: (202) 408-0960

arcola
 TOWERS
 112 N. WASHINGTON ST
 SUITE 201
 MIDDLEBURG, VA 20117
 PHONE: (571) 895 3990

SITE INFORMATION

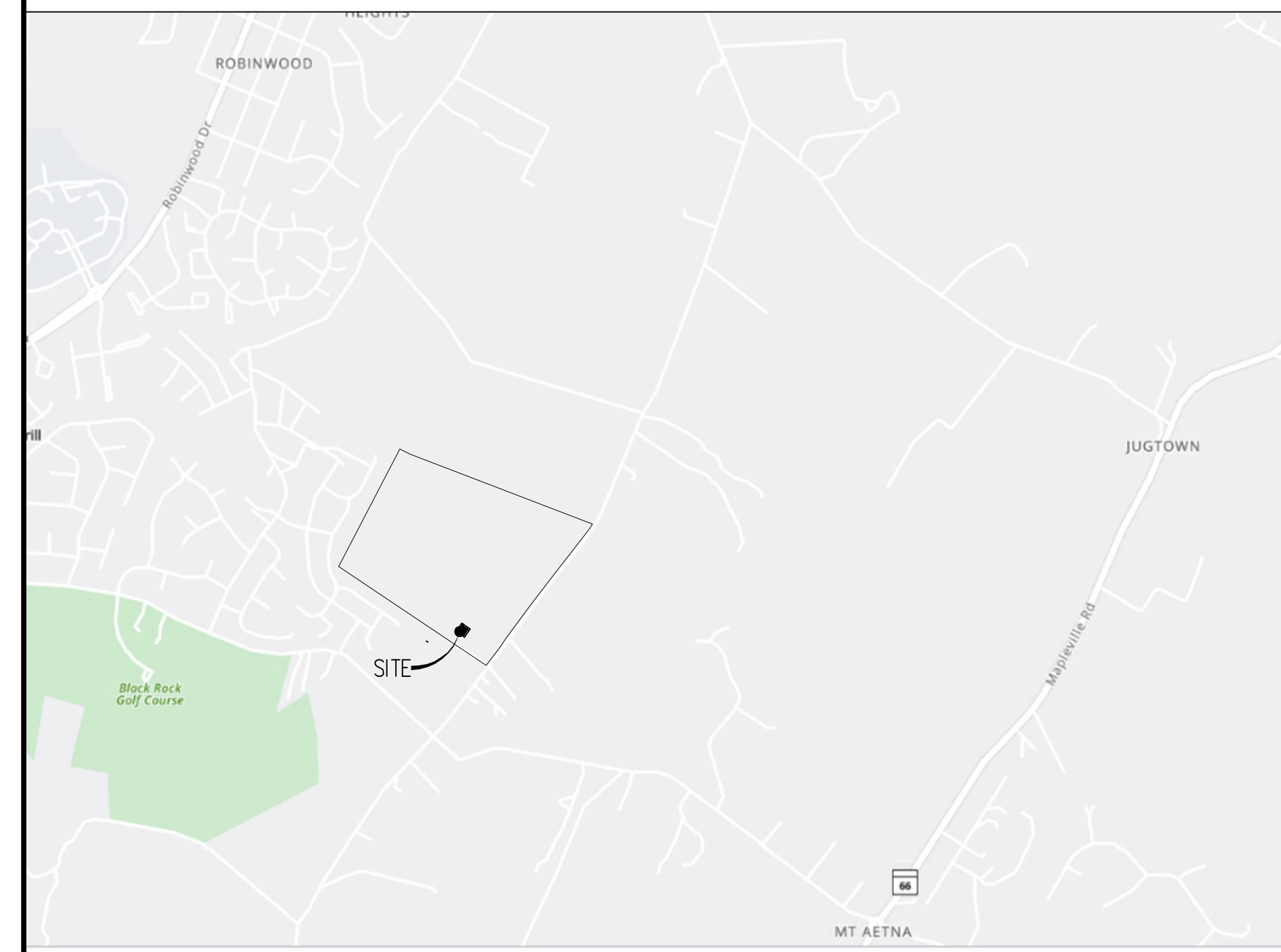
SCOPE OF WORK:

1. INSTALL EROSION AND SEDIMENT CONTROLS MEASURES
2. CLEAR AND GRADE SITE TO FINAL SUBGRADE ELEVATION
3. INSTALL MONOPOLE FOUNDATION AND MONOPOLE
4. INSTALL GROUNDING, TELCO HANDHOLES, UTILITY WIREWAY
5. INSTALL ELECTRICAL AND TELEPHONE CONDUITS AND HANDHOLES
6. INSTALL FENCE AND SITE IMPROVEMENTS

PROPERTY INFORMATION:

JURISDICTION: WASHINGTON COUNTY, MD
 ZONING: A(R)
 TAX ACCOUNT NUMBER: 18-013932
 PARCEL AREA: 154.0000 ACRES
 PARCEL OWNER: MILLER, JAMES H & ELAINE K
 OWNER ADDRESS: 12290 SCOTT RD, WAYNESBORO, PA 17268
 STRUCTURE TYPE: 187' MONOPOLE + 2' LIGHTNING ROD
 GROUND ELEVATION: 605.0' NAVD88
 LATITUDE: N 39° 36' 51.686" (N 39.614357°)
 LONGITUDE: W 77° 38' 45.768" (W 77.646047°)

VICINITY MAP



SCALE: 1" = 2,000'

MAGNETIC NORTH 10.11° W TRUE NORTH

SHEET INDEX

- T-1 TITLE SHEET
- Z-1 SITE PLAN
- Z-2 ADJOINERS
- Z-3 PARTIAL SITE PLAN
- Z-4 COMPOUND PLAN & MONOPOLE ELEVATION
- Z-5 EROSION AND SEDIMENT CONTROL PLAN
- Z-6 SITE DETAILS
- Z-7 SIGNAGE AND GATE DETAILS

ARCOLA TOWERS
WHITEHALL
10944 WHITE HALL RD
SMITHSBURG, MD 21783



SUBMITTALS

DATE	DESCRIPTION	REV.
09-09-25	ZONING REVIEW	A
09-24-25	ZONING REVIEW	B
10-08-25	ZONING REVIEW	C
10-14-25	ZONING	1

PROJECT TEAM

APPLICANT:

ARCOLA TOWERS
 112 W WASHINGTON STREET, SUITE 201
 MIDDLEBURG, VA 20117
 (571) 895 3990

ARCHITECT/ENGINEER:

ENTREX COMMUNICATION SERVICES, INC.
 6100 EXECUTIVE BLVD, SUITE 430
 ROCKVILLE, MD 20852
 CAMILLE SHABSHAB (202) 408-0960

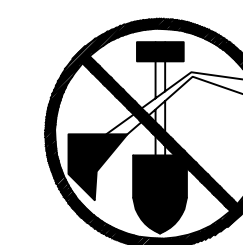
CODE COMPLIANCE

ALL WORK AND MATERIALS SHALL BE PERFORMED AND INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THE LATEST EDITIONS OF THE FOLLOWING CODES.

- 2021 INTERNATIONAL BUILDING CODE
- 2021 INTERNATIONAL EXISTING BUILDING CODE
- 2023 NATIONAL ELECTRICAL CODE
- 2018 NFPA 101 LIFE SAFETY CODE
- AMERICAN CONCRETE INSTITUTE
- AMERICAN INSTITUTE OF STEEL CONSTRUCTION
- ANSI/TIA-222-H

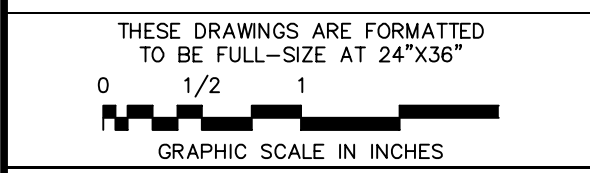
DRAWING APPROVALS

	SIGNATURE	DATE
OWNER REPRESENTATIVE		
SITE ACQUISITION		
CONSTRUCTION MANAGER		
ZONING		
RF ENGINEER		



CALL
 UTILITIES NOTIFICATION
 MISS UTILITY
 1-800-257-7777
 3 WORKING DAYS PRIOR TO DIGGING

PROJECT NO: 1164.015
 DESIGNER: R.S.
 ENGINEER: C.S.

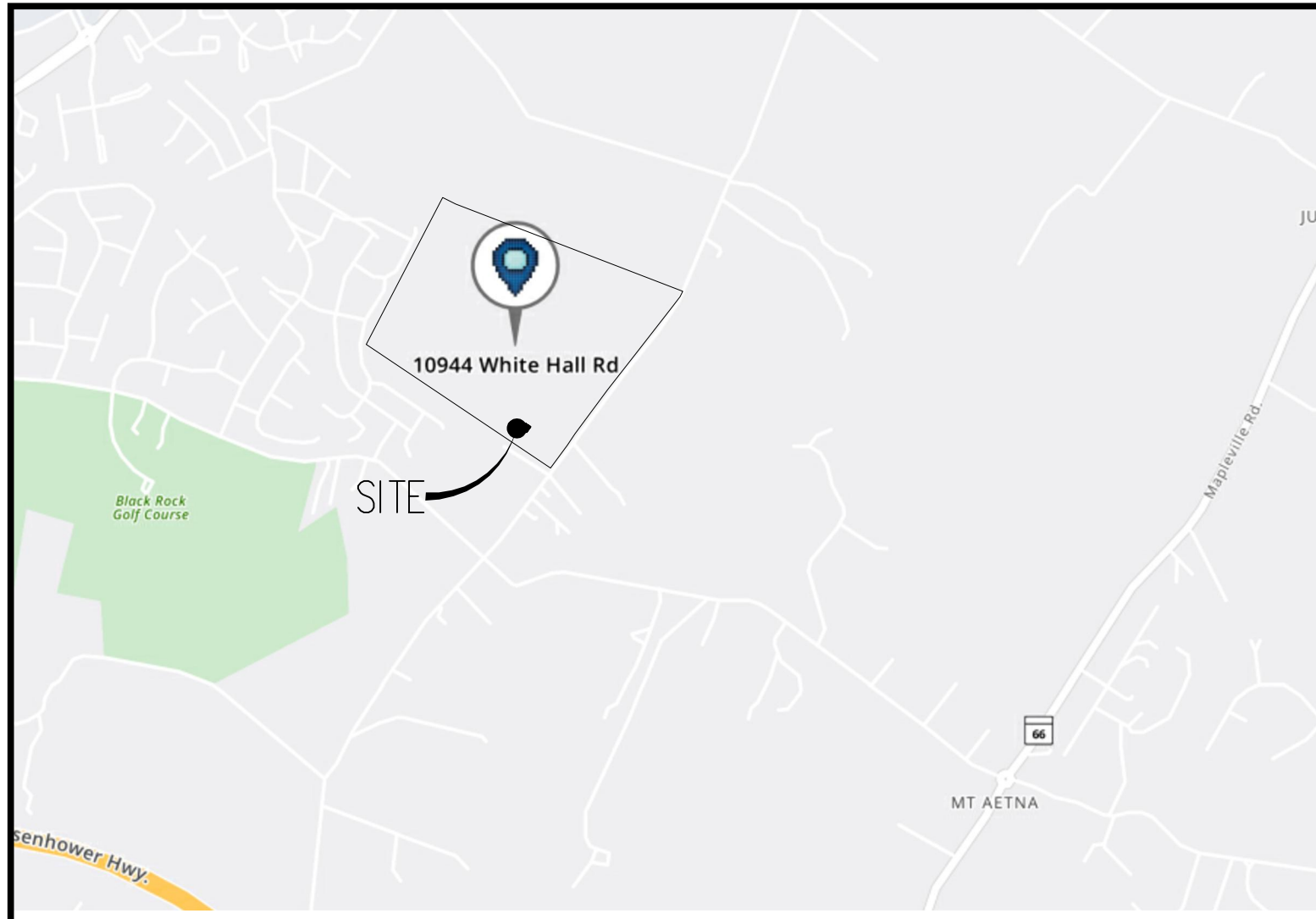


SHEET TITLE:

TITLE SHEET

SHEET NUMBER:

T-1



VICINITY MAP
SCALE: 1" = 2,000'-0"



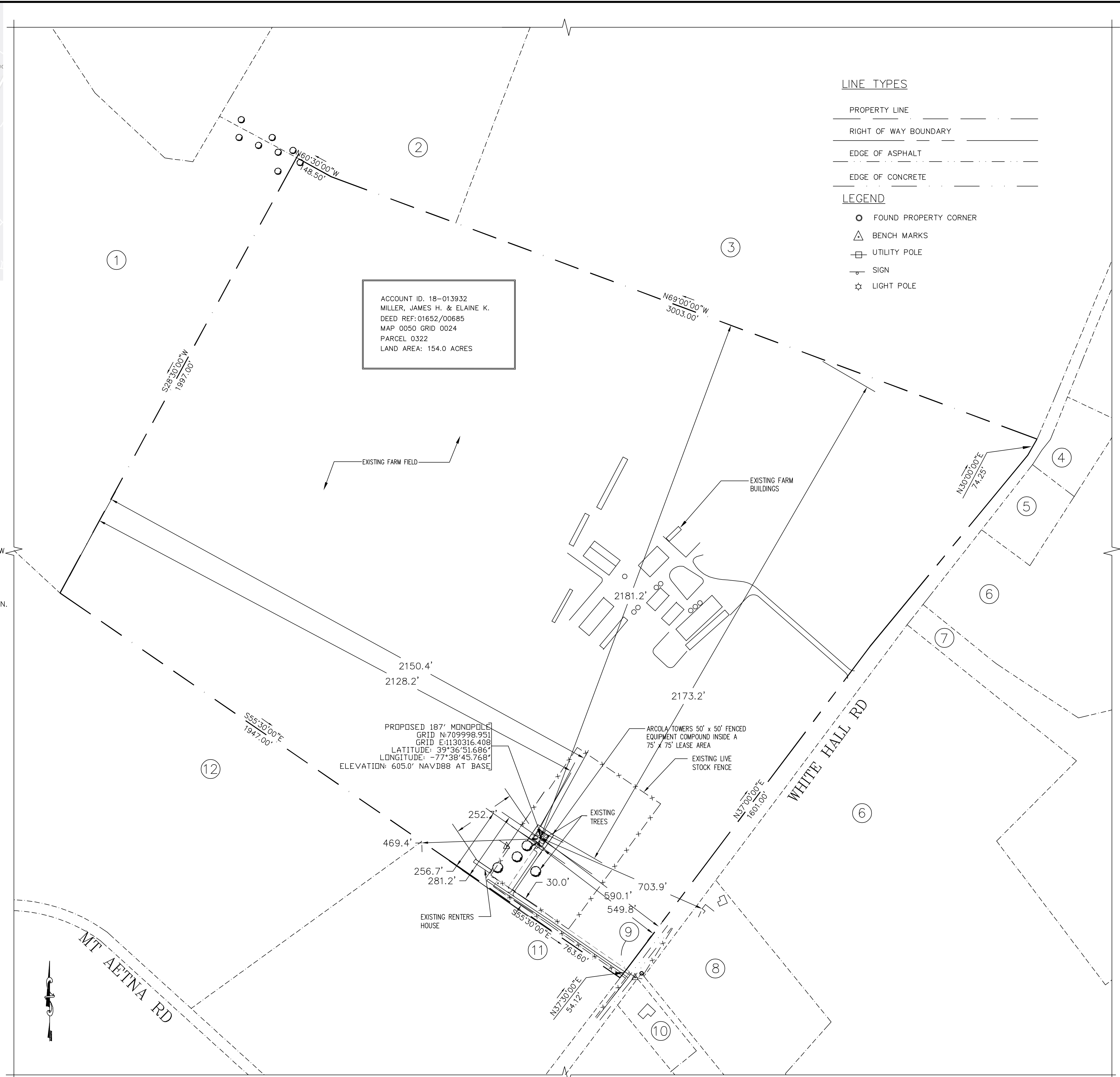
ZONING DRAWING NOTES

- SITE NAME: ARCOLA TOWERS SITE - WHITEHALL
- THIS IS NOT A BOUNDARY SURVEY AND IS NOT TO BE USED FOR THE TRANSFER OF PROPERTY.
- THE SUBJECT PARCEL INFORMATION;
OWNER: MILLER, JAMES H. & ELAINE K.
PREMISES ADDRESS: 10944 WHITE HALL ROAD
SMITHSBURG, MD 21783
MAILING ADDRESS: 12290 SCOTT ROAD
WAYNESBORO, PA 17268

JURISDICTION: WASHINGTON COUNTY
ACCOUNT NO.: 18-013932
PARCEL: 0322, DEED REF: 01652/00685
MAP 0050 GRID 0024
AREA: 154 ACRES
- THE LOCATION OF THE PROPOSED MONOPOLE IS AS FOLLOWS; THE VALUES LISTED BELOW ARE WITHIN ±50' HORIZONTAL AND ±20' VERTICAL.
LATITUDE: ± N 39° 36' 51.686"
LONGITUDE: ± W 77° 38' 45.768"
ELEVATION: 605.0' NAVD88 AT MONOPOLE BASE
- THERE IS NO FLOODPLAIN ON THE PROPERTY THAT IS THE SUBJECT OF THIS APPLICATION. THE CURRENT FLOOD INSURANCE RATE MAP (FIRM) OF WASHINGTON COUNTY, MD, COMMUNITY PANEL NUMBER FOR THE PROPERTY THAT IS THE SUBJECT OF THIS APPLICATION IS 24043C0310D, EFFECTIVE AUGUST 15, 2017.
- THIS SITE IS NOT LOCATED WITHIN THE APPALACHIAN TRAIL CARTER SPECIAL PLANNING AREA, THE ANTIETAM OVERLAY ZONING DISTRICT OR THE HISTORIC PRESERVATION ZONING DISTRICT
- THE SITE IS LOCATED MORE THAN 389' FROM ANY EXISTING OFFSITE DWELLING, SCHOOL, CHURCH, OR INSTITUTION FOR HUMAN CARE.
- THE CLOSEST EXISTING TOWER IS A 205' AMERICAN TOWERS SELF-SUPPORT TOWER, LOCATED APPROXIMATELY 1.75 MI SOUTH.
- THERE ARE NO OVERHEAD TRANSMISSION LINES WITHIN TWO TIMES THE HEIGHT OF THE TOWER.
- NO WETLANDS HAVE BEEN DEFINED AND ANY AREAS SHOWN AS MARSH, PONDS OR DITCHES ARE DONE SO FROM VISIBLE SURFACE FEATURES AND IN NO WAY CONSTITUTE A DEFINED WETLAND.
- EXISTING HEAVY MATURE VEGETATION WILL BE UTILIZED IN LIEU OF NEW LANDSCAPING.

MONOPOLE SETBACKS		
	REQUIRED	PROPOSED
FRONT YARD (SOUTHEAST)	187'	590.1'
REAR YARD (NORTHWEST)	187'	2150.4'
SIDE YARD (SOUTHWEST)	187'	281.2'
SIDE YARD (NORTHEAST)	187'	2181.2'
CLOSEST OFF SITE RESIDENCE	187'	703.9'

COMPOUND SETBACKS		
	REQUIRED	PROPOSED
FRONT YARD (SOUTHEAST)	50'	549.8'
REAR YARD (NORTHWEST)	50'	2128.2'
SIDE YARD (SOUTHWEST)	50'	256.7'
SIDE YARD (NORTHEAST)	50'	2173.2'



ACCOUNT ID: 18-013932
MILLER, JAMES H. & ELAINE K.
DEED REF: 01652/00685
MAP 0050 GRID 0024
PARCEL 0322
LAND AREA: 154.0 ACRES

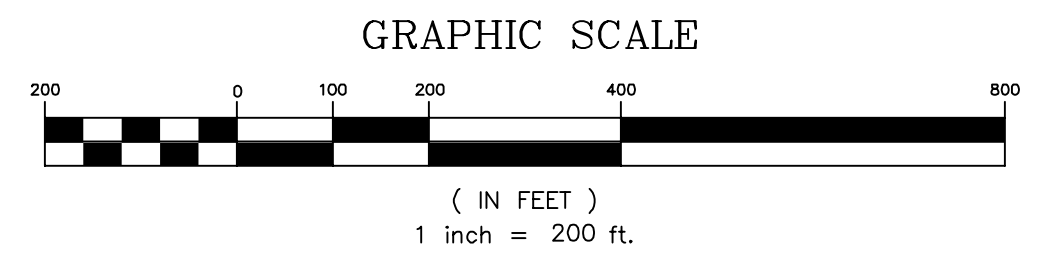
LINE TYPES

- PROPERTY LINE
- RIGHT OF WAY BOUNDARY
- EDGE OF ASPHALT
- EDGE OF CONCRETE

LEGEND

- FOUND PROPERTY CORNER
- △ BENCH MARKS
- UTILITY POLE
- SIGN
- ☆ LIGHT POLE

PROPOSED 187' MONOPOLE
GRID N:709998.951
GRID E:1130316.408
LATITUDE: 39°36'51.686"
LONGITUDE: -77°38'45.768"
ELEVATION: 605.0' NAVD88 AT BASE



SITE PLAN
SCALE: 1" = 200'

6100 EXECUTIVE BLVD.
SUITE 430
ROCKVILLE, MD 20852
PHONE: (202) 408-0960

112 N. WASHINGTON ST
SUITE 201
MIDDLEBURG, VA 20117
PHONE: (571) 895 3990

**ARCOLA TOWERS
WHITEHALL**
10944 WHITE HALL RD
SMITHSBURG, MD 21783

SEAL:
Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 17841, Expiration Date: 12/31/2025

SUBMITTALS

DATE	DESCRIPTION	REV.
09-09-25	ZONING REVIEW	A
09-24-25	ZONING REVIEW	B
10-08-25	ZONING REVIEW	C
10-14-25	ZONING	1

PROJECT NO: 1164.015
DESIGNER: R.S.
ENGINEER: C.S.
THESE DRAWINGS ARE FORMATTED TO BE FULL-SIZE AT 24"x36"
0 1/2 1
GRAPHIC SCALE IN INCHES
SHEET TITLE:
SITE PLAN
SHEET NUMBER:
Z-1

SUBJECT PARCEL
 ACCOUNT ID: 18-013932
 N/F
 MILLER, JAMES H. & ELAINE K.
 PARCEL ADDRESS: 10944 WHITE HALL RD
 SMITHSBURG MD 21738
 MAILING ADDRESS: 12290 SCOTT ROAD
 WAYNESBORO, PA 17268
 MAP/GRID/PARCEL: 0050/0024/0322
 DEED BOOK 01652 PAGE 00685
 USE: AGRICULTURAL
 ZONING: A(R)
 AREA: 154.00 AC

1.
 ACCOUNT ID: 18-019493
 N/F
 DRB GROUP MID-ATLANTIC LLC
 PARCEL ADDRESS: 11301 BOVEY LN
 SMITHSBURG MD 21783
 MAILING ADDRESS: 2099 GAITHER RD STE 600
 ROCKVILLE, MD 20850
 MAP/GRID/PARCEL: 0050/0017/0309
 DEED BOOK 06871 PAGE 00421
 USE: RESIDENTIAL
 ZONING: RT
 AREA: 153.45 AC

2.
 ACCOUNT ID: 18-002264
 N/F
 TOOTHMAN RONALD G & TOOTHMAN COLLEEN M
 PARCEL ADDRESS: 11350 KIEFFER FUNK RD
 SMITHSBURG MD 21783
 MAILING ADDRESS: P.O.BOX 185
 CHEWSVILLE, MD 21721
 MAP/GRID/PARCEL: 0051/0018/0307
 DEED BOOK 000787 PAGE 00975
 USE: AGRICULTURAL
 ZONING: A(R)
 AREA: 22.4 AC

3.
 ACCOUNT ID: 18-009315
 N/F
 MARTIN NATHAN R & MARTIN REBECCA
 PARCEL ADDRESS: 11219 KIEFFER FUNK RD
 SMITHSBURG MD 21783
 MAILING ADDRESS: 11131 WHITE HALL RD
 SMITHSBURG MD 21783
 MAP/GRID/PARCEL: 0051/0018/0306
 DEED BOOK 02887 PAGE 00635
 USE: AGRICULTURAL
 ZONING: A(R)
 AREA: 145.63 AC

4.
 ACCOUNT ID: 18-042347
 N/F
 RIDENOUR HAROLD E JR L/E & RIDENOUR PEGGY J L/E
 PARCEL ADDRESS: 11047 WHITE HALL RD
 SMITHSBURG MD 21783
 MAILING ADDRESS: 11047 WHITE HALL RD
 SMITHSBURG MD 21783
 MAP/GRID/PARCEL: 0051/0019/0718
 DEED BOOK 07072 PAGE 00360
 USE: RESIDENTIAL
 ZONING: A(R)
 AREA: 1.69 AC

5.
 ACCOUNT ID: 18-017121
 N/F
 RIDENOUR DAVID MATTHEW & RIDENOUR BETHANY DIANE
 PARCEL ADDRESS: 11035 WHITE HALL RD
 SMITHSBURG MD 21783
 MAILING ADDRESS: 11035 WHITE HALL RD
 SMITHSBURG MD 21783
 MAP/GRID/PARCEL: 0050/0024/0323
 DEED BOOK 06201 PAGE 00420
 USE: RESIDENTIAL
 ZONING: A(R)
 AREA: 1.77 AC

6.
 ACCOUNT ID: 18-018195
 N/F
 SCRIVENER THOMAS F
 PARCEL ADDRESS: 10901 WHITE HALL RD
 SMITHSBURG MD 21783
 MAILING ADDRESS: 10901 WHITE HALL RD
 SMITHSBURG MD 21783
 MAP/GRID/PARCEL: 0051/0019/0061
 DEED BOOK 00509 PAGE 00499
 USE: AGRICULTURAL
 ONING: A(R)
 AREA: 120.51 AC

7.
 ACCOUNT ID: 18-040344
 N/F
 SCRIVENNER JOHN T & SCRIVENER LISA M
 PARCEL ADDRESS: 11005 WHITE HALL RD
 SMITHSBURG, MD 21783
 MAILING ADDRESS: 11005 WHITE HALL RD
 SMITHSBURG, MD 21783
 MAP/GRID/PARCEL: 0050/0024/1699
 DEED BOOK 01225 PAGE 00203
 USE: AGRICULTURAL
 ZONING: A(R)
 AREA: 15.7 AC

8.
 ACCOUNT ID: 18-014831
 N/F
 NEEDY PAUL E
 PARCEL ADDRESS: 10863 WHITE HALL RD
 SMITHSBURG MD 21783
 MAILING ADDRESS: 10863 WHITE HALL RD
 SMITHSBURG, MD 21783
 MAP/GRID/PARCEL: 0050/0024/0324
 DEED BOOK 03779 PAGE 00209
 USE: AGRICULTURAL
 ZONING: A(R)
 AREA: 8.41 AC

9.
 ACCOUNT ID: 18-040387
 N/F
 FERGUSON MICHAEL T &
 FERGUSON LISA ANN
 PARCEL ADDRESS: 10853 WHITE HALL RD
 SMITHSBURG MD 21783
 MAILING ADDRESS: 10849 WHITE HALL RD
 SMITHSBURG, MD 21783
 MAP/GRID/PARCEL: 0050/0024/1703
 DEED BOOK 01414 PAGE 00755
 USE: RESIDENTIAL
 ZONING: A(R)
 AREA: 1.48 AC

10.
 ACCOUNT ID: 18-028328
 N/F
 FERGUSON MICHAEL T &
 FERGUSON LISA A
 PARCEL ADDRESS: 10849 WHITE HALL RD
 SMITHSBURG MD 21783
 MAILING ADDRESS: 10849 WHITE HALL RD
 SMITHSBURG MD 21783
 MAP/GRID/PARCEL: 0050/0024/1649
 DEED BOOK 01072 PAGE 01021
 USE: RESIDENTIAL
 ZONING: A(R)
 AREA: 40,336 SF

11.
 ACCOUNT ID: 16-008664
 N/F
 PRYOR DAVID ET AL &
 PRYOR KENNETH
 PARCEL ADDRESS: MT AETNA RD
 HAGERSTOWN, MD 21742
 MAILING ADDRESS: 20615 MT AETNA
 HAGERSTOWN, MD 21742
 MAP/GRID/PARCEL: 0050/0023/0001
 DEED BOOK 06275 PAGE 00017
 USE: AGRICULTURAL
 ZONING: A(R)
 AREA: 70.29 AC

12.
 ACCOUNT ID: 18-019507
 N/F
 DRB GROUP MID-ATLANTIC LLC
 PARCEL ADDRESS: MT AETNA RD
 HAGERSTOWN MD 21742
 MAILING ADDRESS: 2099 GAITHER RD STE 600
 ROCKVILLE, MD 20850
 MAP/GRID/PARCEL: 0050/0023/0321
 DEED BOOK 07544 PAGE 00155
 USE: RESIDENTIAL
 ZONING: RT
 AREA: 63.97 AC



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**ARCOLA TOWERS
 WHITEHALL
 10944 WHITE HALL RD
 SMITHSBURG, MD 21783**



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10-08-25	ZONING REVIEW	C
10-14-25	ZONING	1

PROJECT NO: 1164.015
 DESIGNER: R.S.
 ENGINEER: C.S.

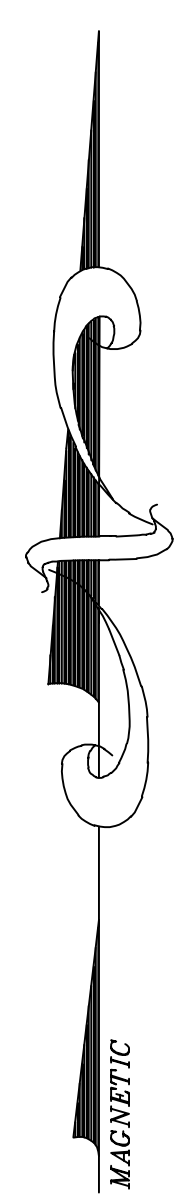
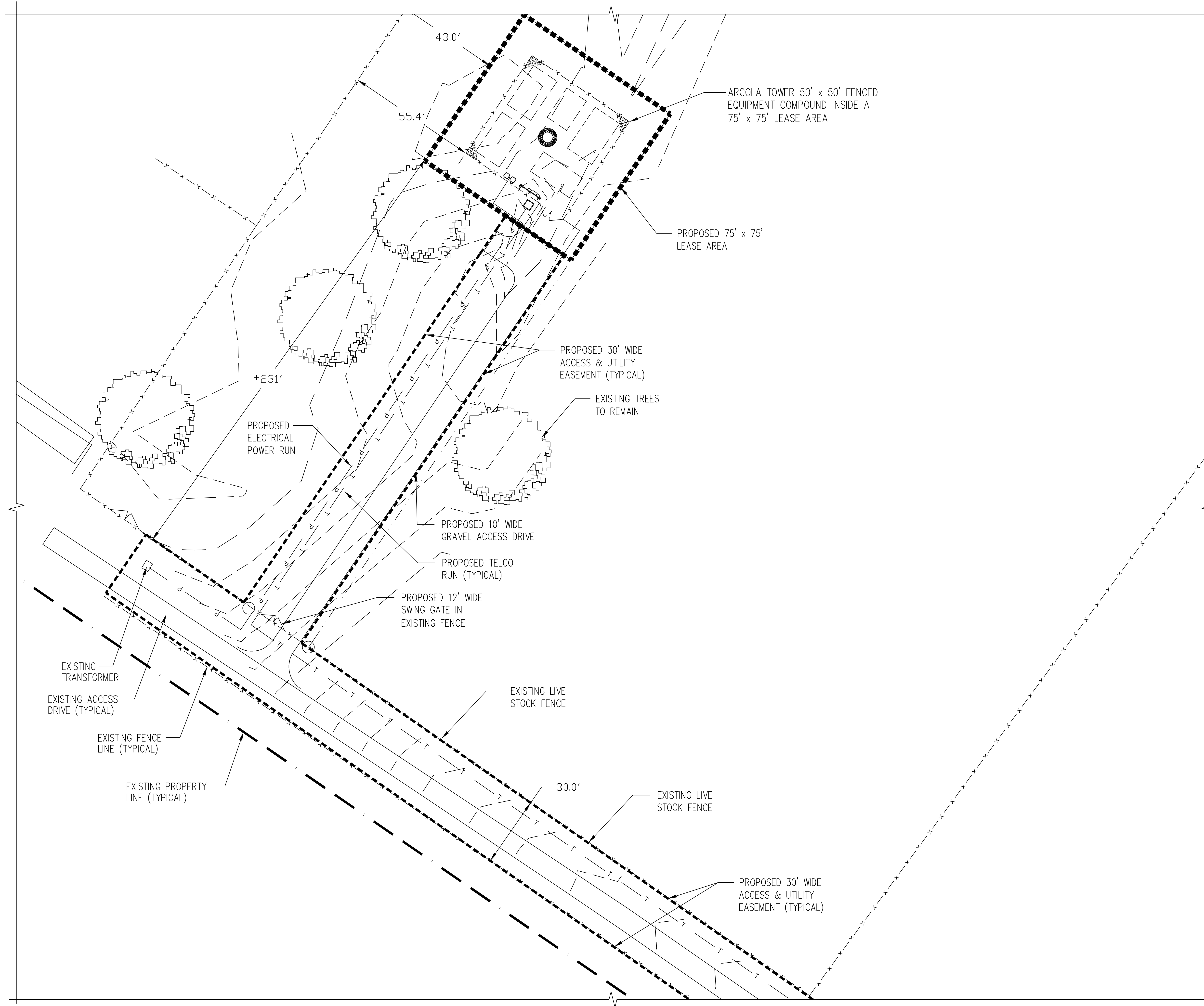
THESE DRAWINGS ARE FORMATTED
 TO BE FULL-SIZE AT 24"x36"
 0 1/2 1
 GRAPHIC SCALE IN INCHES

SHEET TITLE:

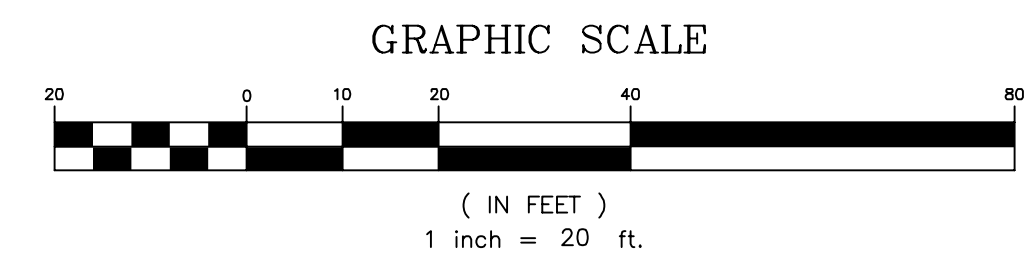
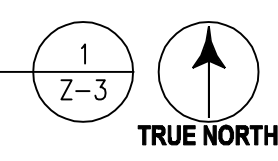
ADJOINERS

SHEET NUMBER:

Z-2



PARTIAL SITE PLAN
SCALE: 1" = 20'



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communication services, inc.
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arcola
TOWERS
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PHONE: (571) 895 3990

**ARCOLA TOWERS
WHITEHALL**
10944 WHITE HALL RD
SMITHSBURG, MD 21783



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10-14-25	ZONING	1

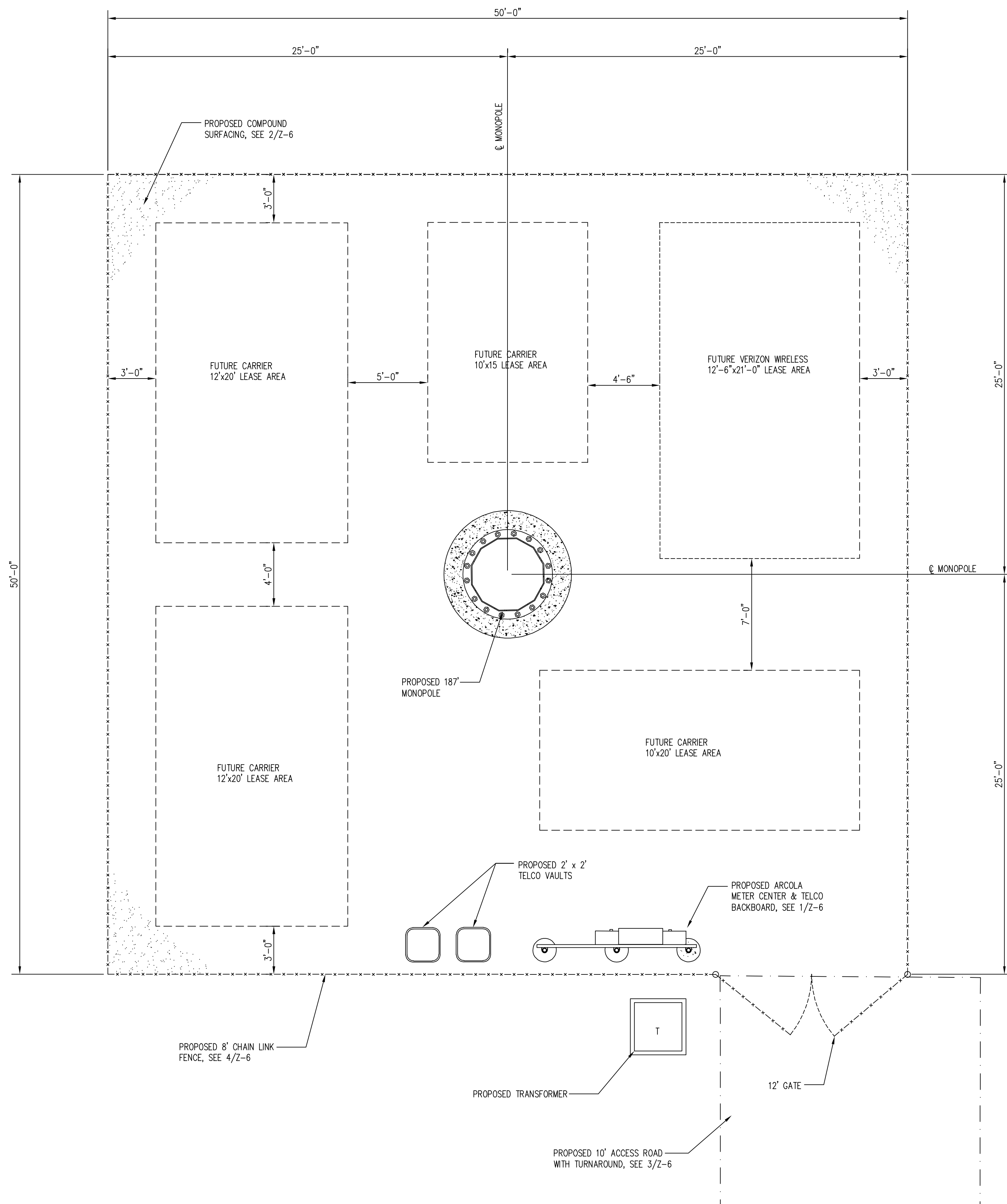
PROJECT NO: 1164.015
DESIGNER: R.S.
ENGINEER: C.S.

THESE DRAWINGS ARE FORMATTED TO BE FULL-SIZE AT 24"x36"
0 1/2 1
GRAPHIC SCALE IN INCHES
SHEET TITLE:

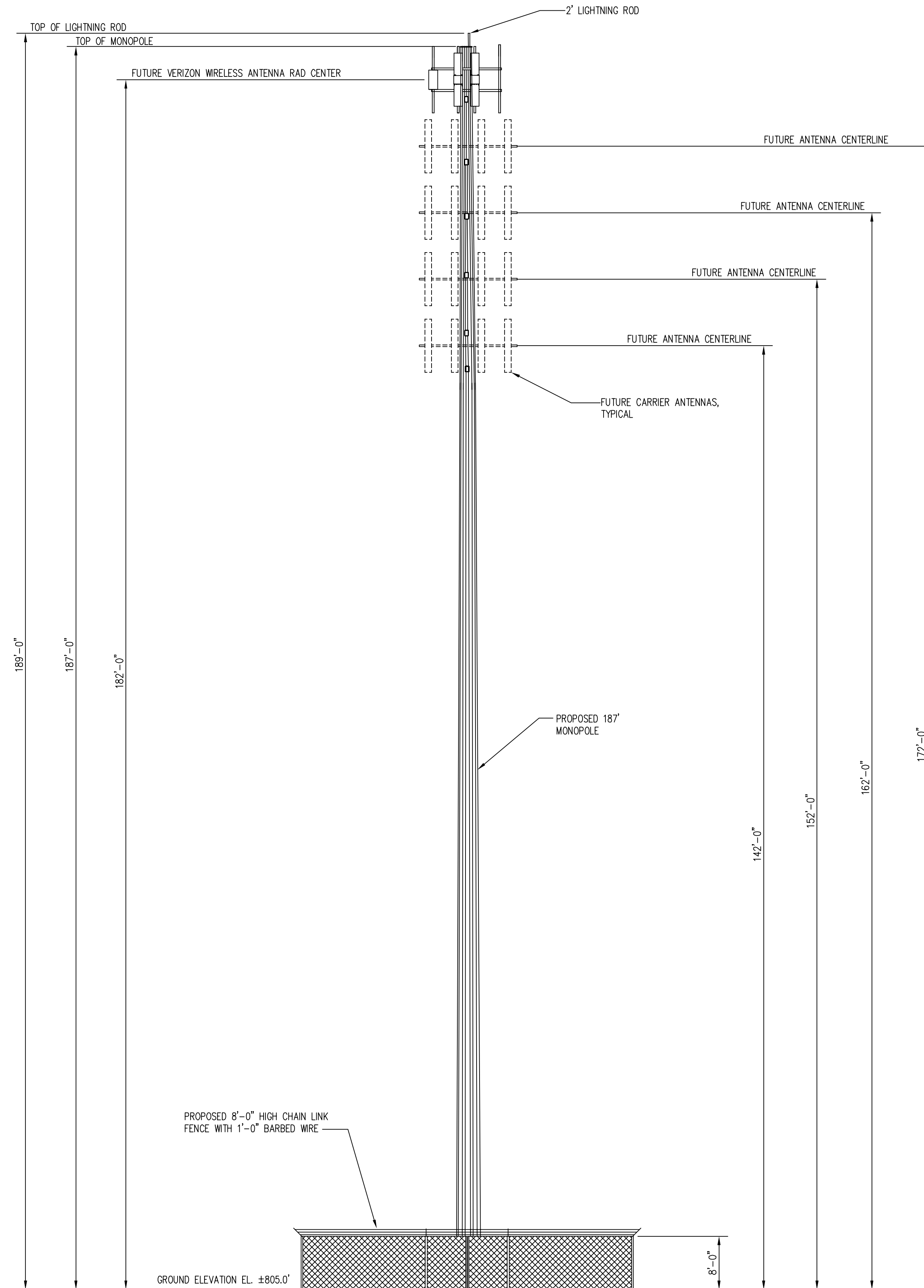
PARTIAL SITE PLAN

SHEET NUMBER:

Z-3



COMPOUND PLAN 1
SCALE: 1/4"=1'-0" Z-4
TRUE NORTH



MONOPOLE ELEVATION 2
SCALE: 3/32"=1'-0" Z-4

DESIGN NOTES

1. PROPOSED ANTENNA, ANTENNAS ARRAYS, CABLES, AND OTHER MONOPOLE ATTACHMENTS WILL BE PAINTED GRAY TO BETTER BLEND WITH GALVANIZED METAL MONOPOLE AND FURTHER MINIMIZE THE VISUAL IMPACT.
2. THERE IS NO ILLUMINATION PROPOSED FOR THE MONOPOLE.

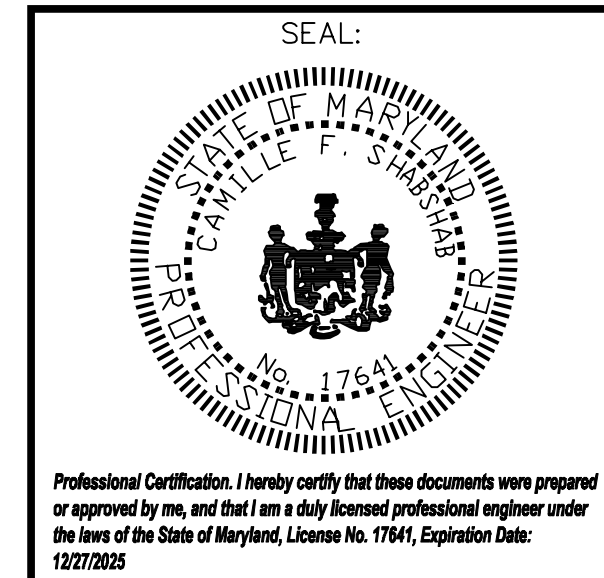


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SUITE 430
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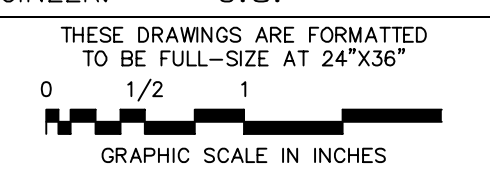
**ARCOLA TOWERS
WHITEHALL
10944 WHITE HALL RD
SMITHSBURG, MD 21783**



SUBMITTALS

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10-14-25	ZONING	1

PROJECT NO: 1164.015
DESIGNER: R.S.
ENGINEER: C.S.



SHEET TITLE:

**COMPOUND PLAN
& MONOPOLE
ELEVATION**

SHEET NUMBER:

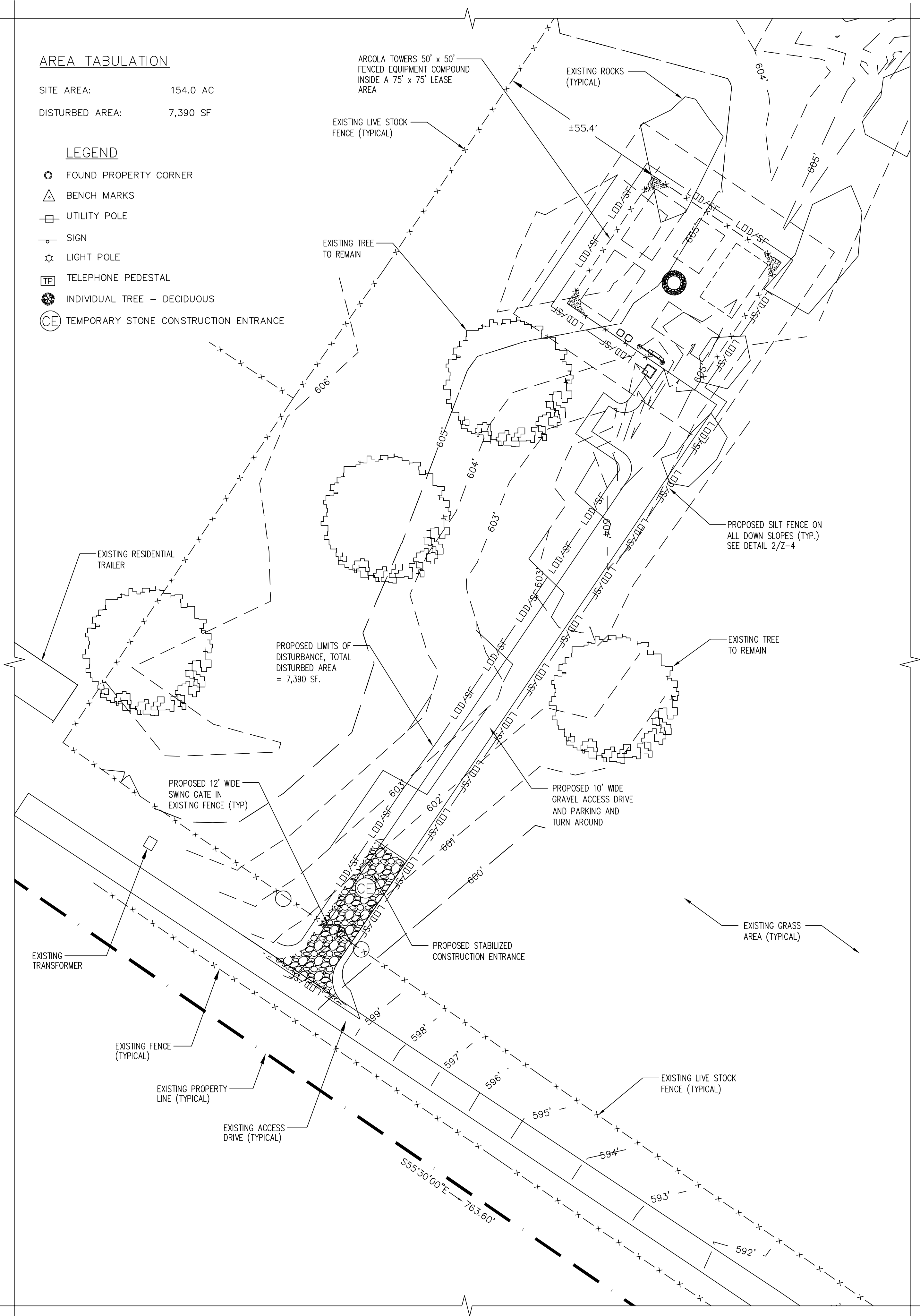
Z-4

AREA TABULATION

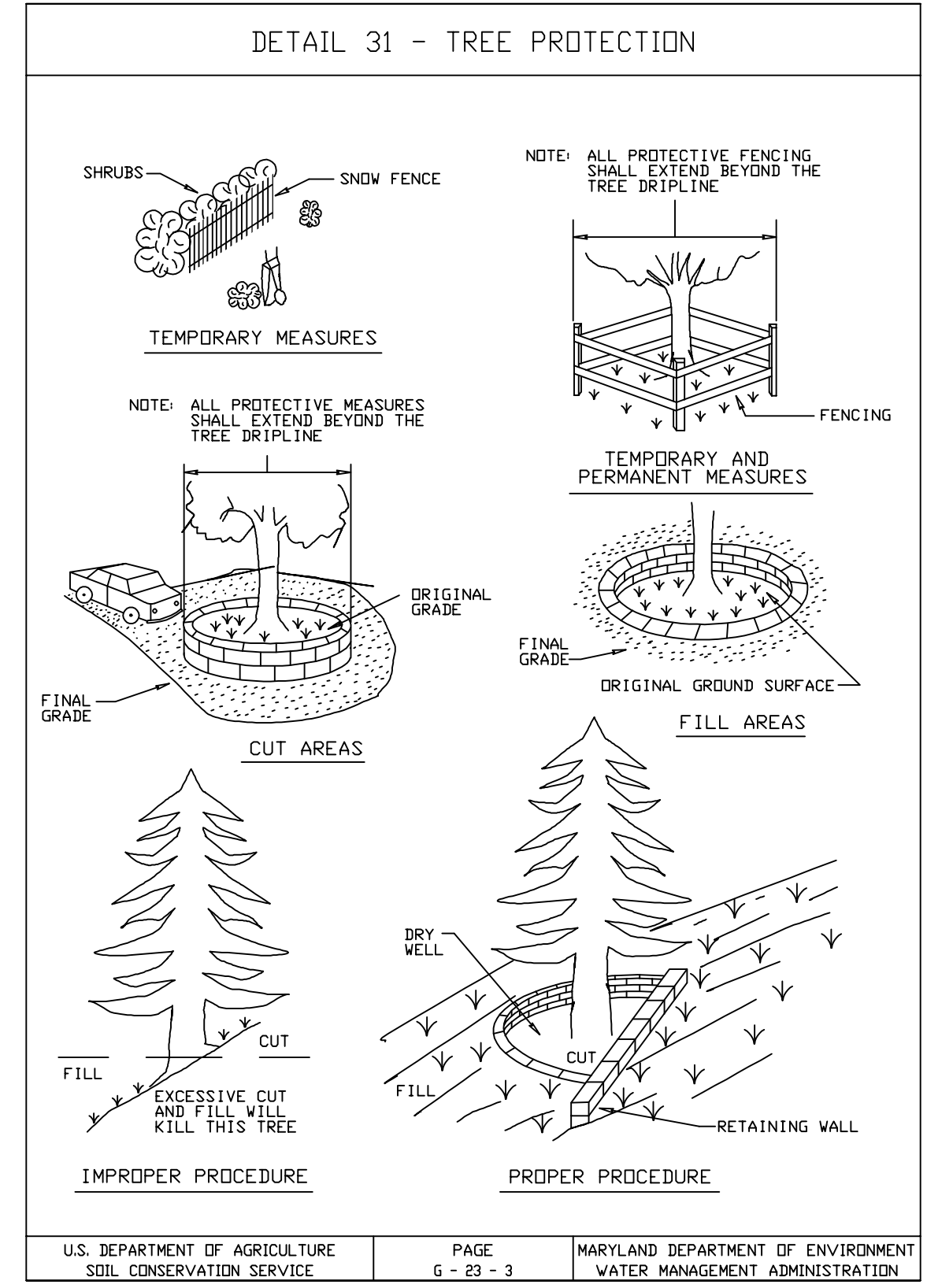
SITE AREA: 154.0 AC
 DISTURBED AREA: 7,390 SF

LEGEND

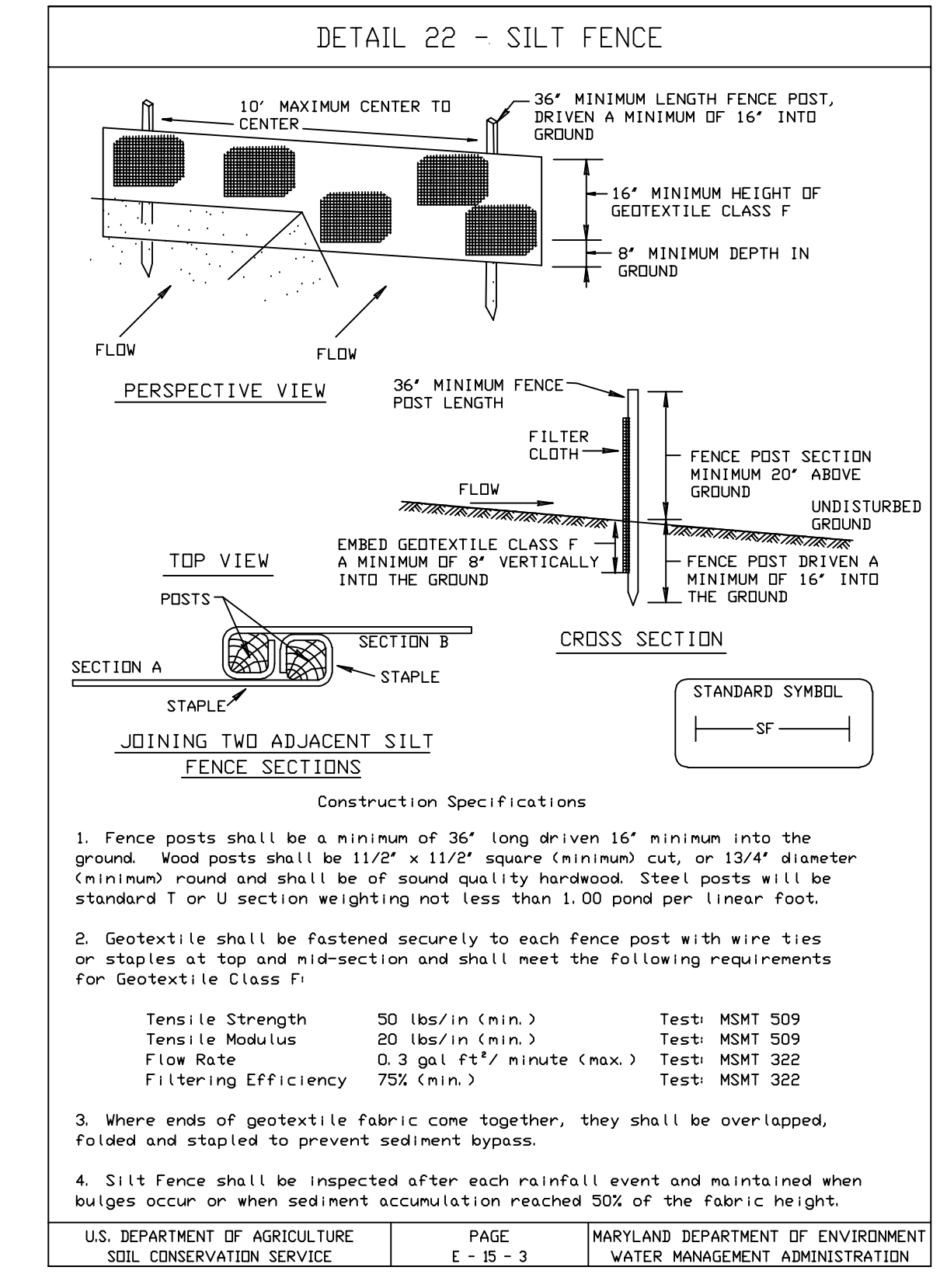
- FOUND PROPERTY CORNER
- △ BENCH MARKS
- UTILITY POLE
- ◇ SIGN
- ☆ LIGHT POLE
- TP TELEPHONE PEDESTAL
- INDIVIDUAL TREE - DECIDUOUS
- CF TEMPORARY STONE CONSTRUCTION ENTRANCE



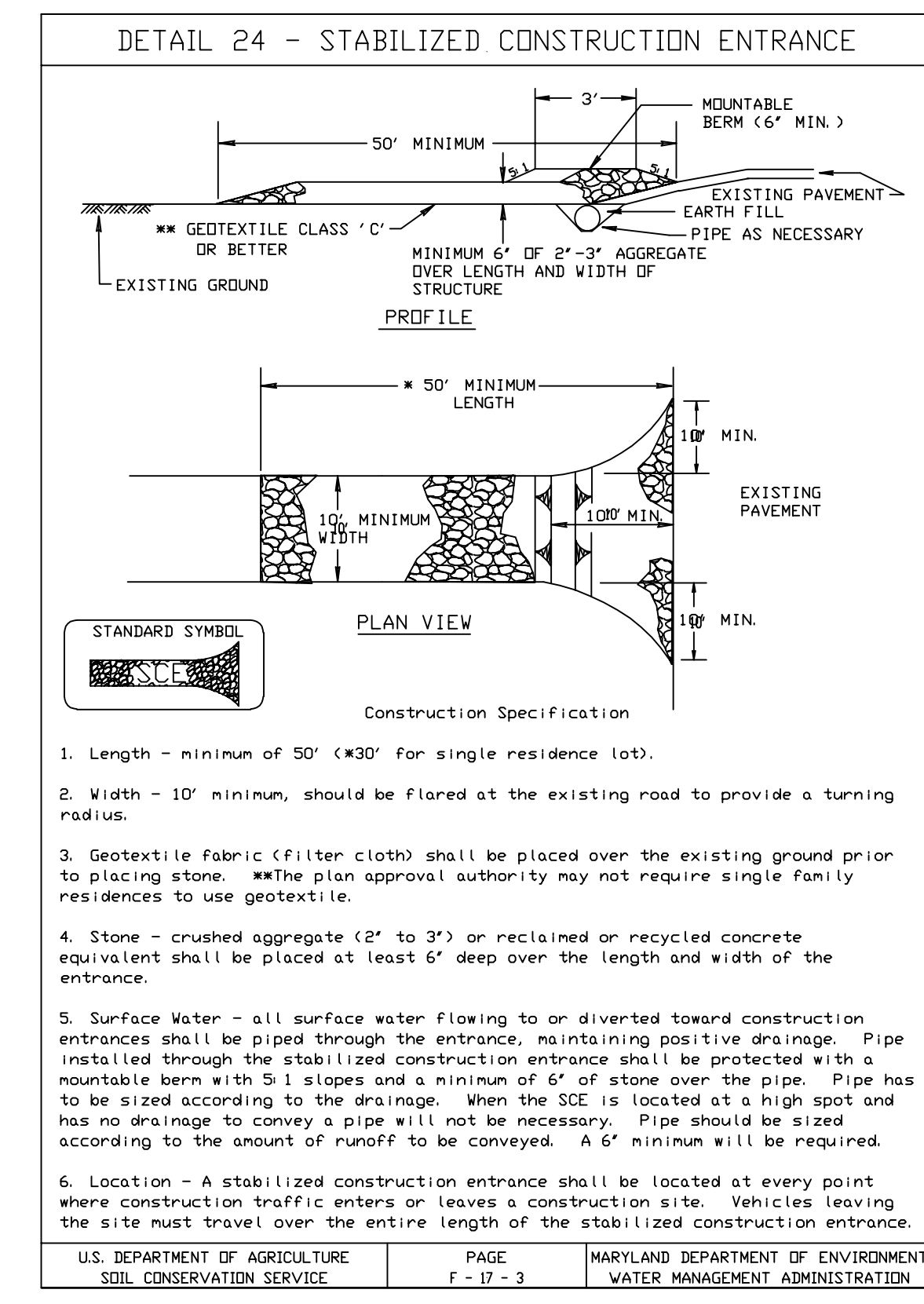
EROSION AND SEDIMENT CONTROL PLAN
 SCALE: 1" = 20'



TREE PROTECTION FENCE DETAIL
 SCALE: N.T.S.



SILT FENCE DETAIL
 SCALE: N.T.S.



STABILIZED CONSTRUCTION ENTRANCE DETAIL
 SCALE: N.T.S.



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**ARCOLA TOWERS
 WHITEHALL**
 10944 WHITE HALL RD
 SMITHSBURG, MD 21783



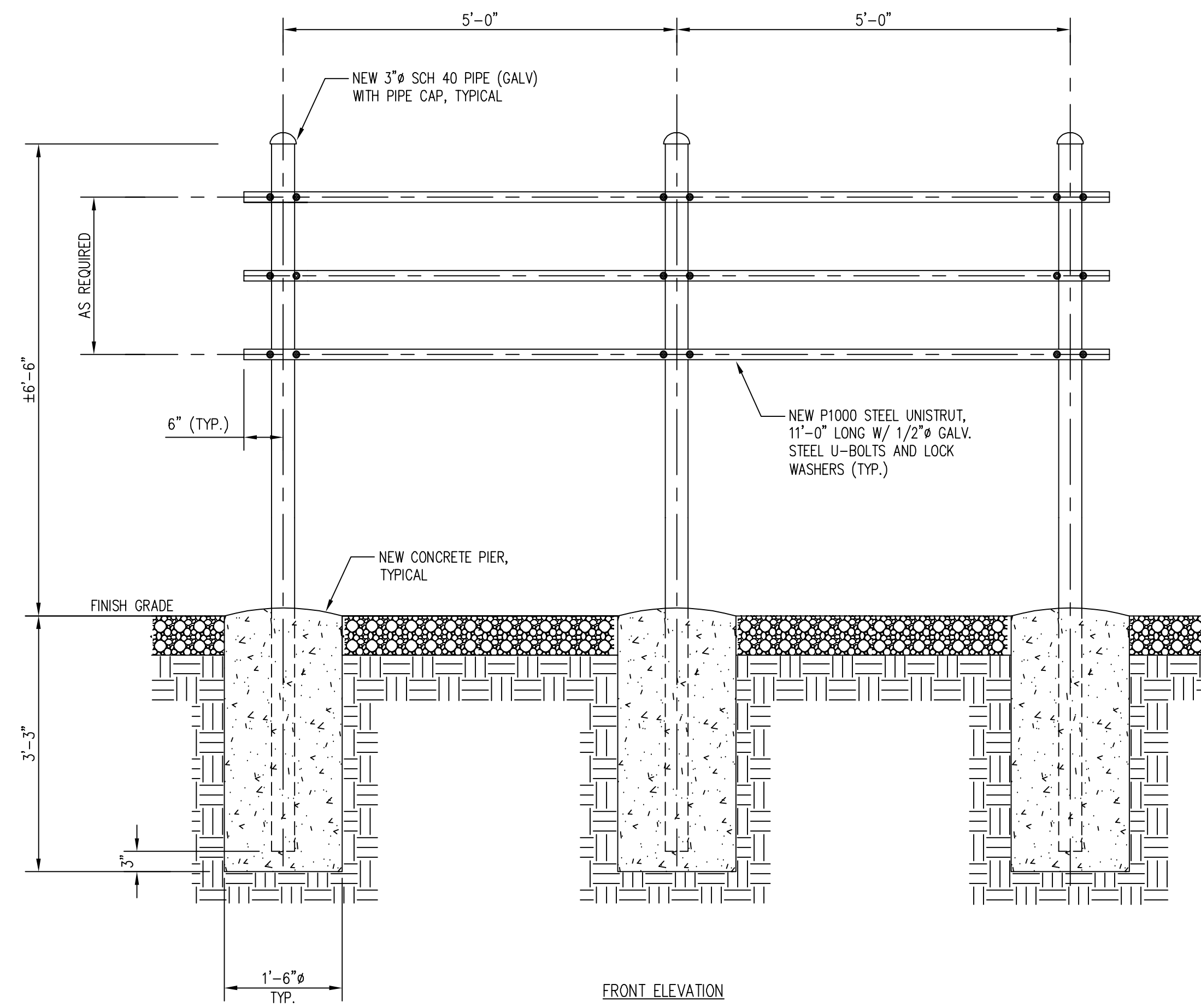
SUBMITTALS

DATE	DESCRIPTION	REV.
09-09-25	ZONING REVIEW	A
09-24-25	ZONING REVIEW	B
10-08-25	ZONING REVIEW	C
10-14-25	ZONING	1

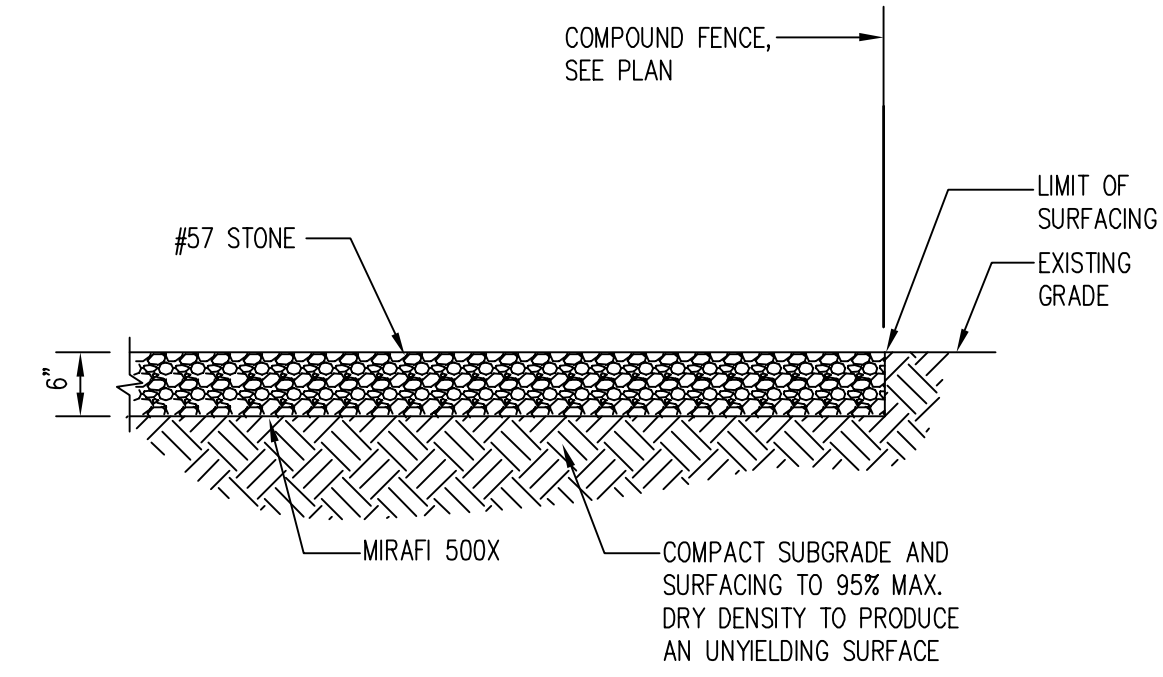
PROJECT NO: 1164.015
 DESIGNER: R.S.
 ENGINEER: C.S.
 THESE DRAWINGS ARE FORMATTED TO BE FULL-SIZE AT 24"x36"
 0 1/2 1
 GRAPHIC SCALE IN INCHES
 SHEET TITLE:

**EROSION AND
 SEDIMENT
 CONTROL PLAN**

SHEET NUMBER:
Z-5

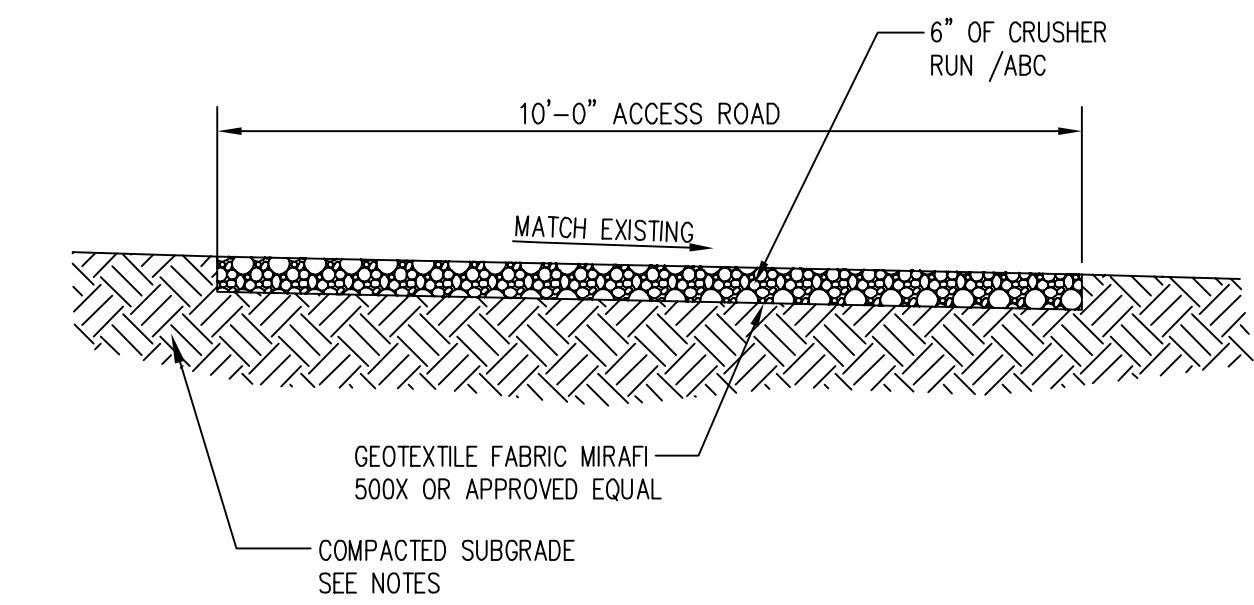


UTILITY FRAME DETAIL 1
SCALE: 3/4"=1'-0" Z-6



NOTE:
COMPOUND AREA SHALL BE CLEARED AND GRUBBED. REMOVE UNSUITABLE LOOSE OR SOFT SOIL, ORGANIC MATERIAL OR RUBBLE TO FIRM GRADE. FILL UNDERCUT AND COMPACT UP TO 6" BELOW FINISH GRADE. PLACE A MIRAFIX 500X SOIL STABILIZATION FABRIC ON SUBGRADE. FILL WITH 6" OF AASHTO 57 STONE TO FINISH GRADE.

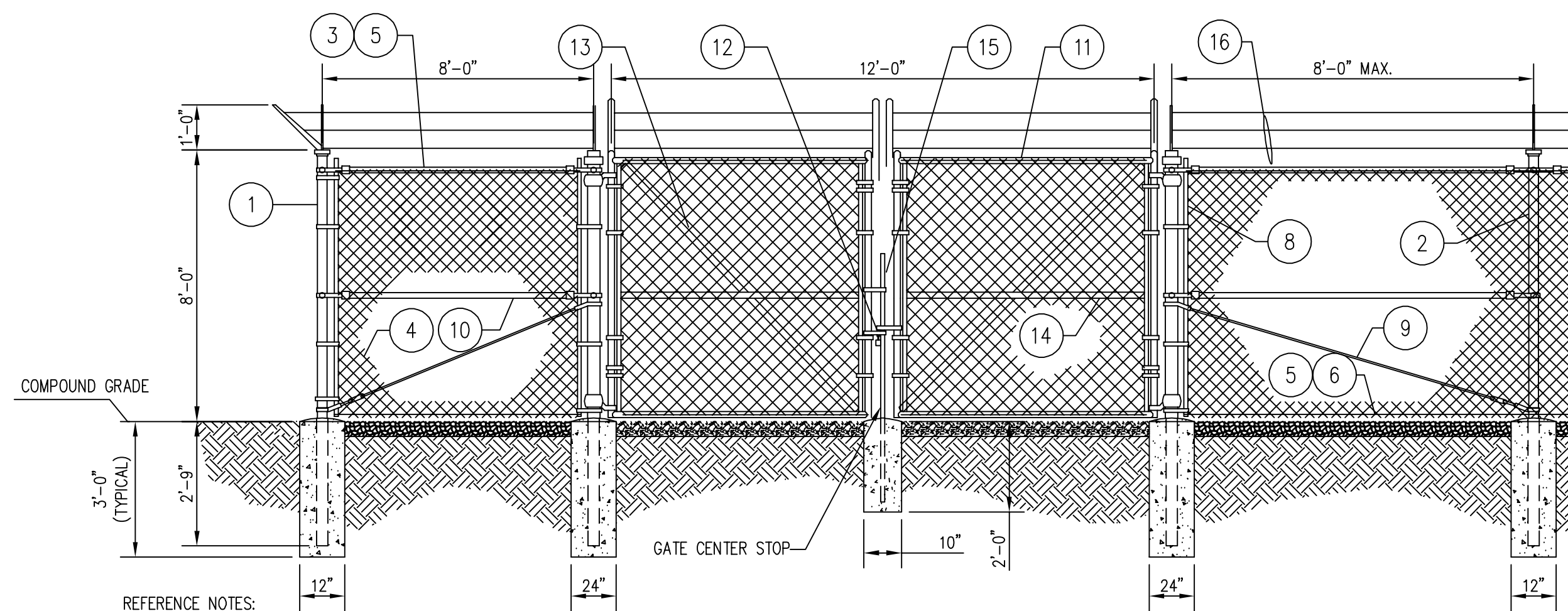
COMPOUND SURFACING DETAIL 2
SCALE: 1"=1'-0" Z-6



ACCESS ROAD:
FILL, UNDERCUT AND COMPACT TEMPORARY ACCESS ROAD UP TO 6" BELOW FINISH GRADE. PLACE MIRAFIX 500X STABILIZATION FABRIC ON SUBGRADE. PULL TIGHT AND STAKE IN PLACE. PLACE AND COMPACT 6" DEPTH OF CRUSHER RUN/ABC TO FINISH GRADE.

COMPACTION NOTES:
1. SURFACE COURSE AND BASE COURSE SHALL BE COMPACTED TO 98% MAX. DRY DENSITY STANDARD PROCTOR.
2. SUBGRADE SOIL SHALL BE COMPACTED TO 95% MAX. DRY DENSITY STANDARD PROCTOR.

GRAVEL ACCESS ROAD DETAIL 3
SCALE: N.T.S. Z-6



- REFERENCE NOTES:
- 1 GATE POST: 6" NOMINAL SCHEDULE 40 PIPE. CORNER, END OR PULL POST: 4" NOMINAL SCHEDULE 40 PIPE.
 - 2 LINE POST: 3" SCHEDULE 40 PIPE, PER ASTM-F1083. LINE POSTS SHALL BE EQUALLY SPACED AT MAXIMUM 6'-0" O.C.
 - 3 TOP RAIL & BRACE RAIL: 1 1/2" PIPE, PER ASTM-F1083.
 - 4 FABRIC: 9 GA CORE WIRE SIZE 2" MESH, CONFORMING TO ASTM-A392.
 - 5 TIE WIRE: MINIMUM 11 GA GALVANIZED STEEL AT POSTS AND RAILS A SINGLE WRAP OF FABRIC TIE AND AT TENSION WIRE BY HOG RINGS SPACED MAX. 24" INTERVALS.
 - 6 TENSION WIRE: 9 GA. GALVANIZED STEEL.
 - 7 NOT USED
 - 8 STRETCHER BAR.

- 9 3/8" DIAGONAL ROD WITH GALVANIZED STEEL TURNBUCKLE OR DIAGONAL THREADED ROD.
- 10 FENCE CORNER POST BRACE: 1 5/8" DIA. EACH CORNER EACH WAY.
- 11 GATE FRAME: 1 1/2" PIPE, PER ASTM-F1083.
- 12 NOT USED
- 13 NOT USED
- 14 GATE FRAME BRACE: 1 5/8" DIAMETER.
- 15 CENTER GATE STOP
- 16 BARBED WIRE: DOUBLE STRAND 12-1/2" O.D. TWISTED WIRE TO MATCH WITH FABRIC 14 GA, 4 PT. BARBS SPACED ON APPROXIMATELY 5" CENTERS.

- GENERAL NOTES:
1. INSTALL FENCING PER ASTM F-567
 2. INSTALL SWING GATES PER ASTM F-900
 3. FENCE PIPE AND COMPONENTS SHALL BE GALVANIZED.
 4. GATE FRAMES SHALL BE WELDED. WELDING SHALL BE COATED WITH (3) COATS OF COLD GALV. (OR EQUAL).
 5. POSTS SHALL HAVE END-CAPS.
 6. GATES SHALL HAVE LOCKING HARDWARE.
 7. PROVIDE GATE STOPS TO SECURE GATES IN OPEN POSITION.

GALVANIZED STEEL FENCE AND GATE DETAIL 4
SCALE: N.T.S. Z-6

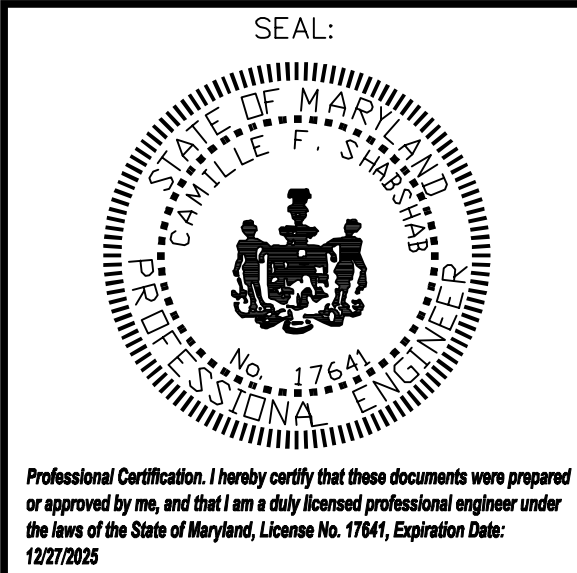


6100 EXECUTIVE BLVD.
SUITE 430
ROCKVILLE, MD 20852
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**ARCOLA TOWERS
WHITEHALL
10944 WHITE HALL RD
SMITHSBURG, MD 21783**



SUBMITTALS

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10-08-25	ZONING REVIEW	C
10-14-25	ZONING	1

PROJECT NO: 1164.015
DESIGNER: R.S.
ENGINEER: C.S.

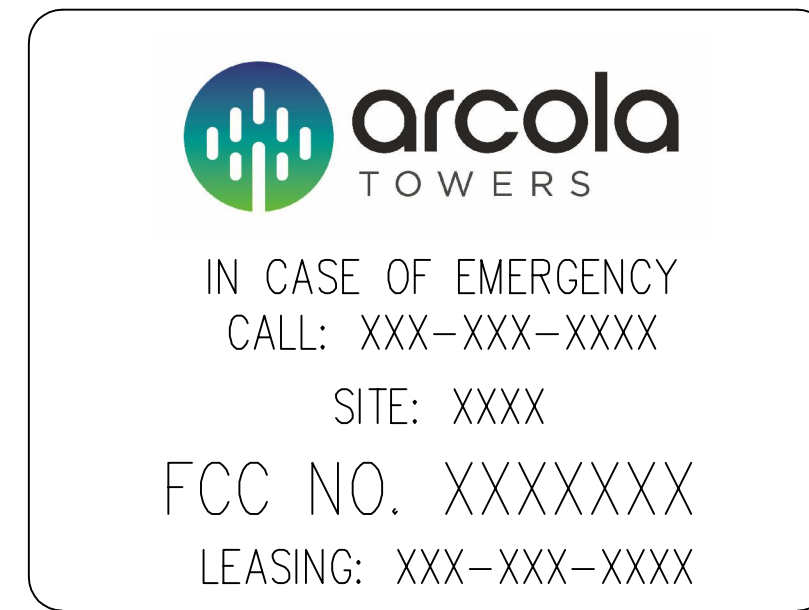
THESE DRAWINGS ARE FORMATTED TO BE FULL-SIZE AT 24"x36"
0 1/2 1
GRAPHIC SCALE IN INCHES

SITE DETAILS

SHEET NUMBER:

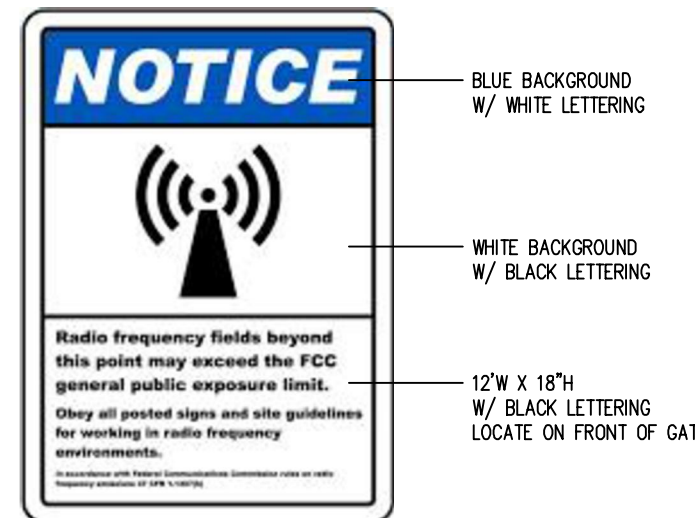
Z-6

SITE SIGNAGE DETAILS



WHITE BACK GROUND
W/ BLACK LETTERING
(SITE ID SIGN SHALL NOT EXCEED 6 SF)

MARKETING / ID SIGN 1
SCALE: N.T.S. Z-7



BLUE BACKGROUND
W/ WHITE LETTERING

WHITE BACKGROUND
W/ BLACK LETTERING

12'W X 18'H
W/ BLACK LETTERING
LOCATE ON FRONT OF GATE

RF NOTICE SIGN 3
SCALE: N.T.S. Z-7



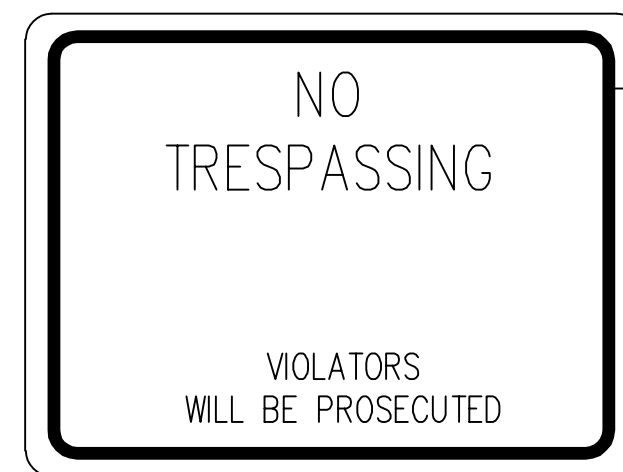
ORANGE BACKGROUND
W/ BLACK LETTERING

ORANGE BACKGROUND
W/ BLACK LETTERING

WHITE BACKGROUND
W/ BLACK LETTERING

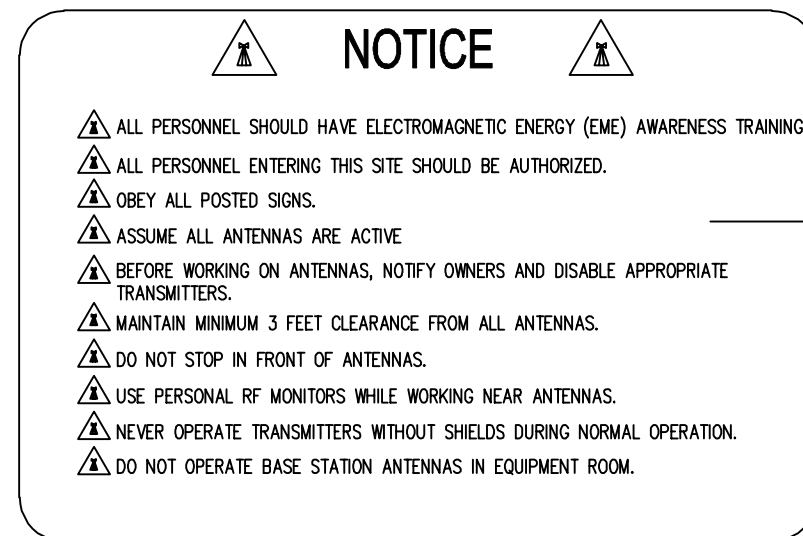
12'W X 18'H
LOCATE AT BASE OF TOWER

RF WARNING SIGN 5
SCALE: N.T.S. Z-7



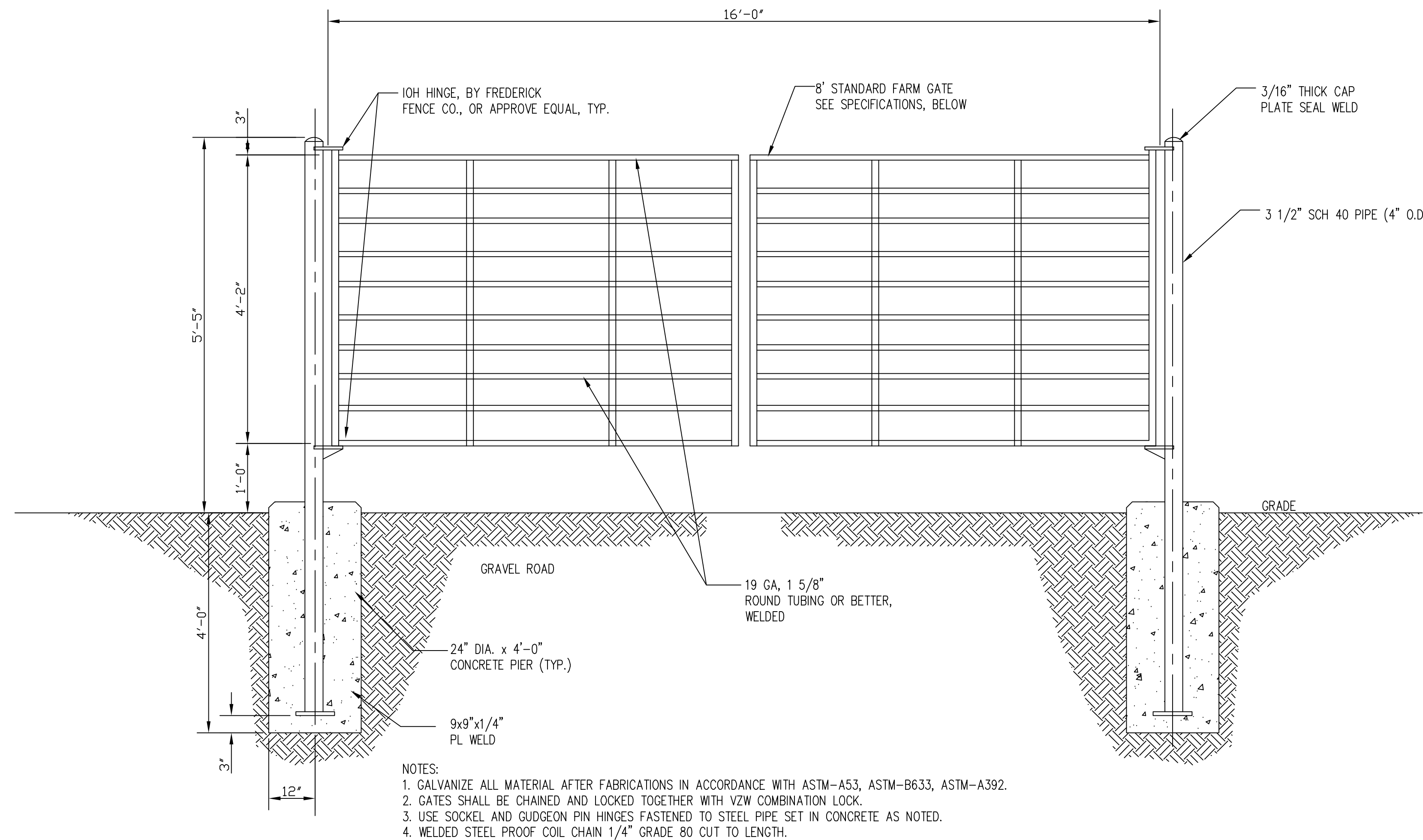
WHITE BACKGROUND
W/ RED LETTERING
18'H X 24"W

NO TRESPASSING SIGN 2
SCALE: N.T.S. Z-7



ORANGE BACKGROUND
W/ BLACK LETTERING
12'W X 18'H

RF NOTICE SIGN 2 4
SCALE: N.T.S. Z-7



GATE DETAIL 5
SCALE: NTS Z-7

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arcola
TOWERS

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**ARCOLA TOWERS
WHITEHALL**
10944 WHITE HALL RD
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10-14-25	ZONING	1

PROJECT NO: 1164.015
DESIGNER: R.S.
ENGINEER: C.S.

THESE DRAWINGS ARE FORMATTED TO BE FULL-SIZE AT 24"x36"

0 1/2 1
GRAPHIC SCALE IN INCHES

SHEET TITLE:

**SIGNAGE DETAILS
AND GATE DETAIL**

SHEET NUMBER:

Z-7

Exhibit “2”



Arcola Towers: Whitehall

Alternative Candidate Analysis

Arcola Towers submits this document to address the alternative candidates it considered during its selection process. Verizon has a specific search ring in the Whitehall Road area in Washington County, Maryland, with a requested antenna centerline of 182 ft.

Existing Structures in the Search Ring

Arcola Towers was able to confirm that there are no existing structures within the search ring. There are no structures at 182 ft. in height within the specified search ring, nor within a reasonable distance outside of the search ring. Arcola Towers utilized existing databases (Tower Source, etc.) to determine the existence of nearby towers while confirming that information via driving in and around the search ring area.



Existing Structures outside of the Search Ring

Arcola Towers then located the nearest existing tower locations as shown on the attached map in a 1-mile radius:

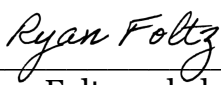


This map illustrates a 1-mile radius from the proposed search ring center, and as shown on the map, there are no existing towers. The closest tower is 1.29 miles to the Southeast of the search ring and will not cover the intended area.

Conclusion

As noted above, there are no existing structures within the search area, and as noted above, the closest existing tower to the Search Ring Center is a 195' self-support American Tower site that is 1.29 miles away and too far to cover Verizon's intended coverage area. Arcola has leased a property from a willing landlord whose property will be developed in accordance with Washington County regulations.

I certify that the foregoing is true and correct:



Ryan Foltz on behalf of Arcola Towers

Exhibit “3”



Structural Design Report

187' Monopole

Site: MD-003 Whitehall, MD

Prepared for: ARCOLA TOWERS

by: Sabre Industries™

Job Number: 26-2535-TLJ

October 24, 2025

Monopole Profile..... 1

Foundation Design Summary (Preliminary) (Option 1)..... 2

Foundation Design Summary (Preliminary) (Option 2)..... 3

Pole Calculations..... 4-22

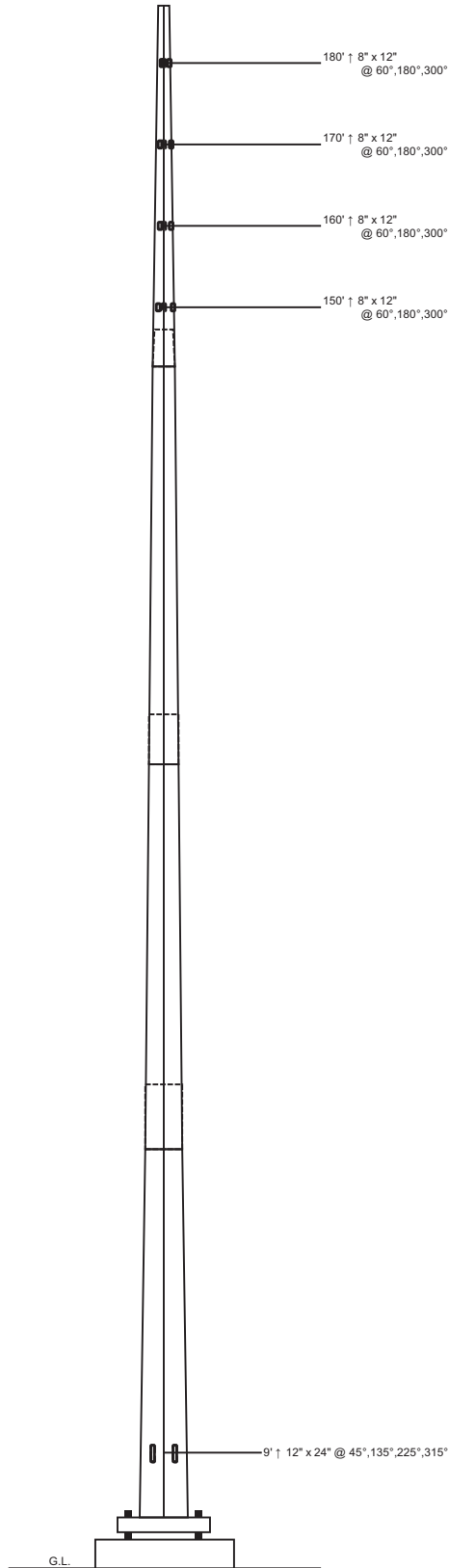
Foundation Calculations..... 23-31



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 40946, Expiration Date:

10/24/25
8/9/2027

Length (ft)	53'-3"	53'-6"	53'-6"	44'-6"
Number Of Sides	18			
Thickness (in)	7/16"	3/8"	1/4"	
Lap Splice (ft)	8'-0"	6'-3"	A	
Top Diameter (in)	53.74"	41.98"	19"	
Bottom Diameter (in)	68.53"	56.84"	31.36"	
Taper (in/ft)		0.2777		
Grade		A572-65		
Weight (lbs)	18347	13132	8434	3722
Overall Steel Height (ft)	186			



Designed Appurtenance Loading

Elev	Description	Tx-Line
187	(1) 2 sq. ft. EPA	
182	(1) 250 Sq.Ft. EPA (7000 lbs)	(6) 1 5/8"
172	(1) 200 sq. ft. EPA, 4,500 lb Weight	(6) 1 5/8"
162	(1) 175 Sq. Ft. EPA (4,500 lbs)	(6) 1 5/8"
152	(1) 150 sq. ft. EPA, 4,500 lb Weight	(6) 1 5/8"

Design Criteria - ANSI/TIA-222-H

Wind Speed (No Ice)	112 mph
Wind Speed (Ice)	40 mph
Design Ice Thickness	1.00 in
Risk Category	II
Exposure Category	C
Topographic Factor Procedure	Method 1 (Simplified)
Topographic Category	1
Ground Elevation	604 ft
Seismic Importance Factor, Ie	1.00
0.2-sec Spectral Response, Ss	0.124 g
1-sec Spectral Response, S1	0.042 g
Site Class	D (DEFAULT)
Seismic Design Category	B
Basic Seismic Force-Resisting System	Telecommunication Tower (Pole: Steel)

Limit State Load Combination Reactions

Load Combination	Axial (kips)	Shear (kips)	Moment (ft-k)	Deflection (ft)	Sway (deg)
1.2 D + 1.0 Wo	76.12	53.73	8054.65	17.04	10.96
0.9 D + 1.0 Wo	57.19	53.18	7848.28	16.57	10.64
1.2 D + 1.0 Di + 1.0 Wi	111.55	10.81	1676.58	3.71	2.38
1.2 D + 1.0 Ev + 1.0 Eh	77.75	1.91	316.36	0.71	0.45
0.9 D - 1.0 Ev + 1.0 Eh	55.39	1.9	310.55	0.69	0.44
1.0 D + 1.0 Wo (Service @ 60 mph)	63.52	13.73	2056.21	4.47	2.83

Base Plate Dimensions

Shape	Diameter	Thickness	Bolt Circle	Bolt Qty	Bolt Diameter
Round	81.5"	2.25"	75.75"	22	2.25"

Anchor Bolt Dimensions

Length	Diameter	Hole Diameter	Weight	Type	Finish
84"	2.25"	2.625"	2664.2	A615-75	Galv

Material List

Display	Value
A	4' - 6"

Notes

- 1) Antenna Feed Lines Run Inside Pole
- 2) All dimensions are above ground level, unless otherwise specified.
- 3) Weights shown are estimates. Final weights may vary.
- 4) Full Height Step Bolts
- 5) This tower design and, if applicable, the foundation design(s) shown on the following page(s) also meet or exceed the requirements of the 2021 International Building Code.
- 6) Tower Rating: 99.4%

 Sabre Industries 7101 Southbridge Drive P.O. Box 658 Sioux City, IA 51102-0658 Phone: (712) 258-6690 Fax: (712) 279-0814	Job:	26-2535-TLJ
	Customer:	ARCOLA TOWERS
	Site Name:	MD-003 Whitehall, MD
	Description:	187' Monopole
	Date:	10/24/2025

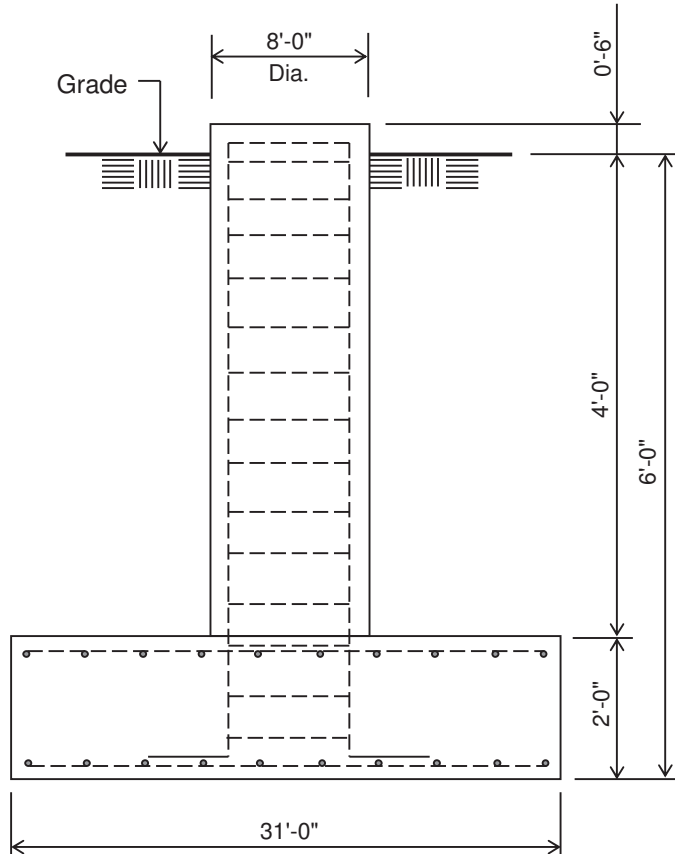
Information contained herein is the sole property of Sabre Communications Corporation, constitutes a trade secret as defined by Iowa Code Ch. 550 and shall not be reproduced, copied or used in whole or part for any purpose whatsoever without the prior written consent of Sabre Communications Corporation.

Customer: ARCOLA TOWERS

Site: MD-003 Whitehall, MD

187' Monopole

PRELIMINARY -NOT FOR CONSTRUCTION-



ELEVATION VIEW

(79.56 Cu. Yds.)

(1 REQUIRED; NOT TO SCALE)

Notes:

- 1) Concrete shall have a minimum 28-day compressive strength of 4,500 psi, in accordance with ACI 318-14.
- 2) Rebar to conform to ASTM specification A615 Grade 60.
- 3) All rebar to have a minimum of 3" concrete cover.
- 4) All exposed concrete corners to be chamfered 3/4".
- 5) The foundation design is based on presumptive clay soil as defined in ANSI/TIA-222-H-2017. It is recommended that a soil analysis of the site be performed to verify the soil parameters used in the design.
- 6) 4 ft of soil cover is required over the entire area of the foundation slab.
- 7) The bottom anchor bolt template shall be positioned as closely as possible to the bottom of the anchor bolts.

Rebar Schedule for Pad and Pier

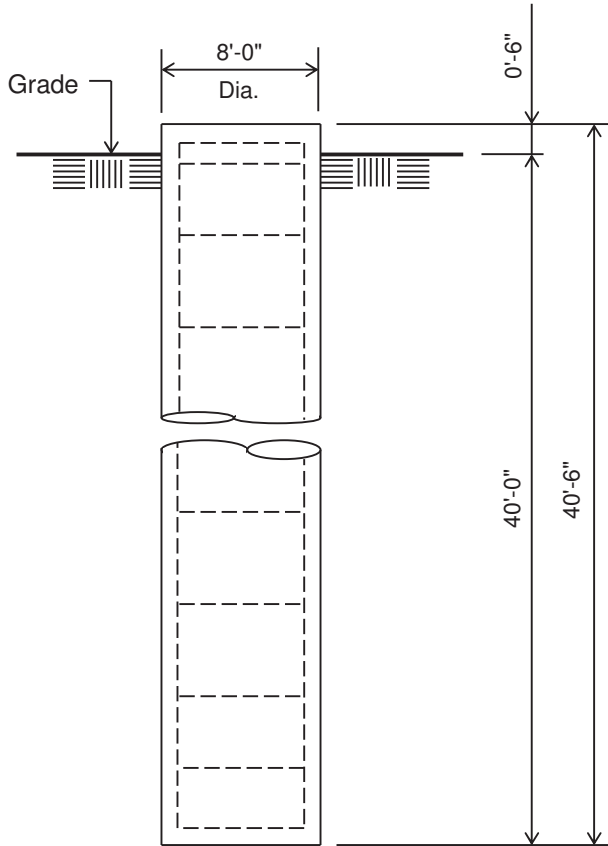
Rebar Schedule for Pad and Pier	
Pier	(54) #9 vertical rebar w/ hooks at bottom w/ #5 ties, (2) within top 5" of pier, then 4" C/C
Pad	(53) #10 horizontal rebar evenly spaced each way top and bottom (212 total)

Customer: ARCOLA TOWERS

Site: MD-003 Whitehall, MD

187' Monopole

PRELIMINARY -NOT FOR CONSTRUCTION-



ELEVATION VIEW

(75.40 Cu. Yds.)

(1 REQUIRED; NOT TO SCALE)

Notes:

- 1) Concrete shall have a minimum 28-day compressive strength of 4,500 psi, in accordance with ACI 318-14.
- 2) Rebar to conform to ASTM specification A615 Grade 60.
- 3) All rebar to have a minimum of 3" concrete cover.
- 4) All exposed concrete corners to be chamfered 3/4".
- 5) The foundation design is based on presumptive clay soil as defined in ANSI/TIA-222-H-2017. It is recommended that a soil analysis of the site be performed to verify the soil parameters used in the design.
- 6) The bottom anchor bolt template shall be positioned as closely as possible to the bottom of the anchor bolts.

Rebar Schedule for Pier	
Pier	(54) #9 vertical rebar w/ #5 ties, (2) within top 5" of pier, then 7" C/C

Tel: (416) 736-7453 Fax: (416) 736-4372 Web: www.guymast.com

Processed under license at:

Sabre Towers and Poles on: 24 oct 2025 at: 9:44:11
 =====

187' Monopole / MD-003 Whitehall, MD

* All pole diameters shown on the following pages are across corners.
 See profile drawing for widths across flats.

POLE GEOMETRY
 =====

ELEV	SECTION	No.	OUTSIDE	THICK	RESISTANCES		SPLICE	...OVERLAP...		w/t
ft	NAME	SIDE	DIAM	-NESS	*Pn	*Mn	TYPE	LENGTH	RATIO	
			in	in	kip	ft-kip		ft		
186.0	A	18	19.29	0.250	1105.4	424.0				12.2
			30.57	0.250	1660.2	1018.9				
146.0	A/B	18	30.57	0.250	1660.2	1018.9	SLIP	4.50	1.75	
			31.34	0.375	2696.2	1683.6				
141.5	B	18	31.34	0.375	2696.2	1683.6				13.6
			43.38	0.375	3577.9	3113.2				
98.7	B/C	18	43.38	0.375	3577.9	3113.2	SLIP	6.25	1.72	
			44.40	0.438	4418.9	3926.0				
92.5	C	18	44.40	0.438	4418.9	3926.0				16.8
			55.44	0.438	5180.1	5769.9				
53.2	C/D	18	55.44	0.438	5180.1	5769.9	SLIP	8.00	1.72	
			56.84	0.438	5266.2	6015.7				
45.2	D	18	56.84	0.438	5266.2	6015.7				21.7
			69.58	0.438	5950.1	8345.0				
0.0										

POLE ASSEMBLY
 =====

SECTION	BASEBOLTS AT BASE OF SECTION.....				CALC
NAME	ELEV	NUMBER	TYPE	DIAM	STRENGTH	BASE
	ft			in	ksi	ELEV
						ft
A	141.500	0	A325	0.00	92.0	141.500
B	92.500	0	A325	0.00	92.0	92.500
C	45.250	0	A325	0.00	92.0	45.250
D	0.000	0	A325	0.00	92.0	0.000

POLE SECTIONS
 =====

SECTION	No. of	LENGTH	OUTSIDE DIAMETER		BEND	MAT-	FLANGE ID		FLANGE WELD	
NAME	SIDES		BOT	TOP	RAD	ERIAL	BOT	TOP	..GROUP.ID..	
		ft	*	*	in	ID			BOT	TOP
			in	in						
A	18	44.50	31.84	19.29	0.625	1	0	0	0	0
B	18	53.50	45.15	30.07	0.625	2	0	0	0	0
C	18	53.50	57.71	42.63	0.625	3	0	0	0	0
D	18	53.25	69.58	54.57	0.625	4	0	0	0	0

* - Diameter of circumscribed circle

MATERIAL TYPES

=====

TYPE OF SHAPE	TYPE NO	NO OF ELEM.	ORIENT	HEIGHT	WIDTH	.THICKNESS.		IRREGULARITY	
			& deg	in	in	WEB	FLANGE	.PROJECTION. % OF AREA	ORIENT deg
PL	1	1	0.0	31.84	0.25	0.250	0.250	0.00	0.0
PL	2	1	0.0	45.15	0.38	0.375	0.375	0.00	0.0
PL	3	1	0.0	57.71	0.44	0.438	0.438	0.00	0.0
PL	4	1	0.0	69.58	0.44	0.438	0.438	0.00	0.0

& - With respect to vertical

MATERIAL PROPERTIES

=====

MATERIAL TYPE NO.	ELASTIC MODULUS ksi	UNIT WEIGHT pcf	.. STRENGTH ..		THERMAL COEFFICIENT /deg
			Fu ksi	Fy ksi	
1	29000.0	490.0	80.0	65.0	0.00001170
2	29000.0	490.0	80.0	65.0	0.00001170
3	29000.0	490.0	80.0	65.0	0.00001170
4	29000.0	490.0	80.0	65.0	0.00001170

* Only 5 condition(s) shown in full

=====

LOADING CONDITION A

112 mph wind with no ice. Wind Azimuth: 0° (1.2 D + 1.0 Wo)

LOADS ON POLE

=====

LOAD TYPE	ELEV ft	APPLY. RADIUS ft	LOAD AT AZI ft	LOAD AZI FORCES MOMENTS	
					HORIZ kip	DOWN kip	VERTICAL ft-kip	TORSNAL ft-kip
C	186.000	0.00	0.0	0.0	0.0948	0.0180	0.0000	0.0000
C	183.000	0.00	0.0	0.0	0.0207	0.0101	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.0000	1.3553	0.0000	0.0000
C	181.000	0.00	0.0	0.0	11.7840	8.4000	0.0000	0.0000
C	175.000	0.00	0.0	0.0	0.0341	0.0168	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.0000	1.2804	0.0000	0.0000
C	171.000	0.00	0.0	0.0	9.3157	5.4000	0.0000	0.0000
C	165.000	0.00	0.0	0.0	0.0337	0.0168	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.0000	1.2056	0.0000	0.0000
C	161.000	0.00	0.0	0.0	8.0491	5.4000	0.0000	0.0000
C	155.000	0.00	0.0	0.0	0.0333	0.0168	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.0000	1.1307	0.0000	0.0000
C	151.000	0.00	0.0	0.0	6.8073	5.4000	0.0000	0.0000
C	145.000	0.00	0.0	0.0	0.0328	0.0168	0.0000	0.0000
C	135.000	0.00	0.0	0.0	0.0323	0.0168	0.0000	0.0000
C	125.000	0.00	0.0	0.0	0.0318	0.0168	0.0000	0.0000
C	115.000	0.00	0.0	0.0	0.0312	0.0168	0.0000	0.0000
C	105.000	0.00	0.0	0.0	0.0306	0.0168	0.0000	0.0000
C	95.000	0.00	0.0	0.0	0.0300	0.0168	0.0000	0.0000
C	85.000	0.00	0.0	0.0	0.0293	0.0168	0.0000	0.0000
C	75.000	0.00	0.0	0.0	0.0286	0.0168	0.0000	0.0000
C	65.000	0.00	0.0	0.0	0.0277	0.0168	0.0000	0.0000
C	55.000	0.00	0.0	0.0	0.0267	0.0168	0.0000	0.0000
C	45.000	0.00	0.0	0.0	0.0256	0.0168	0.0000	0.0000
C	35.000	0.00	0.0	0.0	0.0243	0.0168	0.0000	0.0000
C	25.000	0.00	0.0	0.0	0.0227	0.0168	0.0000	0.0000
C	15.000	0.00	0.0	0.0	0.0204	0.0168	0.0000	0.0000
D	186.000	0.00	180.0	0.0	0.0500	0.0635	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0709	0.0941	0.0000	0.0000

D	146.000	0.00	180.0	0.0	0.0736	0.2441	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0736	0.2441	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0756	0.1526	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0936	0.2015	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0959	0.4506	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0959	0.4506	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0970	0.2501	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.1065	0.3026	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.1068	0.6220	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.1068	0.6220	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.1071	0.3195	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0977	0.3637	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0984	0.3725	0.0000	0.0000
D	0.000	0.00	180.0	0.0	0.1008	0.3814	0.0000	0.0000

LOADING CONDITION M

112 mph wind with no ice. Wind Azimuth: 0° (0.9 D + 1.0 Wo)

LOADS ON POLE

LOAD TYPE	ELEV ft	APPLY RADIUS ft	LOAD AT AZI	LOAD AZI	FORCES		MOMENTS	
					HORIZ kip	DOWN kip	VERTICAL ft-kip	TORSNAL ft-kip
C	186.000	0.00	0.0	0.0	0.0948	0.0135	0.0000	0.0000
C	183.000	0.00	0.0	0.0	0.0207	0.0076	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.0000	1.0165	0.0000	0.0000
C	181.000	0.00	0.0	0.0	11.7840	6.3000	0.0000	0.0000
C	175.000	0.00	0.0	0.0	0.0341	0.0126	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.0000	0.9603	0.0000	0.0000
C	171.000	0.00	0.0	0.0	9.3157	4.0500	0.0000	0.0000
C	165.000	0.00	0.0	0.0	0.0337	0.0126	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.0000	0.9042	0.0000	0.0000
C	161.000	0.00	0.0	0.0	8.0491	4.0500	0.0000	0.0000
C	155.000	0.00	0.0	0.0	0.0333	0.0126	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.0000	0.8480	0.0000	0.0000
C	151.000	0.00	0.0	0.0	6.8073	4.0500	0.0000	0.0000
C	145.000	0.00	0.0	0.0	0.0328	0.0126	0.0000	0.0000
C	135.000	0.00	0.0	0.0	0.0323	0.0126	0.0000	0.0000
C	125.000	0.00	0.0	0.0	0.0318	0.0126	0.0000	0.0000
C	115.000	0.00	0.0	0.0	0.0312	0.0126	0.0000	0.0000
C	105.000	0.00	0.0	0.0	0.0306	0.0126	0.0000	0.0000
C	95.000	0.00	0.0	0.0	0.0300	0.0126	0.0000	0.0000
C	85.000	0.00	0.0	0.0	0.0293	0.0126	0.0000	0.0000
C	75.000	0.00	0.0	0.0	0.0286	0.0126	0.0000	0.0000
C	65.000	0.00	0.0	0.0	0.0277	0.0126	0.0000	0.0000
C	55.000	0.00	0.0	0.0	0.0267	0.0126	0.0000	0.0000
C	45.000	0.00	0.0	0.0	0.0256	0.0126	0.0000	0.0000
C	35.000	0.00	0.0	0.0	0.0243	0.0126	0.0000	0.0000
C	25.000	0.00	0.0	0.0	0.0227	0.0126	0.0000	0.0000
C	15.000	0.00	0.0	0.0	0.0204	0.0126	0.0000	0.0000
D	186.000	0.00	180.0	0.0	0.0500	0.0476	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0709	0.0705	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0736	0.1831	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0736	0.1831	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0756	0.1144	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0936	0.1511	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0959	0.3379	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0959	0.3379	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0970	0.1876	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.1065	0.2269	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.1068	0.4665	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.1068	0.4665	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.1071	0.2396	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0977	0.2728	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0984	0.2794	0.0000	0.0000
D	0.000	0.00	180.0	0.0	0.1008	0.2860	0.0000	0.0000

LOADING CONDITION Y

40 mph wind with 1 ice. Wind Azimuth: 0° (1.2 D + 1.0 Di + 1.0 Wi)

LOADS ON POLE

=====

LOAD TYPE	ELEV ft	APPLY. RADIUS ft	LOAD . . . AT AZI	LOAD AZI FORCES MOMENTS	
					HORIZ kip	DOWN kip	VERTICAL ft-kip	TORSNAL ft-kip
C	186.000	0.00	0.0	0.0	0.0220	0.0656	0.0000	0.0000
C	183.000	0.00	0.0	0.0	0.0150	0.0221	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.0000	1.3553	0.0000	0.0000
C	181.000	0.00	0.0	0.0	2.2162	16.7034	0.0000	0.0000
C	175.000	0.00	0.0	0.0	0.0246	0.0288	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.0000	1.2804	0.0000	0.0000
C	171.000	0.00	0.0	0.0	1.7488	10.7078	0.0000	0.0000
C	165.000	0.00	0.0	0.0	0.0242	0.0288	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.0000	1.2056	0.0000	0.0000
C	161.000	0.00	0.0	0.0	1.2674	10.6761	0.0000	0.0000
C	155.000	0.00	0.0	0.0	0.0238	0.0288	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.0000	1.1307	0.0000	0.0000
C	151.000	0.00	0.0	0.0	1.2729	10.6426	0.0000	0.0000
C	145.000	0.00	0.0	0.0	0.0233	0.0288	0.0000	0.0000
C	135.000	0.00	0.0	0.0	0.0228	0.0288	0.0000	0.0000
C	125.000	0.00	0.0	0.0	0.0223	0.0288	0.0000	0.0000
C	115.000	0.00	0.0	0.0	0.0218	0.0288	0.0000	0.0000
C	105.000	0.00	0.0	0.0	0.0212	0.0288	0.0000	0.0000
C	95.000	0.00	0.0	0.0	0.0206	0.0288	0.0000	0.0000
C	85.000	0.00	0.0	0.0	0.0200	0.0288	0.0000	0.0000
C	75.000	0.00	0.0	0.0	0.0192	0.0288	0.0000	0.0000
C	65.000	0.00	0.0	0.0	0.0185	0.0288	0.0000	0.0000
C	55.000	0.00	0.0	0.0	0.0176	0.0288	0.0000	0.0000
C	45.000	0.00	0.0	0.0	0.0166	0.0288	0.0000	0.0000
C	35.000	0.00	0.0	0.0	0.0154	0.0288	0.0000	0.0000
C	25.000	0.00	0.0	0.0	0.0140	0.0288	0.0000	0.0000
C	15.000	0.00	0.0	0.0	0.0121	0.0288	0.0000	0.0000
D	186.000	0.00	180.0	0.0	0.0124	0.0944	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0170	0.1381	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0176	0.2899	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0176	0.2899	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0180	0.1996	0.0000	0.0000
D	135.393	0.00	180.0	0.0	0.0180	0.1996	0.0000	0.0000
D	135.393	0.00	180.0	0.0	0.0187	0.2100	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0219	0.2613	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0224	0.5123	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0224	0.5123	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0227	0.3128	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.0246	0.3747	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.0247	0.6954	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.0247	0.6954	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.0247	0.3937	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0224	0.4395	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0225	0.4460	0.0000	0.0000
D	0.000	0.00	180.0	0.0	0.0230	0.4499	0.0000	0.0000

=====

LOADING CONDITION AK

Seismic - Azimuth: 0° (1.2 D + 1.0 Ev + 1.0 Eh)

LOADS ON POLE

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LOAD TYPE	ELEV ft	APPLY. RADIUS ft	LOAD . . . AT AZI	LOAD AZI FORCES MOMENTS	
					HORIZ kip	DOWN kip	VERTICAL ft-kip	TORSNAL ft-kip
C	186.000	0.00	0.0	0.0	0.0010	0.0184	0.0000	0.0000
C	183.000	0.00	0.0	0.0	0.0006	0.0103	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.0728	1.3851	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.4511	8.5848	0.0000	0.0000
C	175.000	0.00	0.0	0.0	0.0008	0.0172	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.0614	1.3086	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.2588	5.5188	0.0000	0.0000
C	165.000	0.00	0.0	0.0	0.0007	0.0172	0.0000	0.0000
C	163.750	0.00	0.0	0.0	0.1576	3.6639	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.0512	1.2320	0.0000	0.0000

C	161.000	0.00	0.0	0.0	0.2294	5.5188	0.0000	0.0000
C	155.000	0.00	0.0	0.0	0.0007	0.0172	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.0423	1.1555	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.2018	5.5188	0.0000	0.0000
C	145.000	0.00	0.0	0.0	0.0006	0.0172	0.0000	0.0000
C	135.000	0.00	0.0	0.0	0.0005	0.0172	0.0000	0.0000
C	125.000	0.00	0.0	0.0	0.0004	0.0172	0.0000	0.0000
C	119.250	0.00	0.0	0.0	0.2216	9.7175	0.0000	0.0000
C	115.000	0.00	0.0	0.0	0.0004	0.0172	0.0000	0.0000
C	105.000	0.00	0.0	0.0	0.0003	0.0172	0.0000	0.0000
C	95.000	0.00	0.0	0.0	0.0002	0.0172	0.0000	0.0000
C	85.000	0.00	0.0	0.0	0.0002	0.0172	0.0000	0.0000
C	75.000	0.00	0.0	0.0	0.0002	0.0172	0.0000	0.0000
C	72.000	0.00	0.0	0.0	0.1259	15.1446	0.0000	0.0000
C	65.000	0.00	0.0	0.0	0.0001	0.0172	0.0000	0.0000
C	55.000	0.00	0.0	0.0	0.0001	0.0172	0.0000	0.0000
C	45.000	0.00	0.0	0.0	0.0001	0.0172	0.0000	0.0000
C	35.000	0.00	0.0	0.0	0.0000	0.0172	0.0000	0.0000
C	26.620	0.00	0.0	0.0	0.0212	18.6840	0.0000	0.0000
C	25.000	0.00	0.0	0.0	0.0000	0.0172	0.0000	0.0000
C	15.000	0.00	0.0	0.0	0.0000	0.0172	0.0000	0.0000
D	186.000	0.00	180.0	180.0	0.0000	0.0000	0.0000	0.0000
D	0.000	0.00	180.0	180.0	0.0000	0.0000	0.0000	0.0000

LOADING CONDITION AL

Seismic - Azimuth: 0° (0.9 D - 1.0 Ev + 1.0 Eh)

LOADS ON POLE

LOAD TYPE	ELEV ft	APPLY. RADIUS ft	LOAD. AZI	AT AZI	FORCES		MOMENTS	
					HORIZ kip	DOWN kip	VERTICAL ft-kip	TORSNAL ft-kip
C	186.000	0.00	0.0	0.0	0.0010	0.0131	0.0000	0.0000
C	183.000	0.00	0.0	0.0	0.0006	0.0074	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.0728	0.9867	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.4511	6.1152	0.0000	0.0000
C	175.000	0.00	0.0	0.0	0.0008	0.0122	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.0614	0.9321	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.2588	3.9312	0.0000	0.0000
C	165.000	0.00	0.0	0.0	0.0007	0.0122	0.0000	0.0000
C	163.750	0.00	0.0	0.0	0.1576	2.6098	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.0512	0.8776	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.2294	3.9312	0.0000	0.0000
C	155.000	0.00	0.0	0.0	0.0007	0.0122	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.0423	0.8231	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.2018	3.9312	0.0000	0.0000
C	145.000	0.00	0.0	0.0	0.0006	0.0122	0.0000	0.0000
C	135.000	0.00	0.0	0.0	0.0005	0.0122	0.0000	0.0000
C	125.000	0.00	0.0	0.0	0.0004	0.0122	0.0000	0.0000
C	119.250	0.00	0.0	0.0	0.2216	6.9220	0.0000	0.0000
C	115.000	0.00	0.0	0.0	0.0004	0.0122	0.0000	0.0000
C	105.000	0.00	0.0	0.0	0.0003	0.0122	0.0000	0.0000
C	95.000	0.00	0.0	0.0	0.0002	0.0122	0.0000	0.0000
C	85.000	0.00	0.0	0.0	0.0002	0.0122	0.0000	0.0000
C	75.000	0.00	0.0	0.0	0.0002	0.0122	0.0000	0.0000
C	72.000	0.00	0.0	0.0	0.1259	10.7879	0.0000	0.0000
C	65.000	0.00	0.0	0.0	0.0001	0.0122	0.0000	0.0000
C	55.000	0.00	0.0	0.0	0.0001	0.0122	0.0000	0.0000
C	45.000	0.00	0.0	0.0	0.0001	0.0122	0.0000	0.0000
C	35.000	0.00	0.0	0.0	0.0000	0.0122	0.0000	0.0000
C	26.620	0.00	0.0	0.0	0.0212	13.3091	0.0000	0.0000
C	25.000	0.00	0.0	0.0	0.0000	0.0122	0.0000	0.0000
C	15.000	0.00	0.0	0.0	0.0000	0.0122	0.0000	0.0000
D	186.000	0.00	180.0	180.0	0.0000	0.0000	0.0000	0.0000
D	0.000	0.00	180.0	180.0	0.0000	0.0000	0.0000	0.0000

Processed under license at:

Sabre Towers and Poles

on: 24 oct 2025 at: 9:44:11

187' Monopole / MD-003 Whitehall, MD

MAXIMUM POLE DEFORMATIONS CALCULATED (w.r.t. wind direction)

MAST ELEV ft	DEFLECTIONS (ft)			ROTATIONS (deg)		
	HORIZONTAL ALONG	ACROSS	DOWN	TILT ALONG	ACROSS	TWIST
186.0	17.04B	-0.09U	2.27K	10.96B	-0.04U	-0.01W
180.3	15.98B	-0.08U	2.06K	10.96B	-0.04U	-0.01W
174.6	14.93B	-0.08U	1.86K	10.88B	-0.04U	-0.01W
168.9	13.89B	-0.07U	1.67K	10.69B	-0.04U	-0.01W
163.1	12.87B	-0.07U	1.48K	10.40B	-0.04U	-0.01W
157.4	11.89B	-0.07U	1.30K	10.02B	-0.04U	-0.01W
151.7	10.94B	-0.06U	1.14K	9.55B	-0.04U	-0.01W
146.0	10.03B	-0.06U	1.00K	9.02B	-0.04U	-0.01W
141.5	9.35B	-0.05U	0.89K	8.71B	-0.04U	-0.01W
135.4	8.47B	-0.05U	0.76K	8.23B	-0.04U	-0.01W
129.3	7.63B	-0.05U	0.64K	7.75B	-0.04U	-0.01W
123.2	6.85B	-0.04U	0.54K	7.25B	-0.04U	0.00U
117.1	6.11B	-0.04U	0.45B	6.76B	-0.04U	0.00U
111.0	5.43B	-0.03U	0.37B	6.27B	-0.04U	0.00U
104.9	4.79B	-0.03U	0.31B	5.79B	-0.03U	0.00U
98.7	4.21B	-0.03U	0.25B	5.32B	-0.03U	0.00U
92.5	3.65B	-0.02U	0.20B	4.92B	-0.03U	0.00U
86.9	3.19B	-0.02U	0.16B	4.55B	-0.03U	0.00U
81.3	2.77B	-0.02U	0.13B	4.19B	-0.03U	0.00U
75.7	2.38B	-0.01U	0.10B	3.84B	-0.02U	0.00U
70.1	2.02B	-0.01U	0.08B	3.51B	-0.02U	0.00U
64.5	1.69B	-0.01U	0.06B	3.18B	-0.02U	0.00U
58.9	1.40B	-0.01U	0.05B	2.86B	-0.02U	0.00U
53.2	1.13B	-0.01U	0.03B	2.56B	-0.02U	0.00U
45.2	0.81B	-0.01U	0.02B	2.14B	-0.01U	0.00U
39.6	0.61B	0.00U	0.01B	1.84B	-0.01U	0.00U
33.9	0.44B	0.00U	0.01B	1.55B	-0.01U	0.00U
28.3	0.30B	0.00U	0.01B	1.27B	-0.01U	0.00U
22.6	0.19B	0.00U	0.00B	1.00B	-0.01U	0.00U
17.0	0.11B	0.00U	0.00B	0.74B	0.00U	0.00U
11.3	0.05B	0.00U	0.00B	0.48B	0.00U	0.00U

5.7	0.01B	0.00U	0.00Y	0.24B	0.00U	0.00U
0.0	0.00A	0.00A	0.00A	0.00A	0.00A	0.00A

MAXIMUM POLE FORCES CALCULATED(w.r.t. to wind direction)

MAST ELEV ft	TOTAL AXIAL kip	SHEAR.w.r.t. ALONG kip	WIND.DIR ACROSS kip	MOMENT.w.r.t. ALONG ft-kip	WIND.DIR ACROSS ft-kip	TORSION ft-kip
186.0	0.07 Y	0.10 K	0.01 B	-0.08 W	0.04 B	-0.01 B
180.3	18.70 Y	12.19 K	0.01 B	-10.98 K	-0.04 B	-0.01 W
174.6	18.71 AB	12.27 O	-0.04 O	-11.03 I	-0.06 Q	-0.01 W
168.9	19.33 AB	12.61 O	-0.04 O	-90.08 C	-0.20 H	-0.10 W
163.1	19.33 AG	12.62 E	0.04 E	-90.09 H	-0.23 H	-0.09 W
157.4	31.95 AG	22.25 E	0.04 E	-193.59 E	0.35 O	-0.23 W
151.7	31.95 AD	22.27 E	-0.08 W	-193.57 E	-0.29 E	-0.22 W
146.0	32.64 AD	22.64 E	-0.08 W	-335.69 E	-0.57 L	-0.45 W
141.5	32.65 AD	22.66 K	-0.09 W	-335.76 E	-0.51 L	-0.44 W
135.4	45.23 AD	31.07 K	-0.09 W	-511.59 K	-0.87 L	-0.70 W
129.3	45.23 AJ	31.03 Q	0.18 O	-511.61 K	-0.95 L	-0.69 W
123.2	45.99 AJ	31.44 Q	0.18 O	-709.10 K	-1.21 L	-0.92 W
117.1	45.99 AJ	31.51 R	-0.12 R	-709.28 K	-1.27 L	-0.92 W
111.0	58.53 AJ	38.70 R	-0.12 R	-947.23 K	-1.47 L	-1.20 W
104.9	58.53 AJ	38.73 K	-0.16 Q	-947.10 K	-1.47 L	-1.20 W
98.7	59.86 AJ	39.09 K	-0.16 Q	-1140.99 K	-2.17 L	-1.41 W
92.5	59.87 AI	39.12 K	-0.25 U	-1141.06 K	-2.07 O	-1.41 W
86.3	61.09 AI	39.58 K	-0.25 U	-1406.53 K	-3.39 L	-1.65 W
80.1	61.09 AI	39.68 B	-0.25 U	-1406.62 K	-3.40 L	-1.67 W
73.9	62.42 AI	40.19 B	-0.25 U	-1675.22 K	-4.41 L	-1.80 W
67.7	62.42 Y	40.21 K	-0.27 U	-1675.11 K	-4.36 L	-1.80 W
61.5	63.81 Y	40.74 K	-0.27 U	-1946.70 K	-5.39 L	-1.86 W
55.3	63.81 Y	40.80 K	-0.35 U	-1946.43 K	-5.35 L	-1.87 W
49.1	65.22 Y	41.31 K	-0.35 U	-2220.90 K	6.85 U	-1.97 W
42.9	65.22 Y	41.40 U	-0.47 U	-2220.76 K	6.91 U	-1.96 W
36.7	66.72 Y	41.96 U	-0.47 U	-2498.03 K	9.72 U	-2.12 W
30.5	66.71 Y	42.00 U	-0.47 U	-2497.63 K	9.81 U	-2.14 W
24.3	68.26 Y	42.58 U	-0.47 U	-2777.61 K	12.63 U	-2.19 W
18.1	68.26 Y	42.56 U	-0.52 U	-2777.29 K	12.48 U	-2.19 W
11.9	69.83 Y	43.12 U	-0.52 U	-3059.00 K	15.63 U	-2.30 W
5.7	69.83 Y	43.10 B	-0.55 U	-3058.99 K	15.50 U	-2.30 W
0.0	73.06 Y	43.73 B	-0.55 U	-3350.70 K	18.90 U	-2.39 U
	73.06 Y	43.82 B	0.35 W	-3350.55 K	18.93 U	-2.39 U
	74.84 Y	44.37 B	0.35 W	-3615.94 B	20.48 U	-2.53 U

86.9	74.84 Y	44.26 B	-0.36 U	-3615.99 B	20.24 U	-2.51 U
	76.70 Y	44.84 B	-0.36 U	-3884.09 B	22.22 U	-2.67 U
81.3	76.70 Y	45.09 B	0.42 F	-3883.74 B	22.18 U	-2.68 U
	78.57 Y	45.65 B	0.42 F	-4155.45 B	24.35 U	-2.83 U
75.7	78.57 Y	45.57 B	-0.45 U	-4155.52 B	24.42 U	-2.83 U
	80.53 Y	46.17 B	-0.45 U	-4429.29 B	26.95 U	-3.00 U
70.1	80.53 Y	46.28 B	-0.39 U	-4429.33 B	26.94 U	-3.00 U
	82.53 Y	46.89 B	-0.39 U	-4706.01 B	29.09 U	-3.13 U
64.5	82.54 Y	47.10 B	0.35 F	-4705.76 B	29.02 U	-3.11 U
	84.56 Y	47.69 B	0.35 F	-4986.36 B	30.93 U	-3.21 U
58.9	84.56 Y	47.66 B	0.36 H	-4986.09 B	30.82 U	-3.22 U
	86.67 Y	48.28 B	0.36 H	-5268.72 B	32.67 U	-3.30 U
53.2	86.67 Y	48.22 B	0.37 H	-5268.68 B	32.70 U	-3.30 U
	92.23 Y	49.07 B	0.37 H	-5675.94 B	35.27 U	-3.41 U
45.2	92.23 Y	49.02 B	0.41 H	-5675.86 B	35.25 U	-3.41 U
	94.51 Y	49.65 B	0.41 H	-5966.60 B	37.06 U	-3.47 U
39.6	94.51 Y	49.64 B	0.41 H	-5966.59 B	37.11 U	-3.47 U
	96.83 Y	50.26 B	0.41 H	-6259.39 B	39.23 U	-3.53 U
33.9	96.83 Y	50.28 B	0.36 F	-6259.46 B	39.26 U	-3.53 U
	99.16 Y	50.87 B	0.36 F	-6554.23 B	40.92 U	-3.57 U
28.3	99.16 Y	50.89 B	0.33 H	-6554.28 B	40.92 U	-3.57 U
	101.57 Y	51.48 B	0.33 H	-6851.11 B	42.70 U	-3.61 U
22.6	101.57 Y	51.49 B	-0.34 T	-6851.10 B	42.70 U	-3.61 U
	103.99 Y	52.06 B	-0.34 T	-7149.76 B	44.61 U	-3.63 U
17.0	103.99 Y	52.01 B	0.34 H	-7149.77 B	44.61 U	-3.63 U
	106.48 Y	52.59 B	0.34 H	-7449.83 B	46.48 U	-3.65 U
11.3	106.48 Y	52.60 B	0.40 H	-7449.83 B	46.50 U	-3.65 U
	109.01 Y	53.16 B	0.40 H	-7751.50 B	48.43 U	-3.67 U
5.7	109.01 Y	53.16 B	0.39 H	-7751.50 B	48.43 U	-3.67 U
	111.55 Y	53.73 B	0.39 H	-8054.65 B	50.46 U	-3.67 U

base	111.55 Y	-53.73 B	-0.39 H	8054.65 B	-50.46 U	3.67 U
reaction	-----					

COMPLIANCE WITH 4.8.2 & 4.5.4
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ELEV	AXIAL	BENDING	SHEAR +	TOTAL	SATISFIED	D/t (w/t)	MAX
ft			TORSIONAL				ALLOWED
186.00	0.00Y	0.00W	0.00K	0.00W	YES	12.17A	45.2
	0.02Y	0.02K	0.02K	0.03K	YES	13.29A	45.2
180.29	0.02AB	0.02I	0.02O	0.03K	YES	13.29A	45.2
	0.01AB	0.16C	0.02O	0.16C	YES	14.41A	45.2

174.57	0.01AG	0.16H	0.02E	0.16H	YES	14.41A	45.2
	0.02AG	0.29E	0.03E	0.30E	YES	15.52A	45.2
168.86	0.02AD	0.29E	0.03E	0.30E	YES	15.52A	45.2
	0.02AD	0.44E	0.03E	0.46E	YES	16.64A	45.2
163.14	0.02AD	0.44E	0.03K	0.46E	YES	16.64A	45.2
	0.03AD	0.61K	0.04K	0.63K	YES	17.76A	45.2
157.43	0.03AJ	0.61K	0.04Q	0.63K	YES	17.76A	45.2
	0.03AJ	0.76K	0.04Q	0.78K	YES	18.88A	45.2
151.71	0.03AJ	0.76K	0.04R	0.78K	YES	18.88A	45.2
	0.04AJ	0.93K	0.05R	0.95K	YES	20.00A	45.2
146.00	0.02AJ	0.59K	0.03K	0.61K	YES	13.22A	45.2
	0.02AJ	0.66K	0.03K	0.67K	YES	13.80A	45.2
141.50	0.02AI	0.68K	0.03K	0.69K	YES	13.57A	45.2
	0.02AI	0.75K	0.03K	0.76K	YES	14.37A	45.2
135.39	0.02AI	0.75K	0.03B	0.76K	YES	14.37A	45.2
	0.02AI	0.81K	0.03B	0.82K	YES	15.16A	45.2
129.29	0.02Y	0.81K	0.03K	0.82K	YES	15.16A	45.2
	0.02Y	0.85K	0.03K	0.86K	YES	15.96A	45.2
123.18	0.02Y	0.85K	0.03K	0.86K	YES	15.96A	45.2
	0.02Y	0.89K	0.03K	0.90K	YES	16.76A	45.2
117.07	0.02Y	0.89K	0.03U	0.90K	YES	16.76A	45.2
	0.02Y	0.93K	0.02U	0.94K	YES	17.56A	45.2
110.96	0.02Y	0.93K	0.02U	0.94K	YES	17.56A	45.2
	0.02Y	0.96K	0.02U	0.97K	YES	18.35A	45.2
104.86	0.02Y	0.96K	0.02U	0.97K	YES	18.35A	45.2
	0.02Y	0.98K	0.02U	0.99K	YES	19.15A	45.2
98.75	0.02Y	0.81K	0.02B	0.82K	YES	16.37A	45.2
	0.02Y	0.83K	0.02B	0.84K	YES	17.06A	45.2
92.50	0.02Y	0.85K	0.02B	0.86K	YES	16.76A	45.2
	0.02Y	0.87B	0.02B	0.88B	YES	17.39A	45.2
86.89	0.02Y	0.87B	0.02U	0.88B	YES	17.39A	45.2
	0.02Y	0.88B	0.02U	0.89B	YES	18.02A	45.2
81.29	0.02Y	0.88B	0.02B	0.89B	YES	18.02A	45.2
	0.02Y	0.89B	0.02B	0.90B	YES	18.65A	45.2
75.68	0.02Y	0.89B	0.02B	0.90B	YES	18.65A	45.2
	0.02Y	0.89B	0.02B	0.90B	YES	19.27A	45.2
70.07	0.02Y	0.89B	0.02B	0.90B	YES	19.27A	45.2
	0.02Y	0.90B	0.02B	0.91B	YES	19.90A	45.2
64.46	0.02Y	0.90B	0.02B	0.91B	YES	19.90A	45.2
	0.02Y	0.91B	0.02B	0.92B	YES	20.53A	45.2
58.86							

	0.02Y	0.91B	0.02B	0.92B	YES	20.53A	45.2
	0.02Y	0.91B	0.02B	0.92B	YES	21.16A	45.2
53.25	0.02Y	0.91B	0.02B	0.92B	YES	21.16A	45.2
	0.02Y	0.92B	0.02B	0.93B	YES	22.05A	45.2
45.25	0.02Y	0.94B	0.02B	0.96B	YES	21.70A	45.2
	0.02Y	0.95B	0.02B	0.96B	YES	22.33A	45.2
39.59	0.02Y	0.95B	0.02B	0.96B	YES	22.33A	45.2
	0.02Y	0.95B	0.02B	0.96B	YES	22.96A	45.2
33.94	0.02Y	0.95B	0.02B	0.96B	YES	22.96A	45.2
	0.02Y	0.95B	0.02B	0.97B	YES	23.60A	45.2
28.28	0.02Y	0.95B	0.02B	0.97B	YES	23.60A	45.2
	0.02Y	0.96B	0.02B	0.97B	YES	24.23A	45.2
22.62	0.02Y	0.96B	0.02B	0.97B	YES	24.23A	45.2
	0.02Y	0.96B	0.02B	0.97B	YES	24.86A	45.2
16.97	0.02Y	0.96B	0.02B	0.97B	YES	24.86A	45.2
	0.02Y	0.96B	0.02B	0.97B	YES	25.50A	45.2
11.31	0.02Y	0.96B	0.02B	0.97B	YES	25.50A	45.2
	0.02Y	0.96B	0.02B	0.98B	YES	26.13A	45.2
5.66	0.02Y	0.96B	0.02B	0.98B	YES	26.13A	45.2
	0.02Y	0.97B	0.02B	0.98B	YES	26.76A	45.2
0.00							

MAXIMUM LOADS ONTO FOUNDATION (w.r.t. wind direction)

DOWN	SHEAR w.r.t. WIND.DIR		MOMENT w.r.t. WIND.DIR		TORSION
kip	ALONG	ACROSS	ALONG	ACROSS	ft-kip
	kip	kip	ft-kip	ft-kip	
111.55	53.73	0.39	-8054.65	50.46	-3.67
Y	B	H	B	U	U

(USA 222-H) - Monopole Spatial Analysis (c)2017 Guymast Inc.

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Sabre Towers and Poles on: 24 oct 2025 at: 9:44:39

187' Monopole / MD-003 Whitehall, MD

 ***** Service Load Condition *****

* Only 1 condition(s) shown in full

LOADING CONDITION A =====

60 mph wind with no ice. Wind Azimuth: 0° (1.0 D + 1.0 Wo)

LOADS ON POLE

=====

LOAD TYPE	ELEV ft	APPLY. RADIUS ft	LOAD AT AZI	LOAD AZI FORCES MOMENTS	
					HORIZ kip	DOWN kip	VERTICAL ft-kip	TORSNAL ft-kip
C	186.000	0.00	0.0	0.0	0.0243	0.0150	0.0000	0.0000
C	183.000	0.00	0.0	0.0	0.0053	0.0084	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.0000	1.1294	0.0000	0.0000
C	181.000	0.00	0.0	0.0	3.0259	7.0000	0.0000	0.0000
C	175.000	0.00	0.0	0.0	0.0088	0.0140	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.0000	1.0670	0.0000	0.0000
C	171.000	0.00	0.0	0.0	2.3921	4.5000	0.0000	0.0000
C	165.000	0.00	0.0	0.0	0.0087	0.0140	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.0000	1.0046	0.0000	0.0000
C	161.000	0.00	0.0	0.0	2.0668	4.5000	0.0000	0.0000
C	155.000	0.00	0.0	0.0	0.0085	0.0140	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.0000	0.9422	0.0000	0.0000
C	151.000	0.00	0.0	0.0	1.7480	4.5000	0.0000	0.0000
C	145.000	0.00	0.0	0.0	0.0084	0.0140	0.0000	0.0000
C	135.000	0.00	0.0	0.0	0.0083	0.0140	0.0000	0.0000
C	125.000	0.00	0.0	0.0	0.0082	0.0140	0.0000	0.0000
C	115.000	0.00	0.0	0.0	0.0080	0.0140	0.0000	0.0000
C	105.000	0.00	0.0	0.0	0.0079	0.0140	0.0000	0.0000
C	95.000	0.00	0.0	0.0	0.0077	0.0140	0.0000	0.0000
C	85.000	0.00	0.0	0.0	0.0075	0.0140	0.0000	0.0000
C	75.000	0.00	0.0	0.0	0.0073	0.0140	0.0000	0.0000
C	65.000	0.00	0.0	0.0	0.0071	0.0140	0.0000	0.0000
C	55.000	0.00	0.0	0.0	0.0069	0.0140	0.0000	0.0000
C	45.000	0.00	0.0	0.0	0.0066	0.0140	0.0000	0.0000
C	35.000	0.00	0.0	0.0	0.0062	0.0140	0.0000	0.0000
C	25.000	0.00	0.0	0.0	0.0058	0.0140	0.0000	0.0000
C	15.000	0.00	0.0	0.0	0.0052	0.0140	0.0000	0.0000
D	186.000	0.00	180.0	0.0	0.0128	0.0529	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0182	0.0784	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0189	0.2034	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0189	0.2034	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0194	0.1271	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0240	0.1679	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0246	0.3755	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0246	0.3755	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0249	0.2084	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.0273	0.2522	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.0274	0.5184	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.0274	0.5184	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.0275	0.2662	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0251	0.3031	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0253	0.3105	0.0000	0.0000
D	0.000	0.00	180.0	0.0	0.0259	0.3178	0.0000	0.0000

=====

MAXIMUM POLE DEFORMATIONS CALCULATED (w.r.t. wind direction)

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MAST ELEV ft DEFLECTIONS (ft) ROTATIONS (deg)		
	HORIZONTAL ALONG	ACROSS	DOWN	TILT ALONG	ACROSS	TWIST
186.0	4.47I	-0.02I	0.16I	2.83I	-0.01I	0.00K
180.3	4.18I	-0.02I	0.15I	2.83I	-0.01I	0.00K
174.6	3.90I	-0.02I	0.13I	2.81I	-0.01I	0.00K
168.9	3.62I	-0.02I	0.12I	2.76I	-0.01I	0.00K
163.1	3.35I	-0.01I	0.10I	2.69I	-0.01I	0.00K
157.4	3.09I	-0.01I	0.09I	2.59I	-0.01I	0.00K
151.7	2.84I	-0.01I	0.08I	2.46I	-0.01I	0.00K
146.0	2.60I	-0.01I	0.07I	2.32I	-0.01I	0.00K
141.5	2.42I	-0.01I	0.06I	2.24I	-0.01I	0.00K

135.4	2.19I	-0.01I	0.05I	2.12I	-0.01I	0.00K
129.3	1.97I	-0.01I	0.05I	1.99I	-0.01I	0.00K
123.2	1.77I	-0.01I	0.04I	1.86I	-0.01I	0.00K
117.1	1.57I	-0.01I	0.03I	1.73I	-0.01I	0.00K
111.0	1.40I	-0.01I	0.03I	1.61I	-0.01I	0.00K
104.9	1.23I	-0.01I	0.02I	1.48I	-0.01I	0.00K
98.7	1.08I	-0.01I	0.02I	1.36I	-0.01I	0.00K
92.5	0.94I	-0.01I	0.02I	1.26I	-0.01I	0.00K
86.9	0.82I	0.00I	0.01I	1.16I	-0.01I	0.00K
81.3	0.71I	0.00I	0.01I	1.07I	-0.01I	0.00K
75.7	0.61I	0.00I	0.01I	0.98I	-0.01I	0.00K
70.1	0.52I	0.00I	0.01I	0.90I	0.00I	0.00K
64.5	0.43I	0.00I	0.01I	0.81I	0.00I	0.00K
58.9	0.36I	0.00I	0.00I	0.73I	0.00I	0.00K
53.2	0.29I	0.00I	0.00I	0.65I	0.00I	0.00K
45.2	0.21I	0.00E	0.00I	0.55I	0.00I	0.00K
39.6	0.16I	0.00E	0.00I	0.47I	0.00I	0.00K
33.9	0.11I	0.00E	0.00I	0.40I	0.00E	0.00K
28.3	0.08I	0.00E	0.00I	0.32I	0.00E	0.00K
22.6	0.05I	0.00E	0.00I	0.26I	0.00E	0.00K
17.0	0.03I	0.00E	0.00I	0.19I	0.00E	0.00K
11.3	0.01I	0.00E	0.00I	0.12I	0.00E	0.00K
5.7	0.00I	0.00E	0.00I	0.06I	0.00E	0.00K
0.0	0.00A	0.00A	0.00A	0.00A	0.00A	0.00A

MAXIMUM POLE FORCES CALCULATED(w.r.t. to wind direction)

MAST ELEV ft	TOTAL AXIAL kip	SHEAR w.r.t. WIND DIR ALONG kip	WIND DIR ACROSS kip	MOMENT w.r.t. WIND DIR ALONG ft-kip	WIND DIR ACROSS ft-kip	TORSION ft-kip
186.0	0.02 B	0.03 I	0.00 L	0.02 I	-0.01 L	0.00 L
180.3	8.47 B	3.14 I	0.00 L	-2.88 I	0.01 L	0.00 L
174.6	8.47 A	3.14 A	-0.01 F	-2.87 I	0.02 L	0.00 B
168.9	8.81 A	3.22 A	-0.01 F	-23.38 A	-0.03 B	0.00 I
163.1	8.81 A	3.23 L	-0.01 H	-23.38 A	-0.04 B	0.00 I
157.4	14.74 A	5.71 L	-0.01 H	-50.17 A	0.07 E	0.01 I
151.7	14.74 A	5.70 F	-0.01 E	-50.16 A	0.08 E	0.01 I
146.0	15.13 A	5.80 F	-0.01 E	-86.98 A	0.17 E	0.01 I
140.3	15.13 A	5.82 A	-0.01 C	-86.99 A	0.18 E	0.01 I
134.6	21.03 A	7.98 A	-0.01 C	-132.68 A	0.25 E	0.02 I
128.9	21.04 F	7.98 E	-0.02 E	-132.67 A	0.25 E	0.02 I

151.7	21.47 F	8.08 E	-0.02 E	-183.81 A	0.35 E	0.03 I
	21.47 B	8.08 A	-0.02 L	-183.82 A	0.34 E	0.03 I
146.0	27.35 B	9.93 A	-0.02 L	-245.18 A	0.46 E	0.04 I
	27.35 I	9.95 L	-0.06 K	-245.30 A	0.43 E	0.04 I
141.5	28.28 I	10.05 L	-0.06 K	-294.75 A	0.55 E	0.04 I
	28.28 I	10.03 I	0.03 H	-294.87 A	0.54 E	0.04 I
135.4	29.07 I	10.15 I	0.03 H	-362.83 A	0.64 E	0.04 I
	29.08 I	10.17 B	-0.04 K	-362.86 A	0.65 E	0.04 I
129.3	29.92 I	10.30 B	-0.04 K	-431.65 A	0.80 K	-0.05 K
	29.93 I	10.31 B	-0.04 K	-431.67 A	0.80 K	-0.05 K
123.2	30.80 I	10.44 B	-0.04 K	-501.08 A	1.04 K	-0.06 K
	30.80 I	10.46 A	-0.06 I	-501.12 A	1.03 K	-0.06 K
117.1	31.70 I	10.59 A	-0.06 I	-571.32 A	1.32 K	-0.07 K
	31.70 I	10.58 A	-0.05 I	-571.32 A	1.34 K	-0.07 K
111.0	32.65 I	10.73 A	-0.05 I	-642.01 A	1.54 K	-0.08 K
	32.65 I	10.79 I	-0.08 I	-642.07 A	1.54 K	-0.08 K
104.9	33.64 I	10.94 I	-0.08 I	-713.36 I	1.99 I	-0.08 K
	33.64 I	10.91 I	-0.09 I	-713.38 I	1.96 I	-0.08 K
98.7	34.64 I	11.05 I	-0.09 I	-785.53 I	2.52 I	-0.09 K
	34.64 I	11.12 I	-0.12 I	-785.54 I	2.50 I	-0.09 K
92.5	37.00 I	11.28 I	-0.12 I	-860.59 I	3.27 I	-0.09 K
	37.01 I	11.24 I	-0.11 I	-860.54 I	3.28 I	-0.09 K
86.9	38.19 I	11.38 I	-0.11 I	-928.36 I	3.90 I	-0.09 K
	38.19 I	11.38 I	-0.09 I	-928.35 I	3.88 I	-0.09 K
81.3	39.43 I	11.53 I	-0.09 I	-996.78 I	4.41 I	-0.10 K
	39.43 I	11.53 L	-0.08 E	-996.83 I	4.41 I	-0.10 K
75.7	40.68 I	11.67 L	-0.08 E	-1065.42 I	4.82 I	-0.10 K
	40.68 I	11.65 I	-0.09 E	-1065.39 I	4.81 I	-0.10 K
70.1	41.99 I	11.80 I	-0.09 E	-1134.90 I	5.30 I	-0.10 K
	41.99 I	11.81 L	-0.09 E	-1134.84 I	5.29 I	-0.10 K
64.5	43.33 I	11.97 L	-0.09 E	-1204.99 I	5.75 I	-0.10 K
	43.33 I	11.97 I	-0.14 E	-1204.94 I	5.77 I	-0.10 K
58.9	44.69 I	12.12 I	-0.14 E	-1275.82 I	6.37 I	-0.11 K
	44.69 I	12.13 I	-0.13 E	-1275.80 I	6.35 I	-0.10 K
53.2	46.10 I	12.29 I	-0.13 E	-1347.32 I	7.03 I	-0.10 K
	46.10 I	12.29 I	-0.15 E	-1347.29 I	7.04 I	-0.10 K
45.2	50.25 I	12.51 I	-0.15 E	-1450.55 I	7.90 I	-0.10 K
	50.25 I	12.52 I	-0.14 E	-1450.55 I	7.91 I	-0.10 K
39.6	51.78 I	12.69 I	-0.14 E	-1524.45 I	8.55 I	-0.10 K
	51.78 I	12.68 I	-0.13 E	-1524.44 I	8.55 I	-0.10 K
	53.36 I	12.84 I	-0.13 E	-1598.87 I	9.26 I	-0.10 K

33.9	53.36 I	12.82 I	-0.15 E	-1598.89 I	9.26 I	-0.10 K
28.3	54.95 I	12.97 I	-0.15 E	-1673.79 I	9.77 I	-0.11 K
	54.95 I	12.99 I	-0.14 E	-1673.78 I	9.78 I	-0.11 K
22.6	56.59 I	13.14 I	-0.14 E	-1749.28 I	10.27 I	-0.11 K
	56.59 I	13.14 I	-0.14 E	-1749.27 I	10.26 I	-0.11 K
17.0	58.25 I	13.28 I	-0.14 E	-1825.27 I	10.79 E	-0.11 K
	58.25 I	13.29 I	-0.15 E	-1825.26 I	10.79 E	-0.11 K
11.3	59.96 I	13.43 I	-0.15 E	-1901.76 I	11.65 E	-0.11 K
	59.96 I	13.45 I	-0.15 E	-1901.76 I	11.65 E	-0.11 K
5.7	61.73 I	13.59 I	-0.15 E	-1978.76 I	12.51 E	-0.11 K
	61.73 I	13.59 I	-0.15 E	-1978.76 I	12.51 E	-0.11 K
	63.52 I	13.73 I	-0.15 E	-2056.21 I	13.37 E	-0.11 K
base reaction	63.52 I	-13.73 I	0.15 E	2056.21 I	-13.37 E	0.11 K

COMPLIANCE WITH 4.8.2 & 4.5.4

ELEV ft	AXIAL	BENDING	SHEAR + TORSIONAL	TOTAL	SATISFIED	D/t (w/t)	MAX ALLOWED
186.00	0.00B	0.00I	0.00I	0.00I	YES	12.17A	45.2
180.29	0.01B	0.01I	0.01I	0.01I	YES	13.29A	45.2
	0.01A	0.01I	0.01A	0.01I	YES	13.29A	45.2
174.57	0.01A	0.04A	0.00A	0.05A	YES	14.41A	45.2
	0.01A	0.04A	0.00L	0.05A	YES	14.41A	45.2
168.86	0.01A	0.08A	0.01L	0.09A	YES	15.52A	45.2
	0.01A	0.08A	0.01F	0.09A	YES	15.52A	45.2
163.14	0.01A	0.12A	0.01F	0.13A	YES	16.64A	45.2
	0.01A	0.12A	0.01A	0.13A	YES	16.64A	45.2
157.43	0.01A	0.16A	0.01A	0.17A	YES	17.76A	45.2
	0.01F	0.16A	0.01E	0.17A	YES	17.76A	45.2
151.71	0.01F	0.20A	0.01E	0.21A	YES	18.88A	45.2
	0.01B	0.20A	0.01A	0.21A	YES	18.88A	45.2
146.00	0.02B	0.24A	0.01A	0.26A	YES	20.00A	45.2
	0.01I	0.15A	0.01L	0.16A	YES	13.22A	45.2
141.50	0.01I	0.17A	0.01L	0.18A	YES	13.80A	45.2
	0.01I	0.18A	0.01I	0.19A	YES	13.57A	45.2
135.39	0.01I	0.19A	0.01I	0.20A	YES	14.37A	45.2
	0.01I	0.19A	0.01B	0.20A	YES	14.37A	45.2
129.29	0.01I	0.21A	0.01B	0.22A	YES	15.16A	45.2
	0.01I	0.21A	0.01B	0.22A	YES	15.16A	45.2
	0.01I	0.22A	0.01B	0.23B	YES	15.96A	45.2

123.18	0.01I	0.22A	0.01A	0.23A	YES	15.96A	45.2
	0.01I	0.23A	0.01A	0.24A	YES	16.76A	45.2
117.07	0.01I	0.23A	0.01A	0.24A	YES	16.76A	45.2
	0.01I	0.24A	0.01A	0.25A	YES	17.56A	45.2
110.96	0.01I	0.24A	0.01I	0.25A	YES	17.56A	45.2
	0.01I	0.25I	0.01I	0.26I	YES	18.35A	45.2
104.86	0.01I	0.25I	0.01I	0.26I	YES	18.35A	45.2
	0.01I	0.25I	0.01I	0.26I	YES	19.15A	45.2
98.75	0.01I	0.21I	0.01I	0.22I	YES	16.37A	45.2
	0.01I	0.21I	0.01I	0.22I	YES	17.06A	45.2
92.50	0.01I	0.22I	0.01I	0.23I	YES	16.76A	45.2
	0.01I	0.22I	0.01I	0.23I	YES	17.39A	45.2
86.89	0.01I	0.22I	0.01I	0.23I	YES	17.39A	45.2
	0.01I	0.22I	0.00I	0.23I	YES	18.02A	45.2
81.29	0.01I	0.22I	0.00L	0.23I	YES	18.02A	45.2
	0.01I	0.23I	0.00L	0.24I	YES	18.65A	45.2
75.68	0.01I	0.23I	0.00I	0.24I	YES	18.65A	45.2
	0.01I	0.23I	0.00I	0.24I	YES	19.27A	45.2
70.07	0.01I	0.23I	0.00L	0.24I	YES	19.27A	45.2
	0.01I	0.23I	0.00L	0.24I	YES	19.90A	45.2
64.46	0.01I	0.23I	0.00I	0.24I	YES	19.90A	45.2
	0.01I	0.23I	0.00I	0.24I	YES	20.53A	45.2
58.86	0.01I	0.23I	0.00I	0.24I	YES	20.53A	45.2
	0.01I	0.23I	0.00I	0.24I	YES	21.16A	45.2
53.25	0.01I	0.23I	0.00I	0.24I	YES	21.16A	45.2
	0.01I	0.24I	0.00I	0.24I	YES	22.05A	45.2
45.25	0.01I	0.24I	0.00I	0.25I	YES	21.70A	45.2
	0.01I	0.24I	0.00I	0.25I	YES	22.33A	45.2
39.59	0.01I	0.24I	0.00I	0.25I	YES	22.33A	45.2
	0.01I	0.24I	0.00I	0.25I	YES	22.96A	45.2
33.94	0.01I	0.24I	0.00I	0.25I	YES	22.96A	45.2
	0.01I	0.24I	0.00I	0.25I	YES	23.60A	45.2
28.28	0.01I	0.24I	0.00I	0.25I	YES	23.60A	45.2
	0.01I	0.24I	0.00I	0.25I	YES	24.23A	45.2
22.62	0.01I	0.24I	0.00I	0.25I	YES	24.23A	45.2
	0.01I	0.24I	0.00I	0.25I	YES	24.86A	45.2
16.97	0.01I	0.24I	0.00I	0.25I	YES	24.86A	45.2
	0.01I	0.25I	0.00I	0.26I	YES	25.50A	45.2
11.31	0.01I	0.25I	0.00I	0.26I	YES	25.50A	45.2
	0.01I	0.25I	0.00I	0.26I	YES	26.13A	45.2
5.66							

	0.01I	0.25I	0.00I	0.26I	YES	26.13A	45.2
	0.01I	0.25I	0.00I	0.26I	YES	26.76A	45.2
0.00						

MAXIMUM LOADS ONTO FOUNDATION (w.r.t. wind direction)

=====

DOWN	SHEAR.w.r.t.WIND.DIR		MOMENT.w.r.t.WIND.DIR		TORSION
	ALONG	ACROSS	ALONG	ACROSS	
kip	kip	kip	ft-kip	ft-kip	ft-kip
63.52	13.73	-0.15	-2056.21	13.37	-0.11
I	I	E	I	E	K

=====



SO#: 26-2535-TLJ
 Site Name: MD-003 Whitehall, MD
 Date: 10/24/2025

Round Flange Plate and Bolts per ANSI/TIA 222-H
Elevation = 186 feet

Pole Data

Diameter: 19 in
 Thickness: 0.25 in
 Yield (Fy): 65 ksi
 # of Sides: 18 "0" IF Round
 Strength (Fu): 80 ksi

Reactions

Moment, Mu: 424 ft-kips
 Axial, Pu: 0 kips
 Shear, Vu: 0 kips

Bolt Data

Quantity: 18
 Diameter: 1 in
 Bolt Material: A325
 Strength (Fu): 120 ksi
 Yield (Fy): 92 ksi
 BC Diam. (in): 22.25 BC Override:

Flange Bolt Results

Allowable Φ *Rnt: 54.54 kips
 Adjusted Φ *Rnt (due to shear): 54.54 kips
 Maximum Bolt Tension: 50.82 kips
 Bolt Interaction Ratio: **93.2% Pass**

Plate Data

Diameter (in): 24.75 Dia. Override:
 Thickness: 1.5 in
 Center Hole Diam.: 13 in
 Yield (Fy): 50 ksi
 Single-Rod B-eff: 3.35 in
 Drain Hole: 1 in. diameter
 Drain Location: 8.5 in. center of pole to center of drain hole

Flange Plate Results

Compression Side Plate (Mu/Z): 26.7 ksi
 Allowable Φ *Fy: 45.0 ksi
 Compr. Plate Interaction Ratio: **59.3% Pass**



SO#: 26-2535-TLJ
 Site Name: MD-003 Whitehall, MD
 Date: 10/24/2025

Round Base Plate and Anchor Rods, per ANSI/TIA 222-H

Pole Data

Diameter: 68.530 in (flat to flat)
 Thickness: 0.4375 in
 Yield (Fy): 65 ksi
 # of Sides: 18 "0" IF Round
 Strength (Fu): 80 ksi

Reactions

Moment, Mu: 8054.65 ft-kips
 Axial, Pu: 76.12 kips
 Shear, Vu: 53.73 kips

Anchor Rod Data

Quantity: 22
 Diameter: 2.25 in
 Rod Material: A615
 Strength (Fu): 100 ksi
 Yield (Fy): 75 ksi
 BC Diam. (in): 75.75 BC Override:

Plate Data

Diameter (in): 81.5 Dia. Override:
 Thickness: 2.25 in
 Yield (Fy): 50 ksi
 Eff Width/Rod: 9.89 in
 Drain Hole: 2.625 in. diameter
 Drain Location: 32.25 in. center of pole to center of drain hole
 Center Hole: 56.5 in. diameter

Anchor Rod Results

(per 4.9.9)

Maximum Put: 229.40 Kips
 $\Phi^t \cdot R_{nt}$: 243.75 Kips
 Vu: 2.44 Kips
 $\Phi^v \cdot R_{nv}$: 149.10 Kips
 Tension Interaction Ratio: 0.89
 Maximum Puc: 235.46 Kips
 $\Phi^c \cdot R_{nc}$: 268.39 Kips
 Vu: 2.44 Kips
 $\Phi^c \cdot R_{ncv}$: 120.77 Kips
 Compression Interaction Ratio: 0.88
 Maximum Interaction Ratio: **88.6% Pass**

Base Plate Results

Base Plate (Mu/Z): 43.8 ksi
 Allowable $\Phi^t \cdot F_y$: 45.0 ksi (per AISC)
 Base Plate Interaction Ratio: **97.3% Pass**

MAT FOUNDATION DESIGN BY SABRE INDUSTRIES

187' Monopole ARCOLA TOWERS MD-003 Whitehall, MD (26-2535-TLJ) 10/24/25 TTW

Overall Loads:

Factored Moment (ft-kips)	8054.65
Factored Axial (kips)	76.12
Factored Shear (kips)	53.73
Bearing Design Strength (ksf)	3.75
Water Table Below Grade (ft)	999
Width of Mat (ft)	31
Thickness of Mat (ft)	2
Depth to Bottom of Slab (ft)	6
Quantity of Bolts in Bolt Circle	22
Bolt Circle Diameter (in)	75.75
Effective Anchor	
Bolt Embedment (in)	66.5
Diameter of Pier (ft)	8
Ht. of Pier Above Ground (ft)	0.5
Ht. of Pier Below Ground (ft)	4
Quantity of Bars in Mat	53
Bar Diameter in Mat (in)	1.27
Area of Bars in Mat (in ²)	67.14
Spacing of Bars in Mat (in)	7.01
Quantity of Bars Pier	54
Bar Diameter in Pier (in)	1.128
Tie Bar Diameter in Pier (in)	0.625
Spacing of Ties (in)	4
Area of Bars in Pier (in ²)	53.96
Spacing of Bars in Pier (in)	5.10
f'c (ksi)	4.5
fy (ksi)	60
Unit Wt. of Soil (kcf)	0.11
Unit Wt. of Concrete (kcf)	0.15

Max. Net Bearing Press. (ksf)	3.29
Allowable Bearing Pressure (ksf)	2.50
Safety Factor	2.00
Ultimate Bearing Pressure (ksf)	5.00
Bearing Φs	0.75

Minimum Pier Diameter (ft)	8.00
Equivalent Square b (ft)	7.09
Square Pier? (Y/N)	N

Recommended Spacing (in)	5 to 12
--------------------------	---------

Minimum Pier A _s (in ²)	36.19
Recommended Spacing (in)	5 to 12

Volume of Concrete (yd³) 79.56

Two-Way Shear Action:

Average d (in)	19.73
φv _c (ksi)	0.195
φv _c = φ(2 + 4/β _c)f' _c ^{1/2}	0.302
φv _c = φ(α _s d/b _o +2)f' _c ^{1/2}	0.195
φv _c = φ4f' _c ^{1/2}	0.201
Shear perimeter, b _o (in)	419.23
β _c	1

v _u (ksi)	0.146
----------------------	-------

J (in ³)	1.528E+07
c + d (in)	104.81
0.40M _{sc} (ft-kips)	3318.6

One-Way Shear:

φV _c (kips)	738.5
------------------------	-------

V _u (kips)	482.2
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Stability:

Overtuning Design Strength (ft-k)	10970.1
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Total Applied M (ft-k)	8403.9
------------------------	--------

Pier-Slab Transfer by Flexure:

b_{slab} (ft)	14.00		
ϕM_n (ft-kips)	4997.8	$0.60M_{sc}$ (ft-kips)	4977.9

Pier Design:

ϕV_n (kips)	1275.9	V_u (kips)	53.7
$\phi V_c = \phi 2(1 + N_u / (2000 A_g)) f'_c{}^{1/2} b_w d$	745.8		
V_s (kips)	706.9	*** $V_s \max = 4 f'_c{}^{1/2} b_w d$ (kips)	1978.3
Maximum Spacing (in)	7.62	(Only if Shear Ties are Required)	
Actual Hook Development (in)	18.46	Req'd Hook Development l_{dh} (in) - Tension	14.12
		Req'd Hook Development l_{dc} (in) - Compression	15.23

Flexure in Slab:

ϕM_n (ft-kips)	5533.2	M_u (ft-kips)	3974.7
a (in)	2.83		
Steel Ratio	0.00915		
β_1	0.825		
Maximum Steel Ratio (ρ_t)	0.0197		
Minimum Steel Ratio	0.0018		
Rebar Development in Pad (in)	135.00	Required Development in Pad (in)	34.08

Condition	1 is OK, 0 Fails
Maximum Soil Bearing Pressure	1
Pier Area of Steel	1
Pier Shear	1
Interaction Diagram	1
Two-Way Shear Action	1
One-Way Shear Action	1
Overturning	1
Flexure	1
Steel Ratio	1
Length of Development in Pad	1
Hook Development	1
Anchor Bolt Pullout	1
Anchor Bolt Punching Shear	1

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LFile for Windows, Version 2019-11.009

Analysis of Individual Files and Drilled Shafts
Subjected to Lateral Loading Using the p-y Method
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Files Used for Analysis

Path to file locations:
\Program Files (x86)\Ensoft\Lpile2019\files\

Name of input data file:
26-2535-TLJ.lp11d

Name of output report file:
26-2535-TLJ.lp11o

Name of plot output file:
26-2535-TLJ.lp11p

Name of runtime message file:
26-2535-TLJ.lp11r

Date and Time of Analysis

Date: October 24, 2025 Time: 9:58:50

Problem Title

Site : MD-003 Whitehall, MD

Tower : 187' Monopole

Prepared for : ARCOLA TOWERS

Job Number : 26-2535-TLJ

Engineer : TTW

Program Options and Settings

Computational Options:

- Conventional Analysis
- Engineering Units Used for Data Input and Computations:
- US Customary System Units (pounds, feet, inches)

Analysis Control Options:

- Maximum number of iterations allowed = 999
- Deflection tolerance for convergence = 1.0000E-05 in
- Maximum allowable deflection = 100.0000 in
- Number of pile increments = 100

Loading Type and Number of Cycles of Loading:

- Static loading specified
- Use of p-y modification factors for p-y curves not selected
- Analysis uses layering correction (Method of Georgiadis)
- No distributed lateral loads are entered
- Loading by lateral soil movements acting on pile not selected
- Input of shear resistance at the pile tip not selected
- Input of moment resistance at the pile tip not selected
- Input of side resistance moment along pile not selected
- Computation of pile-head foundation stiffness matrix not selected
- Push-over analysis of pile not selected
- Buckling analysis of pile not selected

Output Options:

- Output files use decimal points to denote decimal symbols.
- Report only summary tables of pile-head deflection, maximum bending moment, and maximum shear force in output report file.
- No p-y curves to be computed and reported for user-specified depths
- Print using wide report formats

Pile Structural Properties and Geometry

- Number of pile sections defined = 1
- Total length of pile = 40.500 ft
- Depth of ground surface below top of pile = 0.5000 ft

Pile diameters used for p-y curve computations are defined using 2 points.

p-y curves are computed using pile diameter values interpolated with depth over the length of the pile. A summary of values of pile diameter vs. depth follows.

Point No.	Depth Below Pile Head feet	Pile Diameter inches
1	0.000	96.0000
2	40.500	96.0000

Input Structural Properties for Pile Sections:

Pile Section No. 1:

- Section 1 is a round drilled shaft, bored pile, or CIDH pile
- Length of section = 40.500000 ft
- Shaft Diameter = 96.000000 in
- Shear capacity of section = 0.0000 lbs

Ground Slope and Pile Batter Angles

- Ground Slope Angle = 0.000 degrees
- = 0.000 radians
- Pile Batter Angle = 0.000 degrees
- = 0.000 radians

Soil and Rock Layering Information

The soil profile is modelled using 1 layers

Layer 1 is stiff clay without free water

Distance from top of pile to top of layer = 0.500000 ft
Distance from top of pile to bottom of layer = 60.500000 ft
Effective unit weight at top of layer = 110.000000 pcf
Effective unit weight at bottom of layer = 110.000000 pcf
Undrained cohesion at top of layer = 1000.000000 psf
Undrained cohesion at bottom of layer = 1000.000000 psf
Epsilon-50 at top of layer = 0.010000
Epsilon-50 at bottom of layer = 0.010000

(Depth of the lowest soil layer extends 20.000 ft below the pile tip)

Summary of Input Soil Properties

Layer Num.	Soil Type Name (p-y Curve Type)	Layer Depth ft	Effective Unit Wt. pcf	Cohesion psf	E50 or krm
1	Stiff Clay w/o Free Water	0.5000 60.5000	110.0000 110.0000	1000.0000 1000.0000	0.01000 0.01000

Static Loading Type

Static loading criteria were used when computing p-y curves for all analyses.

Pile-head Loading and Pile-head Fixity Conditions

Number of loads specified = 2

Load Analysis No.	Load Type	Condition 1	Condition 2	Axial Thrust Force, lbs	Compute Top y vs. Pile Length	Run
1	1	V = 71640. lbs	M = 128874400. in-lbs	101493.	No	
2	1	V = 13730. lbs	M = 24674520. in-lbs	63520.	No	

V = shear force applied normal to pile axis
M = bending moment applied to pile head
y = lateral deflection normal to pile axis
S = pile slope relative to original pile batter angle
R = rotational stiffness applied to pile head
Values of top y vs. pile lengths can be computed only for load types with specified shear loading (Load Types 1, 2, and 3).
Thrust force is assumed to be acting axially for all pile batter angles.

Computations of Nominal Moment Capacity and Nonlinear Bending Stiffness

Axial thrust force values were determined from pile-head loading conditions

Number of Pile Sections Analyzed = 1

File Section No. 1:

Dimensions and Properties of Drilled Shaft (Bored Pile):

Length of Section = 40.500000 ft
 Shaft Diameter = 96.000000 in
 Concrete Cover Thickness (to edge of long. rebar) = 3.625000 in
 Number of Reinforcing Bars = 54 bars
 Yield Stress of Reinforcing Bars = 60000. psi
 Modulus of Elasticity of Reinforcing Bars = 29000000. psi
 Gross Area of Shaft = 7238. sq. in.
 Total Area of Reinforcing Steel = 53.963715 sq. in.
 Area Ratio of Steel Reinforcement = 0.75 percent
 Edge-to-Edge Bar Spacing = 3.966766 in
 Maximum Concrete Aggregate Size = 0.750000 in
 Ratio of Bar Spacing to Aggregate Size = 5.29
 Offset of Center of Rebar Cage from Center of Pile = 0.0000 in

Axial Structural Capacities:

 Nom. Axial Structural Capacity = $0.85 F_c A_c + F_y A_s$ = 30717.639 kips
 Tensile Load for Cracking of Concrete = -3343.589 kips
 Nominal Axial Tensile Capacity = -3237.823 kips

Reinforcing Bar Dimensions and Positions Used in Computations:

Bar Number	Bar Diam. inches	Bar Area sq. in.	X inches	Y inches
1	1.128000	0.999328	43.811000	0.000000
2	1.128000	0.999328	43.514766	5.086147
3	1.128000	0.999328	42.630069	10.103512
4	1.128000	0.999328	41.168873	14.984244
5	1.128000	0.999328	39.150940	19.662341
6	1.128000	0.999328	36.603557	24.074538
7	1.128000	0.999328	33.561173	28.161168
8	1.128000	0.999328	30.064932	31.866967
9	1.128000	0.999328	26.162115	35.141819
10	1.128000	0.999328	21.905500	37.941439
11	1.128000	0.999328	17.352651	40.227966
12	1.128000	0.999328	12.565136	41.970479
13	1.128000	0.999328	7.607700	43.145412
14	1.128000	0.999328	2.547383	43.736879
15	1.128000	0.999328	-2.547383	43.736879
16	1.128000	0.999328	-7.607700	43.145412
17	1.128000	0.999328	-12.565136	41.970479
18	1.128000	0.999328	-17.352651	40.227966
19	1.128000	0.999328	-21.905500	37.941439
20	1.128000	0.999328	-26.162115	35.141819
21	1.128000	0.999328	-30.064932	31.866967
22	1.128000	0.999328	-33.561173	28.161168
23	1.128000	0.999328	-36.603557	24.074538
24	1.128000	0.999328	-39.150940	19.662341
25	1.128000	0.999328	-41.168873	14.984244
26	1.128000	0.999328	-42.630069	10.103512
27	1.128000	0.999328	-43.514766	5.086147
28	1.128000	0.999328	-43.811000	0.000000
29	1.128000	0.999328	-43.514766	-5.086147
30	1.128000	0.999328	-42.630069	-10.103512
31	1.128000	0.999328	-41.168873	-14.984244
32	1.128000	0.999328	-39.150940	-19.662341
33	1.128000	0.999328	-36.603557	-24.074538
34	1.128000	0.999328	-33.561173	-28.161168
35	1.128000	0.999328	-30.064932	-31.866967
36	1.128000	0.999328	-26.162115	-35.141819
37	1.128000	0.999328	-21.905500	-37.941439
38	1.128000	0.999328	-17.352651	-40.227966
39	1.128000	0.999328	-12.565136	-41.970479
40	1.128000	0.999328	-7.607700	-43.145412
41	1.128000	0.999328	-2.547383	-43.736879
42	1.128000	0.999328	2.547383	-43.736879
43	1.128000	0.999328	7.607700	-43.145412
44	1.128000	0.999328	12.565136	-41.970479
45	1.128000	0.999328	17.352651	-40.227966
46	1.128000	0.999328	21.905500	-37.941439
47	1.128000	0.999328	26.162115	-35.141819
48	1.128000	0.999328	30.064932	-31.866967
49	1.128000	0.999328	33.561173	-28.161168
50	1.128000	0.999328	36.603557	-24.074538
51	1.128000	0.999328	39.150940	-19.662341
52	1.128000	0.999328	41.168873	-14.984244
53	1.128000	0.999328	42.630069	-10.103512
54	1.128000	0.999328	43.514766	-5.086147

NOTE: The positions of the above rebars were computed by LFile

Minimum spacing between any two bars not equal to zero = 3.967 inches
between bars 46 and 47.

Ratio of bar spacing to maximum aggregate size = 5.29

Concrete Properties:

Compressive Strength of Concrete = 4500. psi
Modulus of Elasticity of Concrete = 3823676. psi
Modulus of Rupture of Concrete = -503.115295 psi
Compression Strain at Peak Stress = 0.002001
Tensile Strain at Fracture of Concrete = -0.0001152
Maximum Coarse Aggregate Size = 0.750000 in

Number of Axial Thrust Force Values Determined from Pile-head Loadings = 2

Number	Axial Thrust Force kips
1	63.520
2	101.493

Summary of Results for Nominal Moment Capacity for Section 1

Moment values interpolated at maximum compressive strain = 0.003
or maximum developed moment if pile fails at smaller strains.

Load No.	Axial Thrust kips	Nominal Mom. Cap. in-kip	Max. Comp. Strain
1	63.520	134176.704	0.00300000
2	101.493	135538.710	0.00300000

Note that the values of moment capacity in the table above are not factored by a strength reduction factor (phi-factor).

In ACI 318, the value of the strength reduction factor depends on whether the transverse reinforcing steel bars are tied hoops (0.65) or spirals (0.75).

The above values should be multiplied by the appropriate strength reduction factor to compute ultimate moment capacity according to ACI 318, or the value required by the design standard being followed.

The following table presents factored moment capacities and corresponding bending stiffnesses computed for common resistance factor values used for reinforced concrete sections.

Axial Load No.	Resist. Factor	Nominal Ax. Thrust kips	Nominal Moment Cap in-kips	Ult. (Fac) Ax. Thrust kips	Ult. (Fac) Moment Cap in-kips	Bend. Stiff. at Ult Mom kip-in^2
1	0.65	63.520000	134177.	41.288000	87215.	3.1892E+09
2	0.65	101.493333	135539.	65.970667	88100.	3.2257E+09
1	0.75	63.520000	134177.	47.640000	100633.	3.0731E+09
2	0.75	101.493333	135539.	76.120000	101654.	3.1092E+09
1	0.90	63.520000	134177.	57.168000	120759.	2.0005E+09
2	0.90	101.493333	135539.	91.344000	121985.	2.0258E+09

Summary of Pile-head Responses for Conventional Analyses

Definitions of Pile-head Loading Conditions:

Load Type 1: Load 1 = Shear, V, lbs, and Load 2 = Moment, M, in-lbs
Load Type 2: Load 1 = Shear, V, lbs, and Load 2 = Slope, S, radians
Load Type 3: Load 1 = Shear, V, lbs, and Load 2 = Rot. Stiffness, R, in-lbs/rad.

Load Type 4: Load 1 = Top Deflection, y, inches, and Load 2 = Moment, M, in-lbs
 Load Type 5: Load 1 = Top Deflection, y, inches, and Load 2 = Slope, S, radians

Load Case No.	Load Type 1	Pile-head Load 1	Load Type 2	Pile-head Load 2	Axial Loading lbs	Pile-head Deflection inches	Pile-head Rotation radians	Max Shear in Pile lbs	Max Moment in Pile in-lbs
1	V, lb	71640.	M, in-lb	1.29E+08	101493.	18.7608	-0.07350	-572272.	1.31E+08
2	V, lb	13730.	M, in-lb	2.47E+07	63520.	0.05434	-3.59E-04	-100368.	2.50E+07

Maximum pile-head deflection = 18.7608261487 inches
 Maximum pile-head rotation = -0.0735009674 radians = -4.211295 deg.

The analysis ended normally.

IBC 1807.3.2.1

Moment (ft·k)	8,054.65	
Shear (k)	53.73	
Caisson diameter (ft)	8	
Caisson height above ground (ft)	0.5	
Caisson height below ground (ft)	32	
Lateral soil pressure (lb/ft ²)	300.00	
Ground to application of force, h (ft)	150.41	
Applied lateral force, P (lb)	53,730	
Lateral soil bearing pressure, S ₁ (lb/ft)	3,200.00	
Diameter, b (ft)	8	
A	4.91	$= (2.34P)/(S_1 b)$
Minimum depth of embedment, d (ft)	30.94	$= 0.5A[1 + (1 + (4.36h / A))^{1/2}]$

Exhibit “4”

STATE OF MARYLAND

)

)

AFFIDAVIT OF CHRISTIAN WINKLER

COUNTY OF WASHINGTON

)

I, Christian Winkler, being duly sworn, hereby state and affirm as follows:

1. This affidavit is based on my knowledge and I am competent to testify regarding those things about which I have knowledge.
2. I am the President of Arcola Towers.
3. Arcola Towers will be applying to Washington County for a proposed telecommunications facility at 10944 White Hall Road, Smithsburg, MD 21783, the property of James H. Miller and Elaine K. Miller, Account #18013932 for Verizon Wireless.
4. Pursuant to Section 4.22.6 of the Washington County Zoning Ordinance, I confirm and agree that one ten (10) foot space on the proposed tower will be specifically reserved for use by Washington County, and that other spaces will be made available to other future users, when possible.

Further affiant sayeth not.

Christian Winkler

SWORN to me this 5th day
of November 2025

Notary Public for Commonwealth of Virginia

My Commission Expires 6-30-2026



Exhibit “5”

October 24, 2025

Ryan Foltz
Arcola Towers

RE: Proposed 187' Sabre Monopole for MD-003 Whitehall, MD

Dear Mr. Foltz,

Upon receipt of order, we propose to design and supply the above referenced Sabre monopole for an Ultimate Wind Speed of 112 mph without ice and 40 mph + 1" ice, Risk Category II, Exposure Category C, and Topographic Category 1, in accordance with the Telecommunications Industry Association Standard ANSI/TIA 222-H-2017 "Structural Standard for Antenna Supporting Structures and Antennas".

When designed according to this standard, the wind pressures and steel strength capacities include several safety factors. Therefore, it is highly unlikely that the monopole will fail structurally in a wind event where the design wind speed is exceeded within the range of the built-in safety factors.

Should the wind speed increase beyond the capacity of the built-in safety factors, to the point of failure of one or more structural elements, the most likely location of the failure would be within the monopole shaft, above the base plate. Assuming that the wind pressure profile is similar to that used to design the monopole, the monopole will buckle at the location of the highest combined stress ratio within the monopole shaft. This is likely to result in the portion of the monopole above leaning over and remaining in a permanently deformed condition. This would effectively result in a fall radius of 187' at ground level. *Please note that this letter only applies to the above referenced monopole designed and manufactured by Sabre Industries.*

Sincerely,

Robert E. Beacom, P.E., S.E.
Engineering Manager



sabre

Sabre Industries, Inc.

7101 Southbridge Drive
Sioux City, IA 51111

Exhibit “6”



Mail Processing Center
 Federal Aviation Administration
 Southwest Regional Office
 Obstruction Evaluation Group
 10101 Hillwood Parkway
 Fort Worth, TX 76177

Aeronautical Study No.
 2025-AEA-12000-OE

Issued Date: 01/21/2026

ARCOLA TOWERS
 RYAN FOLTZ
 112 W WASHINGTON ST STE 201
 MIDDLEBURG, VA 20117

**** DETERMINATION OF NO HAZARD TO AIR NAVIGATION ****

The Federal Aviation Administration has conducted an aeronautical study under the provisions of 49 U.S.C., Section 44718 and if applicable Title 14 of the Code of Federal Regulations, part 77, concerning:

Structure: Monopole ARCOLA TOWERS - WHITEHALL
 County, State: Washington, Maryland

Collected Point(s):

Label	Latitude	Longitude	SE	DET AGL	AMSL
pt-1	39-36-51.69N	77-38-45.77W	605 Ft	189 Ft	794 Ft

This aeronautical study revealed that the structure does not exceed obstruction standards and would not be a hazard to air navigation provided the following condition(s), if any, is(are) met:

Emissions from this site must be in compliance with the parameters set by collaboration between the FAA and telecommunications companies and reflected in the FAA 5G C band compatibility evaluation process (such as power, frequencies, and tilt angle). Operational use of this frequency band is not objectionable provided the Wireless Providers (WP) obtain and adhere to the parameters established by the FAA 5G C band compatibility evaluation process. **Failure to comply with this condition will void this determination of no hazard.**

See attachment for additional condition(s) or information.

Based on this evaluation, marking and lighting are not necessary for aviation safety. However, if marking/lighting are accomplished on a voluntary basis, we recommend it be installed in accordance with FAA Advisory circular 70/7460-1 M Change 1.

This determination is based, in part, on the foregoing description which includes specific coordinates, heights, frequency(ies) and power. Any changes in coordinates, heights, and frequencies or use of greater power, except those frequencies specified in the Colo Void Clause Coalition; Antenna System Co-Location; Voluntary Best Practices, will void this determination. Any future construction or alteration, including increase to heights, power, or the addition of other transmitters, requires separate notice to the FAA. This determination includes all previously filed frequencies and power for this structure.

If construction or alteration is dismantled or destroyed, you must submit notice to the FAA within 5 days after the construction or alteration is dismantled or destroyed.

This determination does include temporary construction equipment such as cranes, derricks, etc., which may be used during actual construction of the structure. However, this equipment shall not exceed the overall heights as indicated above. Equipment which has a height greater than the studied structure requires separate notice to the FAA.

This determination concerns the effect of this structure on the safe and efficient use of navigable airspace by aircraft and does not relieve the sponsor of compliance responsibilities relating to any law, ordinance, or regulation of any Federal, State, or local government body.

A copy of this determination will be forwarded to the Federal Communications Commission (FCC) because the structure is subject to their licensing authority.

If we can be of further assistance, please contact our office at 1-817-222-5915, or Rodney.H-CTR.Love@faa.gov. On any future correspondence concerning this matter, please refer to Aeronautical Study Number 2025-AEA-12000-OE.

Signature Control No: 677612029-690948273

(DNE)

Rodney Love
Technician

Attachment(s)
Additional Information
Frequency Data
Map(s)

cc: FCC

BASIS FOR DECISION:

Part 77 authorizes the FAA to evaluate a structure or object's potential electromagnetic effects on air navigation, communication facilities, and other surveillance systems. It also authorizes study of impact on arrival, departure, and en route procedures for aircraft operating under visual or instrument flight rules, as well as the impact on airport traffic capacity at existing public use airports. Broadcast in the 3.7 to 3.98 GHz frequency (5G C band) currently causes errors in certain aircraft radio altimeters and the FAA has determined they cannot be relied upon to perform their intended function when experiencing interference from wireless broadband operations in the 5G C band. The FAA has adopted Airworthiness Directives for all transport and commuter category aircraft equipped with radio altimeters that prohibit certain operations when in the presence of 5G C band.

This determination of no hazard is based upon those mitigations implemented by the FAA and operators of transport and commuter category aircraft, and helicopters operating in the vicinity of your proposed location. It is also based on telecommunication industry and FAA collaboration on acceptable power levels and other parameters as reflected in the FAA 5G C band evaluation process.

The FAA 5G C band compatibility evaluation is a data analytics system used by FAA to evaluate operational hazards related to aircraft design. The FAA 5G C band compatibility evaluation process refers to the process in which the telecommunication companies and the FAA have set parameters, such as power output, locations, frequencies, and tilt angles for antenna that mitigate the hazard to aviation. As the telecommunication companies and FAA refine the tools and methodology, the allowable frequencies and power levels may change in the FAA 5G C band compatibility evaluation process. Therefore, your proposal will not have a substantial adverse effect on the safe and efficient use of the navigable airspace by aircraft provided the equipment and emissions are in compliance with the parameters established through the FAA 5G C band compatibility evaluation process.

Any future changes that are not consistent with the parameters listed in the FAA 5G C band compatibility evaluation process will void this determination of no hazard.

The FAA recognizes emissions in the 3.7-3.98 GHz band at this location will result in Electromagnetic Interference (EMI) as described in Airworthiness Directives (AD) 2021-23-12 and 2021-23-13. NAS services including airport and helicopter operations within a radius of 42 NM will be impacted by 5G RF emissions. Operational use of this frequency band is not objectionable provided the Wireless Providers obtain and adhere to the parameters established by the FAA 5G C band compatibility evaluation process.

Frequency Data for ASN 2025-AEA-12000-OE

LOW FREQUENCY	HIGH FREQUENCY	FREQUENCY UNIT	ERP	ERP UNIT
6	7	GHz	42	dBW
6	7	GHz	55	dBW
10	11.7	GHz	42	dBW
10	11.7	GHz	55	dBW
17.7	19.7	GHz	42	dBW
17.7	19.7	GHz	55	dBW
21.2	23.6	GHz	42	dBW
21.2	23.6	GHz	55	dBW
614	698	MHz	1000	W
614	698	MHz	2000	W
698	806	MHz	1000	W
806	824	MHz	500	W
806	901	MHz	500	W
824	849	MHz	500	W
851	866	MHz	500	W
869	894	MHz	500	W
896	901	MHz	500	W
901	902	MHz	7	W
929	932	MHz	3500	W
930	931	MHz	3500	W
931	932	MHz	3500	W
932	932.5	MHz	17	dBW
935	940	MHz	1000	W
940	941	MHz	3500	W
1670	1675	MHz	500	W
1710	1755	MHz	500	W
1850	1910	MHz	1640	W
1850	1990	MHz	1640	W
1930	1990	MHz	1640	W
1990	2025	MHz	500	W
2110	2200	MHz	500	W
2305	2360	MHz	2000	W
2305	2310	MHz	2000	W
2345	2360	MHz	2000	W
2496	2690	MHz	500	W
3700	3980	MHz	1640	W



Ken Patterson

Airspace Consulting, Inc.

www.airspace-ken.com

Site ID: Whitehall 1164.015

December 1, 2025

To Whom It May Concern:

On December 1, 2025, I personally conducted an evaluation of a proposed telecommunications site for Arcola Towers. The study was to determine if the proposed structure would create any adverse effect on navigable airspace. The site is located near Smithsburg, Maryland at 39° 36' 51.69" North and 77° 38' 45.77" West (NAD 83). The site elevation is 605' above mean sea level (AMSL). The proposed structure height is 189' above ground level (AGL) or 794' AMSL. Part 77 of the Federal Air Regulations and Part 17 of the FCC Rules and Regulations were used as the primary reference for this evaluation.

The closest public use or DOD landing surface is Runway 02 at Hagerstown Regional Richard A. Henson Field. The distance to the runway is 6.58 nautical miles on a true bearing of 144.17° from the runway.

The proposed 189' AGL (794' AMSL) structure would not exceed any FAR Part 77 or FCC Part 17 notice requirement and, therefore, notice to the FAA is not required for this structure. If filed, the 189' AGL structure should be approved by the FAA.

Normally, structures that do not require notice to the FAA do not require marking and/or lighting. Private use landing facilities and AM broadcast stations are not a factor for this study.

For additional information or questions about this study, contact my office anytime.

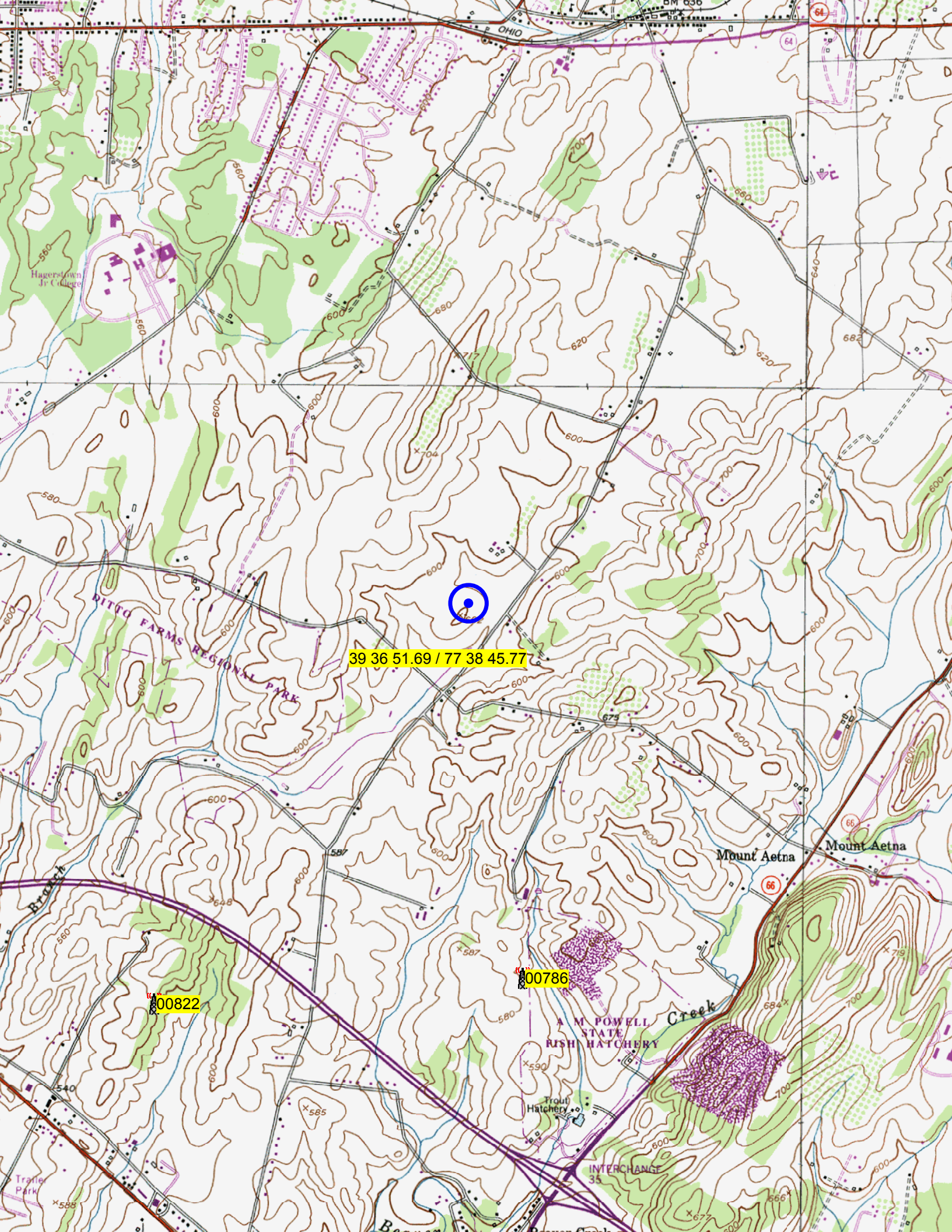
Sincerely,



Ken Patterson

KP16118

150 Discovery Lake Dr, Fayetteville, GA 30215
(770) 461-0563 kpac0@bellsouth.net



39 36 51.69 / 77 38 45.77

00822

00786

INTERCHANGE 35

A. M. POWELL STATE RISH HATCHERY

Mount Aetna

Mount Aetna

Hagerstown Jr College

BITTIG FARMS REGIONAL PARK

OHIO

Trails Park

Creek

Beaver

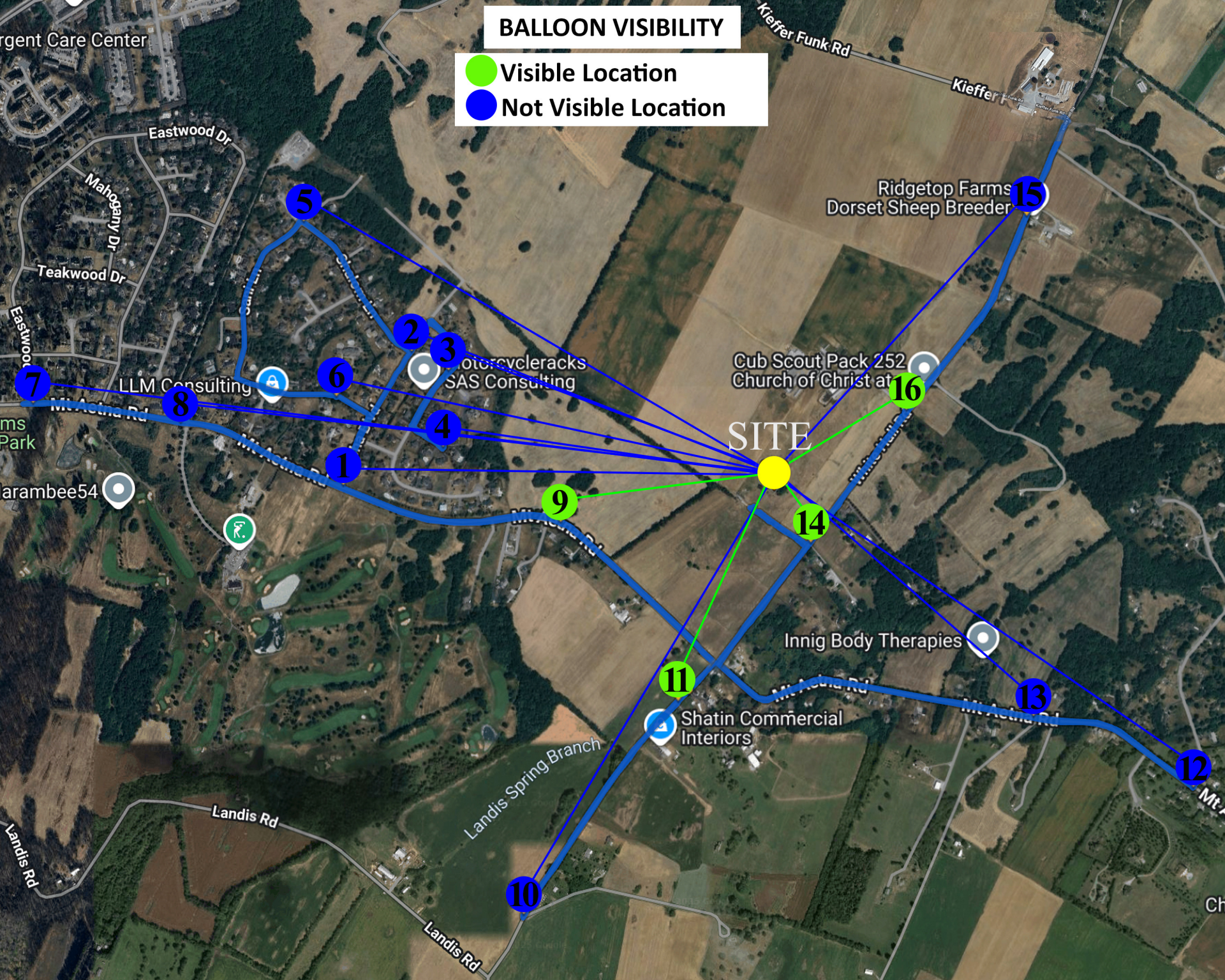
Downs

Exhibit “7”

BALLOON VISIBILITY

Visible Location

Not Visible Location



WGS84
±11ft

39°36'51.40"N, 77°39'33.68"W Δ ^{ft}_{±10ft}

601

 °,T
±18

NE75



NOT VISIBLE

WGS84
±20ft

39°37'3.12"N, 77°39'25.94"W Δ ^{ft}_{±10ft}

621

 °,T
±17

SE106



NOT VISIBLE

24Oct25 08:41 Arcola Whitehall
10941 Sasha Blvd, Hagerstown MD 21742, US © 24-Oct-25 08:41:15

WGS84
±7ft

39°37'1.37"N, 77°39'21.72"W Δ ^{ft}_{±10ft}

633

 °,T
±13

SE110



NOT VISIBLE

WGS84
±17ft

39°36'54.77"N, 77°39'22.36"W Δ ^{ft}_{±10ft}

651

°T
±89

S179



NOT VISIBLE

WGS84
±7ft

39°37'14.42"N, 77°39'38.20"W Δ ^{ft}_{±10ft}

603

°T
±15

SE116



NOT VISIBLE

WGS84
±7ft

39°36'59.17"N, 77°39'34.38"W Δ ^{ft}_{±10ft}

630

°T
±13

SE106



NOT VISIBLE

24Oct25 08:49 Arcola Whitehall
20357 Ayoub Ln, Hagerstown MD 21742, US © 24-Oct-25 08:49:37

WGS84
±23ft

39°36'58.60"N, 77°40'8.78"W Δ ^{ft}_{±10ft}

537

°T
±11

E92



NOT VISIBLE



NOT VISIBLE



PHOTO #9

WHITEHALL PROPOSED MONOPOLE

WGS84
±7ft

39°36'13.99"N, 77°39'13.28"W \wedge ^{ft}_{±10ft}

570

°T
±14

NE33



NOT VISIBLE

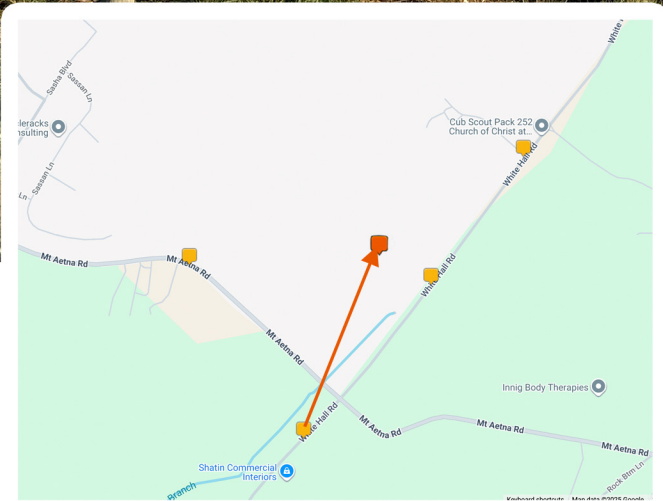


PHOTO #11

WHITEHALL
PROPOSED MONOPOLE



NOT VISIBLE



NOT VISIBLE

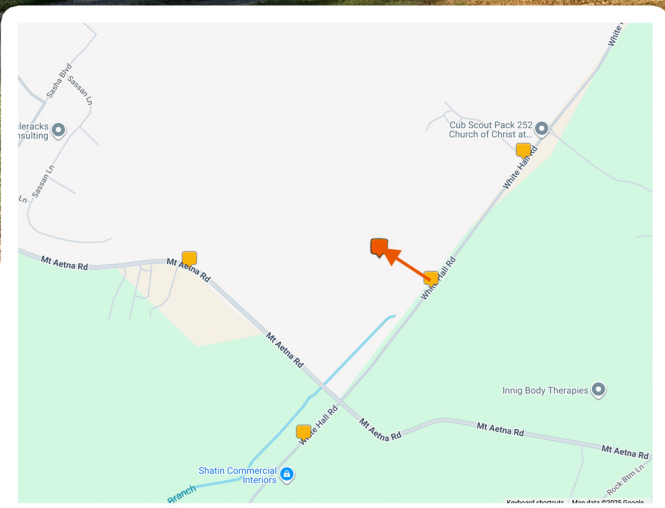


PHOTO #14

WHITEHALL PROPOSED MONOPOLE



NOT VISIBLE



PHOTO #16

WHITEHALL PROPOSED MONOPOLE

Exhibit “8”



December 2, 2025

Katie Rathvon
Zoning Coordinator
Washington County Planning & Zoning

80 West Baltimore Street,
Hagerstown, MD 21740

Re: Proposed Telecommunications Facility at 10944 White Hall Road, Smithsburg, MD 21783 by
Milestone Towers for Verizon Wireless - Telecommunications Facility Application - Tower Removal Letter

Dear Ms. Rathvon,

Arcola Towers, its successors and assigns, provides this statement declaring itself, its successors and assigns of being responsible for compliance with Section 4.22.12 of the Washington County Zoning Ordinance, which requires the following:

1. A Commercial Communication tower that is out of service for a continuous six (6) month period will be deemed to have been abandoned. The Zoning Administrator may issue a Notice of Abandonment to the Owner of the tower that is deemed to be abandoned. The Owner shall have the right to respond in writing to the Notice of Abandonment setting forth the reasons for operation difficulty and providing a reasonable timeframe for correction action, within thirty (30) days from the date of the Notice. The Administrator shall withdraw the Notice of Abandonment and notify the Owner that the Notice has been withdrawn if the Owner provides information that demonstrates the Tower has not been abandoned.
2. If the Tower is determined to be abandoned, the Owner of the Tower shall remove the Tower and all related equipment at the Owner's sole expense within three (3) months of the Date of Notice of Abandonment. If the Owner fails to remove the Tower and related equipment, the Administrator may pursue legal action to have the Tower removed at the Owner's expense.

Please contact me should you have any questions.

A handwritten signature in black ink that reads "Ryan Foltz".

Ryan Foltz
Arcola Towers
Senior Development Director

Exhibit “9”



December 2, 2025

Katie Rathvon
Zoning Coordinator
80 West Baltimore Street,
Hagerstown, MD 21740

Re: Proposed 187-foot monopole-style wireless communication facility to be located at 10944 White Hall Road, Smithsburg, MD 21783 (Account ID # 18-013932) by Arcola Towers – Existing Inventory Letter

Dear Ms. Rathvon,

Arcola Towers' proposed wireless communication facility near located at 10944 White Hall Road, Smithsburg, MD 21783 will be designed and built in full compliance with Section 4.22(B)(1) of the Washington County Zoning Ordinance. Currently, Arcola Towers only owns and/or operates the following tower in the Washington County jurisdiction and within one (1) mile thereof:

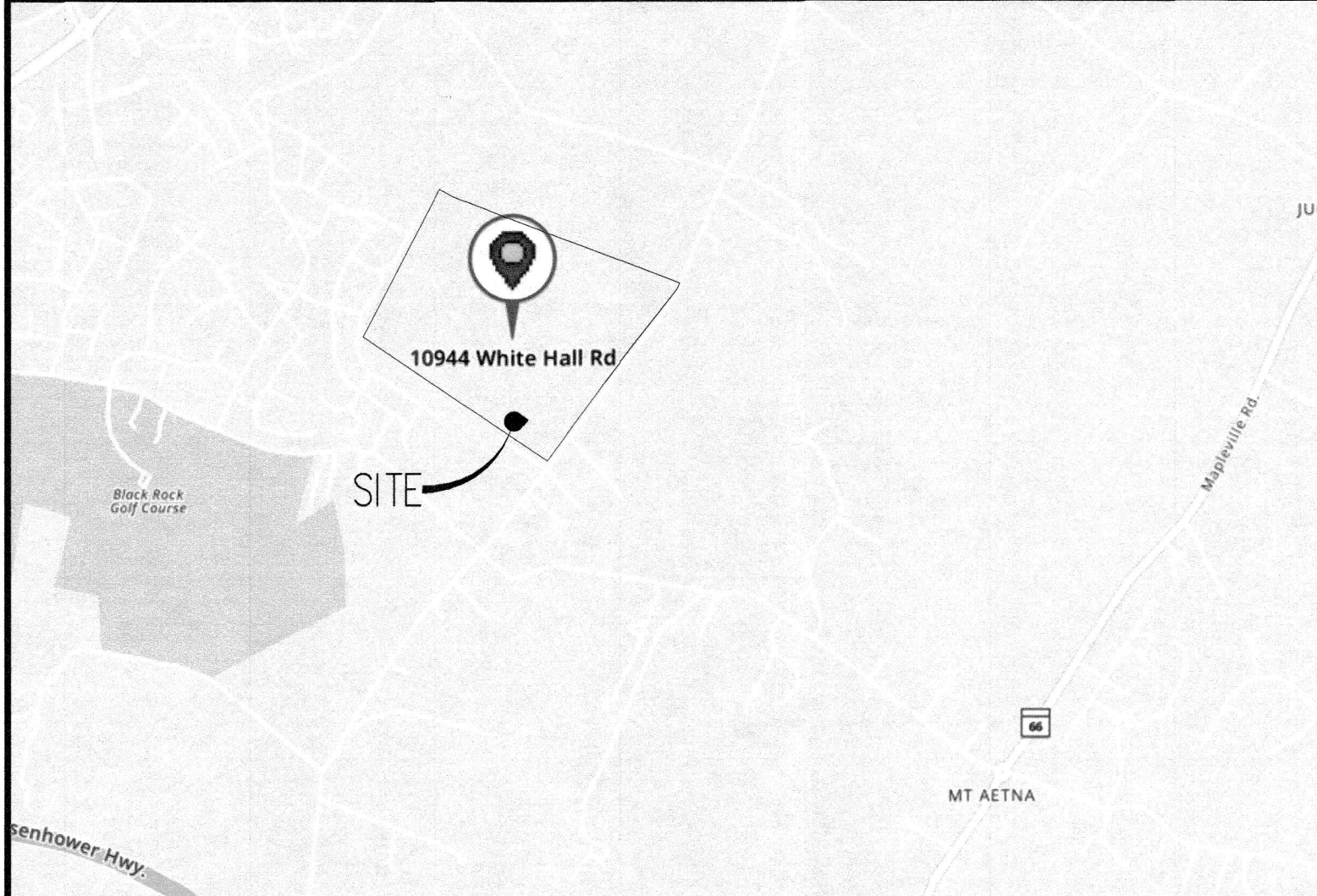
- 1.) **Paramount-** 140' Monopole – 19224 Longmeadow Road, Hagerstown, MD 21742:
Account ID # 27-019196.

If you have further questions, please do not hesitate to contact me directly.

A handwritten signature in black ink that reads "Ryan Foltz". The signature is written in a cursive, flowing style.

Ryan Foltz
Arcola Towers
Senior Development Director

Exhibit “10”



VICINITY MAP
SCALE: 1" = 2,000'-0"

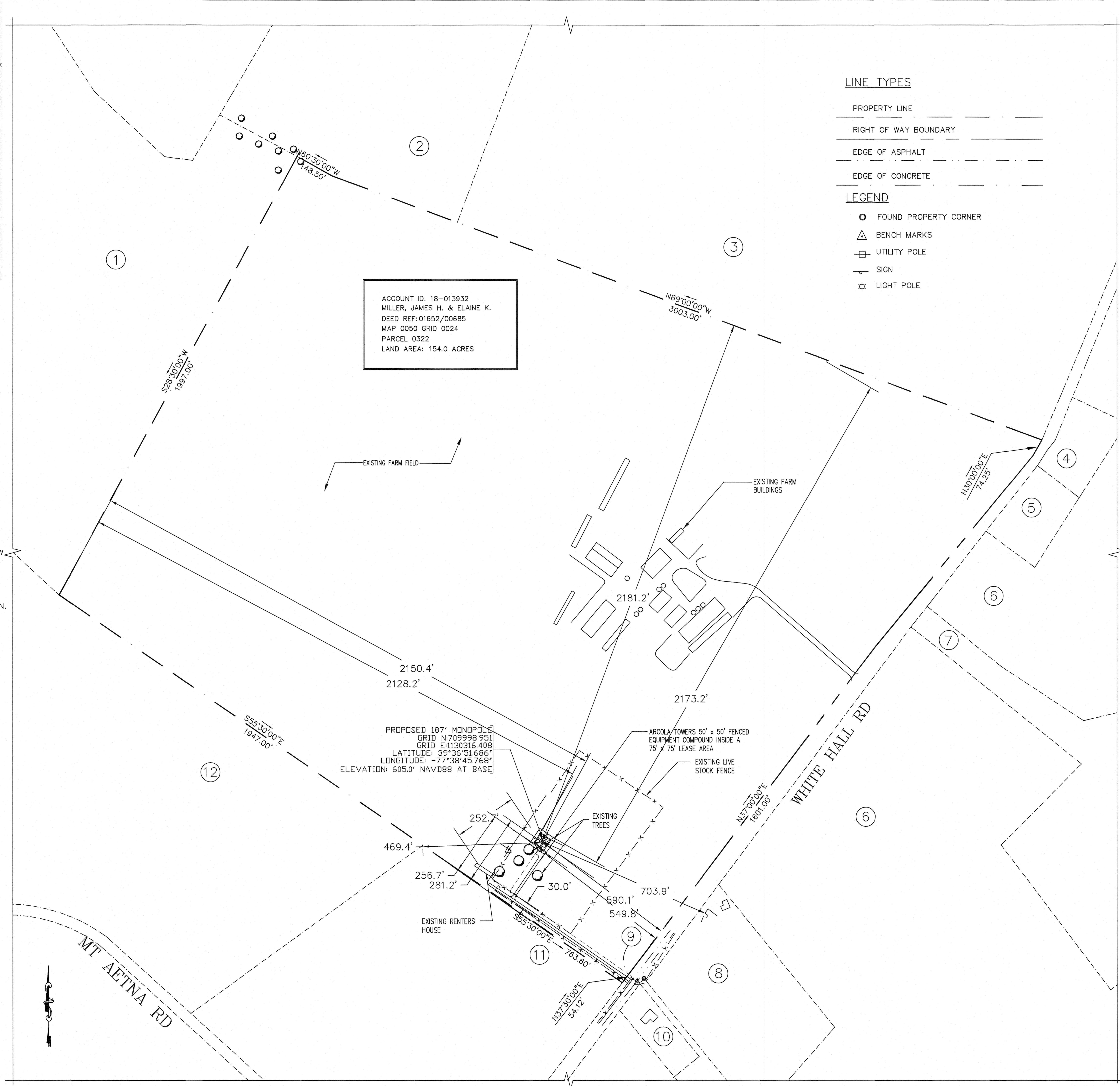


ZONING DRAWING NOTES

- SITE NAME: ARCOLA TOWERS SITE - WHITEHALL
- THIS IS NOT A BOUNDARY SURVEY AND IS NOT TO BE USED FOR THE TRANSFER OF PROPERTY.
- THE SUBJECT PARCEL INFORMATION;
OWNER: MILLER, JAMES H. & ELAINE K.
PREMISES ADDRESS: 10944 WHITE HALL ROAD
SMITHSBURG, MD 21783
MAILING ADDRESS: 12290 SCOTT ROAD
WAYNESBORO, PA 17268
JURISDICTION: WASHINGTON COUNTY
ACCOUNT NO.: 18-013932
PARCEL: 0322, DEED REF: 01652/00685
MAP 0050 GRID 0024
AREA: 154 ACRES
- THE LOCATION OF THE PROPOSED MONOPOLE IS AS FOLLOWS; THE VALUES LISTED BELOW ARE WITHIN ±50' HORIZONTAL AND ±20' VERTICAL.
LATITUDE: ± N 39° 36' 51.686"
LONGITUDE: ± W 77° 38' 45.768"
ELEVATION: 605.0' NAVD88 AT MONOPOLE BASE
- THERE IS NO FLOODPLAIN ON THE PROPERTY THAT IS THE SUBJECT OF THIS APPLICATION. THE CURRENT FLOOD INSURANCE RATE MAP (FIRM) OF WASHINGTON COUNTY, MD, COMMUNITY PANEL NUMBER FOR THE PROPERTY THAT IS THE SUBJECT OF THIS APPLICATION IS 24043C0310D, EFFECTIVE AUGUST 15, 2017.
- THIS SITE IS NOT LOCATED WITHIN THE APPALACHIAN TRAIL CARTER SPECIAL PLANNING AREA, THE ANTIETAM OVERLAY ZONING DISTRICT OR THE HISTORIC PRESERVATION ZONING DISTRICT
- THE SITE IS LOCATED MORE THAN 389' FROM ANY EXISTING OFFSITE DWELLING, SCHOOL, CHURCH, OR INSTITUTION FOR HUMAN CARE.
- THE CLOSEST EXISTING TOWER IS A 205' AMERICAN TOWERS SELF-SUPPORT TOWER, LOCATED APPROXIMATELY 1.75 MI SOUTH.
- THERE ARE NO OVERHEAD TRANSMISSION LINES WITHIN TWO TIMES THE HEIGHT OF THE TOWER.
- NO WETLANDS HAVE BEEN DEFINED AND ANY AREAS SHOWN AS MARSH, PONDS OR DITCHES ARE DONE SO FROM VISIBLE SURFACE FEATURES AND IN NO WAY CONSTITUTE A DEFINED WETLAND.
- EXISTING HEAVY MATURE VEGETATION WILL BE UTILIZED IN LIEU OF NEW LANDSCAPING.

MONOPOLE SETBACKS		
	REQUIRED	PROPOSED
FRONT YARD (SOUTHEAST)	187'	590.1'
REAR YARD (NORTHWEST)	187'	2150.4'
SIDE YARD (SOUTHWEST)	187'	281.2'
SIDE YARD (NORTHEAST)	187'	2181.2'
CLOSEST OFF SITE RESIDENCE	187'	703.9'

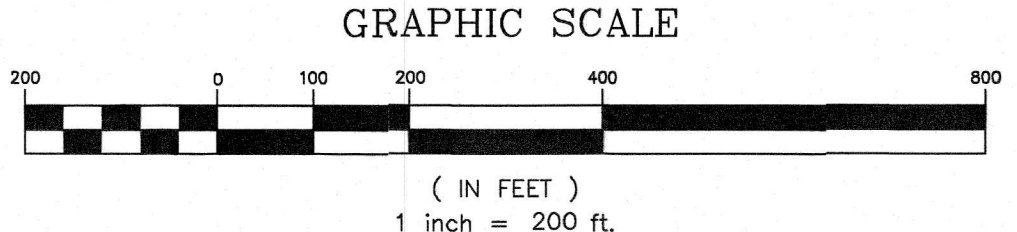
COMPOUND SETBACKS		
	REQUIRED	PROPOSED
FRONT YARD (SOUTHEAST)	50'	549.8'
REAR YARD (NORTHWEST)	50'	21282'
SIDE YARD (SOUTHWEST)	50'	256.7'
SIDE YARD (NORTHEAST)	50'	2173.2'



ACCOUNT ID. 18-013932
MILLER, JAMES H. & ELAINE K.
DEED REF: 01652/00685
MAP 0050 GRID 0024
PARCEL 0322
LAND AREA: 154.0 ACRES

- LINE TYPES**
- PROPERTY LINE
 - RIGHT OF WAY BOUNDARY
 - EDGE OF ASPHALT
 - EDGE OF CONCRETE
- LEGEND**
- FOUND PROPERTY CORNER
 - △ BENCH MARKS
 - UTILITY POLE
 - ⊕ SIGN
 - ☆ LIGHT POLE

PROPOSED 187' MONOPOLE
GRID N:709998.951
GRID E:1130316.408
LATITUDE: 39°36'51.686"
LONGITUDE: -77°38'45.768"
ELEVATION: 605.0' NAVD88 AT BASE



SITE PLAN
SCALE: 1" = 200'
1
Z-1
TRUE NORTH

entrex
communication services, inc.
6100 EXECUTIVE BLVD.
SUITE 430
ROCKVILLE, MD 20852
PHONE: (202) 408-0960

arcola
TOWERS

112 N. WASHINGTON ST
SUITE 201
MIDDLEBURG, VA 20117
PHONE: (571) 895 3990

ARCOLA TOWERS
WHITEHALL
10944 WHITE HALL RD
SMITHSBURG, MD 21783



SUBMITTALS

DATE	DESCRIPTION
09-09-25	ZONING REVIEW
09-24-25	ZONING REVIEW
10-08-25	ZONING REVIEW
10-14-25	ZONING

PROJECT NO: 1164.015
DESIGNER: R.S.
ENGINEER: C.S.

THESE DRAWINGS ARE FORMATTED TO BE FULL-SIZE AT 24"x36"
0 1/2 1
GRAPHIC SCALE IN INCHES
SHEET TITLE:

SITE PLAN

SHEET NUMBER:

Z-1

Exhibit “11”

TITLE NOT SEARCHED OR CERTIFIED

THIS DEED, Made this 1st day of May, 2001, by **Lewis M. Miller and Grace E. Miller**, hereinafter referred to as Grantors.

WITNESSETH: That for and in consideration of the sum of Four Hundred Fifty Thousand (\$450,000.00) Dollars, cash in hand paid, and other good and valuable considerations, the receipt of which is hereby acknowledged, the said Grantors do hereby grant and convey unto **James H. Miller and Elaine K. Miller, husband and wife, as tenants by the entirety**, all that lot or parcel of land, together with the improvements thereon, if any, and all rights, ways, alleys, privileges and appurtenances thereunto belonging or in anywise appertaining, situate on the Western side of the White Hall Road, a short distance South of the Village of Chewsville, in Election District No. 18, in Washington County, Maryland, and being more particularly described as follows:

Being a part of a tract of land called "The Resurvey on the Old Fox Deceived", and part of a tract of land called "The Resurvey on Carrs Queasy", lying contiguous and described by metes and bounds as follows: Beginning at a planted stone, said stone being the beginning of a deed from William J. Huyett to Jonas Q. Huffer, dated the 7th day of April, 1876, and recorded in Liber 74, folio 208, one of the Land Records of Washington County, Maryland, and running thence with the line of the aforesaid deed, South 28 1/2 degrees West 121 perches to the end of a stone wall, thence South 55 1/2 degrees East 118 perches to a stone at the end of the second line of a deed from Jacob Stotler, Sr. and wife to the said Jonas Q. Huffer dated the 20th day of May, A.D. 1881, and running thence with the third line of said deed, South 55 1/2 degrees East 46.28 perches to the middle of the public road leading from Chewsville to Beaver Creek; thence along the middle of said road, and with the fourth line of said last mentioned deed, North 37 1/2 degrees East 3.28 perches to the end of the fourth line of the first mentioned deed, thence along the middle of the said public road with the land of the first mentioned deed, North 37 degrees East 97 perches, North 39 1/2 degrees East 60 perches, North 30 degrees East 4 1/2 perches to a rock near the center of the aforesaid public road; thence leaving the said road, North 69 degrees West 182 perches, North 60 1/2 degrees West 9 perches to the place of beginning; CONTAINING 154 acres and 8 perches of land, more or less.

Being all and the same real estate described and conveyed in a deed from Theophilus C. Herbst and Katherine M. Herbst, to Lewis M. Miller and Grace E. Miller, dated September 30, 1971 and recorded at Liber 531, folio 396, among the Land Records for Washington County, Maryland.

The above-described property is conveyed subject to all covenants, conditions, restrictions, easements and rights of way of record applicable thereto.

And the said Grantors do hereby covenant that they will warrant specially the property hereby conveyed, except as to the aforementioned covenants, conditions, restrictions, easements and rights of way of record, and that they will execute such other and further assurances as may be requisite and necessary.

WITNESS the hands and seals of the said Grantors.

Res# WA02 Rcpt # 40754
DJW TLC Bk # 1655
May 08, 2001 01:33 PM

IMP FD SURE \$ 5.00
RECORDING FEE 20.00
RECORDATION T 3,420.00
TR TAX STATE 2,250.00
TOTAL 5,695.00

01652 00686

CLERK OF THE CIRCUIT COURT
WASHINGTON COUNTY

WITNESS:

Diann L. Linnay

Lewis M. Miller (SEAL)
Lewis M. Miller

Diann L. Linnay

Grace E. Miller (SEAL)
Grace E. Miller

STATE OF MARYLAND, COUNTY OF WASHINGTON, To-Wit:

I HEREBY CERTIFY, That on this 1st day of May, 2001, before me, the subscriber, a Notary Public in and for the State and County aforesaid, personally appeared Lewis M. Miller and Grace E. Miller, and each acknowledged the foregoing deed to be their voluntary act and deed and that the actual consideration paid or to be paid for the foregoing conveyance, including the amount of any mortgage or deed of trust assumed by the Grantees, is in the sum total of (\$450,000.00) Dollars.

WITNESS my hand and Official Notarial Seal.

My Commission Expires: 8/1/04

Diann L. Linnay
Notary Public

THIS IS TO CERTIFY, That the within instrument has been prepared by or under the supervision of an attorney admitted to practice law in the State of Maryland.

Kent N. Oliver
Kent N. Oliver, Attorney

mail to: Kent Oliver
P.O. Box 1269
Thagerstown MD 21741

AGRICULTURE TAX \$ Letter of Intent
ACREAGE 154 Acres
CLERK Julie Shure

RECEIVED FOR TRANSFER
State Department of
Assessments & Taxation
for Washington County
By Julie Shure Date 5/4/01

TAXES PAID May 8, 2001
TODD L. BERSHBY, TREASURER TLB

WASHINGTON COUNTY CIRCUIT COURT (Land Records) DJW 1652, p. 0686, MSA_CE18_1602. Date available: 09/27/2005. Printed 07/07/2025.

01652 00687

Signed L of J

CLERK OF THE CIRCUIT COURT
State of Maryland Land Instrument Intake Sheet
Baltimore City County: Washington County

Information provided is for use of the Clerk's Office, State Department of Assessments and Taxation and County Finance Office only.
(Type or Print in Black Ink Only--All Copies Must Be Legible)

1 Type(s) of Instruments	<input type="checkbox"/> Deed <input checked="" type="checkbox"/> Mortgage <input type="checkbox"/> Other						
	<input type="checkbox"/> Deed of Trust <input type="checkbox"/> Lease <input type="checkbox"/> Other						
2 Conveyance Type Check Box	<input type="checkbox"/> Improved Sale	<input type="checkbox"/> Unimproved Sale	<input type="checkbox"/> Multiple Accounts	<input type="checkbox"/> Not an Arms-Length Sale[9]			
	Arms-Length[1]	Arms-Length[2]	Arms-Length[3]	Length Sale[9]			
3 Tax Exempt (If Applicable) Cite or Explain Authority	Recording:						
	State Transfer:						
	County Transfer:						
4 Consideration And Tax Calculations	Consideration Amount		Finance Office Use Only				
	Purchase Price/Consideration	\$ 450,000.00	Transfer and Recordation Tax Consideration				
	Any New Mortgage	\$ 450,000.00	Transfer Tax Consideration	\$			
	Balance of Existing Mortgage	\$	X () % =	\$			
	Other:	\$	Less Exemption Amount -	\$			
	Other:	\$	Total Transfer Tax -	\$			
	Full Cash Value	\$	Recordation Tax Consideration	\$			
5 Fees	Amount of Fees	Doc. 1	Doc. 2	Agent:			
	Recording Charge	\$ 20.00	\$ 20.00	Tax Bill:			
	Surcharge	\$ 5.00	\$ 5.00	C.B. Credit			
	State Recordation Tax	\$ 3,420.00	\$ 0	Ag. Tax/Other:			
	State Transfer Tax	\$ 2,250.00	\$ 0				
	County Transfer Tax	\$ 0	\$ 0				
	Other	\$ 0	\$ 0				
	Total Fees	\$ 5,695.00	\$ 25.00				
6 Description of Property	District	Property Tax ID No.(1)	Grantor/ Libe/Folio	Map	Parcel No.	Var.LOG (5)	
	18	18-013932	531/396				
	Subdivision Name		Lot(3a)	Block(3b)	Sect/AR(3c)	Plat Ref.	Sq.Ft./Acreage(4)
	Location/Address of Property Being Conveyed (2)						
	154 acres, 10944 White Hall Road Smithsburg						
	Other Property Identifiers (if applicable)					Water Meter Account No.	
	Residential <input type="checkbox"/> or Non-Residential <input type="checkbox"/> Fee Simple <input type="checkbox"/> or Ground Rent <input type="checkbox"/> Amount:						
	Partial Conveyance? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Description/Amt. of SqFt/Acreage Transferred:						
	If Partial Conveyance, List Improvements Conveyed:						
	7 Transferred From	Doc. 1 - Grantor(s) Name(s)			Doc. 2 - Grantor(s) Name(s)		
Lewis M. Miller			James H. Miller				
Grace E. Miller			Elaine K. Miller				
Doc. 1 - Owner(s) of Record, if different from Grantor(s)			Doc. 2 - Owner(s) of Record, if Different from Grantor(s)				
8 Transferred To	Doc. 1 - Grantee(s) Name(s)			Doc. 2 - Grantee(s) Name(s)			
	James H. Miller						
	Elaine K. Miller						
9 Other Names to Be Indexed	New Owner's (Grantee) Mailing Address						
	10944 White Hall Road, Smithsburg MD 21783						
10 Contact/Mail Information	Instrument Submitted By or Contact Person			<input type="checkbox"/> Return to Contact Person			
	Name: Kent N. Oliver			<input type="checkbox"/> Hold for Pickup			
	Firm: Miller, Oliver, Baker, Moylan & Stone			<input checked="" type="checkbox"/> Return Address Provided			
	Address: 28 West Washington Street						
	Hagerstown, MD 21740						
11 IMPORTANT: BOTH THE ORIGINAL DEED AND A PHOTOCOPY MUST ACCOMPANY EACH TRANSFER	Assessment Information						
	Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> Will the property being conveyed be the grantee's principal residence?						
	Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> Does transfer include personal property? If yes, identify: _____						
	Yes <input type="checkbox"/> No <input checked="" type="checkbox"/> Was property surveyed? If yes, attach copy of survey (if recorded, no copy required).						
	Assessment Use Only - Do Not Write Below This Line						
Terminal Verification <input type="checkbox"/>		Agricultural Verification <input type="checkbox"/>		Whole <input type="checkbox"/> Part <input type="checkbox"/>		Trans. Process Verification <input type="checkbox"/>	
Transfer Number:		Date Received:		Deed Reference		Assigned Property No.	
Year	19	19	Geo.	Map	Sub	Block	
Land			Zoning	Grid	Plat	Lot	
Buildings			Use	Parcel	Section	Occ.Cd.	
Total			Town Cd.	Ex. St.	Ex. Cd.		
Remarks: Signed L of J							

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Exhibit “12”



Washington County

M A R Y L A N D

DIVISION OF
PERMITS AND INSPECTIONS

OWNER'S REPRESENTATIVE AFFIDAVIT

This is to certify that Jonathan L. Yates for Arcola Towers is authorized to make application for the work described as construction of a 187' monopole telecommunications facility and located at 10944 White Hall Road, Smithsburg, MD 21783

The said work is authorized by James H. and Elaine K. Miller, the owner in fee, as required by the International Residential Code and the International Building Code.

This office does not enforce covenants or deed restrictions and it is the owner's, builder's and or developer's responsibility to make sure the covenants and/or deed restrictions are not being violated as a result of the issuance of this permit.

PROPERTY OWNER:

JAMES H MILLER

Name

12290 SCOTT RD

Address

WAYNESBORO PA 17268

City, State, Zip Code

James H Miller
Property Owner's Signature

AUTHORIZED REPRESENTATIVE:

Jonathan Yates

Name

105 Broad St. Third Floor

Address

Charleston, SC 29401

City, State, Zip Code

Jonathan L Yates
Authorized Representative's Signature



BOARD OF ZONING APPEALS

Annual Report Worksheet Year 2025

The Board shall approve an Annual Report for the Reporting Year 2025 as required under Board of Appeals of Washington County Amended Rules of Procedure Meetings (2) a. In addition, this Annual Report shall be provided to the Washington County Commissioners.

Section I- Appeals for the Calendar Year of 2025.

Section II- Ordinance Sections Appealed.

Section III- Appeals Granted, Granted with Conditions, or Denied.

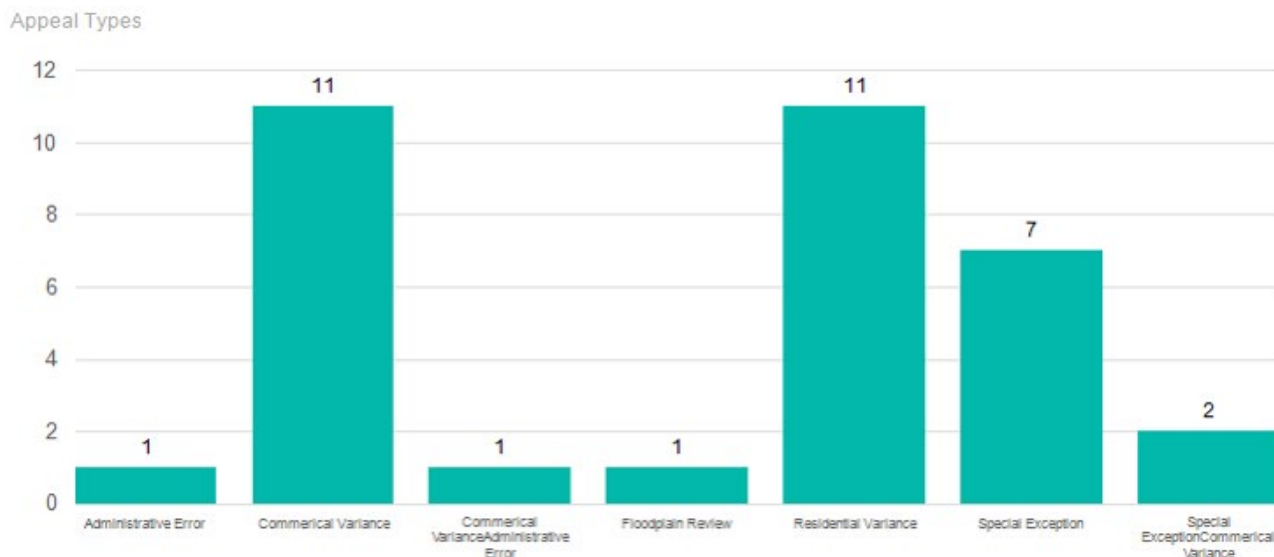
Section IV- Continuances, Postponements, Withdrawals, and Voids.

Section V- Appeals Filed to Circuit Court.

Section VI- Additional Items Completed by the Board.

Section I: Appeals for Calendar Year 2025

Total of 34 appeals were filed for 2025. Within each appeal there could be multiple request types i.e.: special exception with a variance(s).



Section II: Ordinance Sections Appealed

Ordinance Section	# of Appeals Filed
3.3 Land Use Table Special Exceptions	7
22.23 (e) Signage	5
9.5 (a) Residential Urban District Setback Requirements	4
22.12 Parking	4
22.94 (a) Animal Husbandry Setback Requirements	2
5A.5 Agricultural Rural Setback Requirements	2
8.5 (a) Residential Suburban District Setback Requirements	2
5D.3 Rural Village District Residential Uses Dimensional Requirements	2
Washington County Floodplain Ordinance	2
Other Sections of the Zoning Ordinance where only one appeal was filed for Section	7

Section III: Appeals Granted, Granted with Conditions, or Denied

(A) 28 Appeals were Granted by the Board with No Conditions.

(B) 4 Appeals were Denied by the Board.

Appeal	Appeal Type	Request
AP2025-007	Residential Variance	Variance from the required 8 ft. side yard setback to 1 ft. for proposed deck.
AP2025-008	Commercial Variance	Variance from the required 25 ft. setback from the road right-of-way to 22.34 ft. for installed freestanding sign.
AP2025-018	Residential Variance	Variance from the 100 ft. animal husbandry structure setback from all property lines to 7 ft. from the left property line, 40 ft. from the right property line, and 90 ft. from the rear property line.
AP2025-025	Administrative Error and Commercial Variance	Charging Admin. Error of the Zoning Administer stating the interpretation of Section 22.23(e) regarding “use on the Premises Signs” is incorrect for installation of a freestanding sign that would advertise the uses(s) or tenant(s) on adjacent parcels. Variance from the requirement for a property to have a lot frontage of at least 40 ft. in width to be reduced down to 25 ft. and a variance form the 25 ft. setback for the sign support structure from the road right-of-way to 10 ft. for proposed freestanding sign.

(C) 2 Appeals Granted with Conditions

Appeal	Appeal	Condition
AP2025-022	Variance is sought for the following ordinance sections: Section 5.4A to allow lowest floor to be constructed below Flood Elevation, Section 5.4.B.1 to allow floodproofing of a new non-residential building, Section 5.4.B.3.a to allow wet floodproofing in lieu of dry floodproofing for proposed public restroom for public park facility.	Condition: The structure will be elevated 1 ft.
AP2025-024	Special exception to establish a contractor equipment and storage yard.	Conditions: 1. Buffering to be installed and; 2. Downward/shielded lighting for property lines adjacent to residential properties.

(D) 6 Mixed Vote by the Board

Appeal	Vote Count	Appeal Summary
AP2025-004	3-2	Granted-Special exception for proposed 199 ft. monopole commercial communication tower and setbacks for tower and equipment.
AP2025-007	2-2	Denied-Variance from the required 8 ft. side yard setback to 1 ft. for proposed deck.
AP2025-008	3-2	Denied-Variance from the required 25 ft. setback from the road right-of-way to 22.34 ft. for installed freestanding sign.
AP2025-012	3-2	Granted-Variance from the 50 ft. side yard setback for proposed new diesel/gasoline fueling island with canopy.
AP2025-015	3-2	Granted-Variance from the requirement of the substantial improvement interpretation of the floodplain ordinance.
AP2025-025	4-1	Board Found No Error-Charging administrative error of Zoning Administer for interpretation on “use on the Premises Sings” and variance request for propose freestanding sign.

Section IV: Continuances, Postponements, Withdrawals, Reconsideration, and Voids

(A) 2 Appeals were Continued to Second Hearing

Appeal	Appeal Type
AP2025-010	Variance from sign structure and sign from the road right-of-way
AP2025-025	Charging administrative error of Zoning Administer for interpretation on “use on the Premises Sings” and variance request for propose freestanding sign.

(B) 1 Appeal Postponed to Different Hearing

Appeal	Who Made Request	Appeal Type
AP2025-032	Appellant	Variance from front and side yard setback for residential carport

(C) 1 Appeal Withdrawn

Appeal	Who Made Request	Appeal Type
AP2025-032	Appellant	Variance from front and side yard setback for residential carport

(D) No Appeals were Filed for Reconsideration

(E) No Appeals were Voided

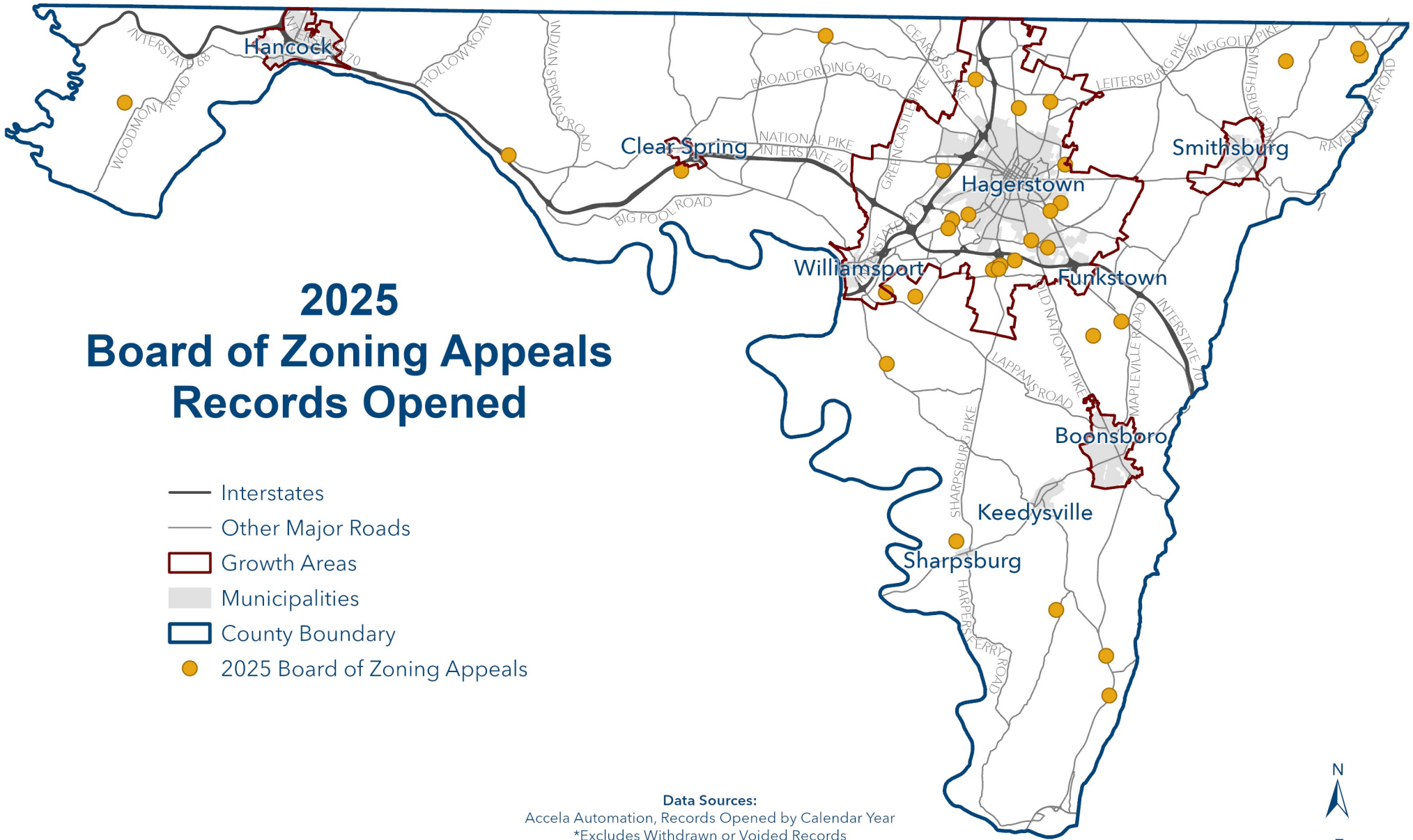
Section V: Appeals Filed to Circuit Court

Appeal	Appeal Summary	Whom Filed to the Court	Status
AP2025-008	Variance from setback for installed freestanding sign	Appellant	Closed-Dismissal-Stipulation
AP2024-038	Residential Variance for Animal Husbandry Setback	Appellant	Closed-Dismissal Lack of Pros.
AP2024-020	Special Exception for Physicians’ Office	Appellant	Closed-Dismissal by Appellant
AP2023-029	Special exception to establish a general retail/merchandise store	Appellant	Pending in Appellate Court

Section VI: Additional Items Completed by the Board

2024 Annual Report

All Opinions Received in a Timely Manner



2025 Board of Zoning Appeals Records Opened

- Interstates
- Other Major Roads
- ▭ Growth Areas
- ▭ Municipalities
- ▭ County Boundary
- 2025 Board of Zoning Appeals

Data Sources:
 Accela Automation, Records Opened by Calendar Year
 *Excludes Withdrawn or Voided Records

Prepared By:
 Washington County Department of Planning and Zoning
 Geographic Information Systems



5

Miles

