BOARD OF APPEALS

April 15, 2026

County Administration Building, 100 W. Washington St., Meeting Room 2000, Hagerstown, at 6:00 p.m.

AGENDA

AP2026-010: An appeal was filed by Arcola Towers for a special exception to establish a commercial communication tower on property owned by James & Elanie Miller and located at 10944 White Hall Road, Smithsburg, Zoned Agricultural Rural District.- **GRANTED**

Annual Report for 2025-APPROVED

Annual Election for Chair and Vice-Chair for the Board of Appeals-Tracie Felker reappointed as Chair, Robert Meyers reappointed as Co-Chair

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Pursuant to the Maryland Open Meetings Law, notice is hereby given that the deliberations of the Board of Zoning Appeals are open to the public. Furthermore, the Board, at its discretion, may render a decision as to some or all of the cases at the hearing described above or at a subsequent hearing, the date and time of which will be announced prior to the conclusion of the public hearing. Individuals requiring special accommodations are requested to contact Katie Rathvon at 240-313-2464 Voice, 240-313-2130 Voice/TDD no later than April 6, 2026. Any person desiring a stenographic transcript shall be responsible for supplying a competent stenographer.

The Board of Appeals reserves the right to vary the order in which the cases are called. Please take note of the Amended Rules of Procedure (Adopted July 5, 2006), Public Hearing, Section 4(d) which states:

Applicants shall have ten (10) minutes in which to present their request and may, upon request to and permission of the Board, receive an additional twenty (20) minutes for their presentation. Following the Applicant's case in chief, other individuals may receive three (3) minutes to testify, except in the circumstance where an individual is representing a group, in which case said individual shall be given eight (8) minutes to testify.

Those Applicants requesting the additional twenty (20) minutes shall have their case automatically moved to the end of the docket.

For extraordinary cause, the Board may extend any time period set forth herein, or otherwise modify or suspend these Rules, to uphold the spirit of the Ordinance and to do substantial justice.

Tracie Felker, Chairman

Board of Zoning Appeals

WASHINGTON COUNTY BOARD OF ZONING APPEALS
747 Northern Avenue | Hagerstown, MD 21742-2723 | P:240.313.2430 | F:240.313.2431 | Hearing Impaired: 7-1-1

ZONING APPEAL

Property Owner: James & Elanie Miller
12290 Scott Road
Waynesboro PA 17268

Appellant: Arcola Towers
112 West Washington Street
Suite 201
Middleburg VA 20117

Property Location: 10944 White Hall Road
Smithsburg, MD 21783

Description Of Appeal: Special exception to establish a commercial communication tower

Docket No: AP2026-010
Tax ID No: 18013932
Zoning: A(R)
RB Overlay: No
Zoning Overlay:

Filed Date: 03/09/2026
Hearing Date: 04/15/2026

Appellant's Legal Interest In Above Property:
Owner: No
Lessee: Yes
Other:
Contract to Rent/Lease: No
Contract to Purchase: No

Previous Petition/Appeal Docket No(s):

Applicable Ordinance Sections: Washington County Zoning Ordinance Section 5A.2 Special Exception, Section 3.3 (1) Table of Land Uses, R. Utilities

Reason For Hardship:

If Appeal of Ruling, Date Of Ruling:

Ruling Official/Agency:

Existing Use: Residential/Agricultural

Proposed Use: Commercial Communication Tower

Previous Use Ceased For At Least 6 Months:

Date Ceased:

Area Devoted To Non-Conforming Use -
Existing:
Proposed:

I hearby affirm that all of the statements and information contained in or filed with this appeal are true and correct.

Appellant Signature

State Of Maryland, Washington County to-wit:

Sworn and subscribed before me this 10 day of March, 2026.

Nov 7, 2029
My Commission Expires

Notary Public

WASHINGTON COUNTY BOARD OF ZONING APPEALS
747 Northern Avenue | Hagerstown, MD 21742-2723 | P:240.313.2430 | F:240.313.2431 | Hearing Impaired: 7-1-1

AFFIDAVIT IN COMPLIANCE WITH SECTION 25.51(C)

Docket No: AP2026-010

State of Maryland Washington County, To Wit:

On 3/9/2026, before me the subscriber, a Notary of the public of the State and County aforesaid, personally appeared Arcola Towers and made oath in due form of law as follows:

Arcola Towers will post the zoning notice sign(s) given to me by the Zoning Administrator in accordance with Section 25.51(c) of the Washington County Zoning Ordinance for the above captioned Board of Appeals case, scheduled for public hearing on 04/15/2026, and that said sign(s) will be erected on the subject property in accordance with the required distances and positioning as set out in the attached posting instructions.

Sign(s) will be posted on 03/31/2026 and will remain until after the above hearing date.

Arcola Towers Agent

Sworn and subscribed before me the day and year first above written.

Notary Public

Seal

Nov. 7, 2029

My Commission Expires

WASHINGTON COUNTY BOARD OF ZONING APPEALS
747 Northern Avenue | Hagerstown, MD 21742-2723 | P:240.313.2430 | F:240.313.2431 | Hearing Impaired: 7-1-1

BOARD OF ZONING APPEALS

ATTENTION!

Posting Instructions

The premises MUST be posted in accordance with the following rules:

1. The sign must be posted a minimum of fourteen (14) days prior to the public hearing Section 25.51(c) Property upon which the application or appeal is concerned shall be posted conspicuously by a zoning notice no less in size than twenty-two (22) inches by twenty-eight (28) inches at least fourteen (14) days before the date of the hearing.
2. The sign must be placed on the property within ten (10) feet of the property line which abuts the most traveled public road.
3. The sign must be posted in a conspicuous manner not over six (6) feet above the ground level, and affixed to a sturdy frame where it will be clearly visible and legible to the public.
4. The sign shall be maintained at all times by the applicant until after the public hearing. If a new sign is needed or required, please contact the Plan Review Department at 240-313-2460.
5. An affidavit certifying the property will be posted for the minimum of fourteen (14) days prior to the public hearing date.

Proper posting of the sign will be spot checked by the Zoning Inspector. IF SIGN IS NOT IN COMPLIANCE, IT MAY RESULT IN RESCHEDULING OF THE HEARING.

Washington County
MARYLAND

BOARD OF ZONING APPEALS

80 West Baltimore Street | Hagerstown, MD 21740 | P: 240.313.2460 | F: 240.313.2461 | Hearing Impaired: 7-1-1

WWW.WASHCO-MD.NET

Appeal for Special Exception

Appeal is hereby made for a special exception under the Washington County Zoning Ordinance as follows:

Location **10944 White Hall Road, Smithsburg, MD 21783**

Appellant's present legal interest in above property: (Check One)

_____ Owner (Including Joint Ownership) __x__ Lessee _____ Contract to rent/lease

_____ Contract to Purchase _____ Other _____

Use Proposed: **187' monopole-style wireless telecommunications facility with a 2 ft. lightning rod for Verizon Wireless.**

Zoning Ordinance section and subsection(s) providing for proposed use: **Section 4.22 Commercial Communications Towers**

If filing functionally similar to a principal permitted use or special exception use, please list the use and describe the use similarities:

Provide Detailed Explanation on Separate Sheet

Has any previous petition or appeal involving this property been made to the Board?

_____ Yes __x__ No

If yes, give docket number(s): _____

Additional comments, if any: **Please see attached narrative.**

I hereby certify that I have, to the best of my knowledge, accurately supplied the information required for the above referenced appeal.

Signature of Appellant

105 Broad St. Third Floor, Charleston, SC 29401
Address of Appellant

jly@hellmanyates.com
Email of Appellant

843-414-9754
Phone Number of Appellant

This appeal form is to be used to assist the customer in gathering the information necessary to submit an application. However, the application shall be processed in person.

Revised April 7, 2021

HELLMAN YATES

JONATHAN L. YATES
DIRECT VOICE 843 414-9754
JLY@HELLMANYATES.COM

HELLMAN & YATES, PA
105 BROAD STREET, THIRD FLOOR
CHARLESTON, SOUTH CAROLINA 29401
V 843 266-9099
F 843 266-9188

March 4, 2026

VIA HAND DELIVERY

Katie Rathvon
Zoning Coordinator
80 West Baltimore Street
Hagerstown, MD 21740

Re: Proposed 187-foot monopole-style communications facility with a 2 ft. lightning rod (for a total height of 189 ft.) located at 10944 White Hall Road, Smithsburg, MD 21783 by Arcola Towers for Verizon Wireless.

Dear Ms. Rathvon,

Enclosed, please find the application of Arcola Towers for a proposed 187-foot monopole-style communications facility with a 2 ft. lightning rod for a total height of 189 feet. The proposed facility is to be located at 10944 White Hall Road, Smithsburg, MD 21783 (Tax Account #18-013932) on the property of James H. and Elaine K. Miller by Arcola Towers for Verizon Wireless. In support of this application, we have taken the liberty of recasting the relevant sections of Washington County Zoning Ordinance, with our answer to the relevant section in bold beneath. As will be evident from a review of the attached, Arcola Towers has not only met, but has exceeded, all the necessary requirements for approval under the Washington County Zoning Ordinance.

Section 4.22 Commercial Communication Towers

The purpose of this section is to regulate the placement, construction, and modification of commercial communications towers as defined in Article 28A (hereinafter "towers") and commercial communications equipment as defined in Article 28A (hereinafter "equipment"). It is the intent of these regulations to minimize the visual impact of towers and equipment, to minimize the number of towers through shared use and co-location, to encourage utilization of technological designs that will either eliminate or reduce the need for new towers to support equipment and to ensure that all towers and equipment are compatible with surrounding land uses while assuring wireless communications service to the citizens of Washington County.

Equipment proposed to be located on an existing tower or antenna support structure as defined in Article 28A shall be allowed in any district provided that the height from grade of the equipment shall not exceed the height from grade of the antenna support structure by more than twenty (20) feet.

The Applicant accepts and acknowledges this provision. This is an application for a new tower. Please see the Site Plan and Drawings by Maryland Professional Engineer Camille Shabshab attached hereto as Exhibit "1" and incorporated herein by reference.

No permit to construct a tower may be issued unless the applicant demonstrates to the Planning Commission, or where applicable, to the Board of Zoning Appeals, need for the tower and that the applicant has exhausted all alternatives to constructing a tower. Applicants are required to prove need by:

a. demonstrating via statement or other evidence that, in terms of location and construction, there are no existing towers, buildings, elevated tanks or other structures able to provide the antenna platform required.

The Applicant accepts and acknowledges this provision. Arcola Towers was able to confirm that there are no existing towers, buildings, elevated tanks, or other structures able to provide the antenna platform required. Please see the Alternative Candidate Analysis by Ryan Foltz attached hereto as Exhibit "2" and incorporated herein by reference. The closest tower is a 195' self-support American Tower site that is 1.29 miles away to the southeast and too far to cover Verizon's intended coverage area.

b. providing evidence, including coverage diagrams and technical reports, demonstrating that co-location on existing sites is not technically possible in order to serve the desired need.

Please see the Alternative Candidate Analysis by Ryan Foltz in Exhibit "2".

A. Design requirements

In addition to the applicable requirements for a site plan as specified in Section 4.11, the applicant shall provide the following information as part of the site plan submittal. These provisions shall apply to towers in all districts where permitted as a principal permitted or special exception use:

1. Subject to a minimum setback of a distance equaling the total height of the tower and equipment. The setback shall be measured from the base of the tower to the boundary line of the property owned, leased, or controlled by easement by the applicant.

The proposed monopole-style communications facility will meet the setback requirements of tower height. As shown on sheet Z-1 of Exhibit "1", the proposed setbacks for the 187 ft. tower are: 590.1 feet to the front, 2150.4 feet to the rear, 281.2 feet to the southwest, and 2181.2 feet to the northeast.

2. Subject to a minimum distance requirement of a distance equaling the height of the tower and equipment plus 200 feet from the RT, RS, RU, RM and R V districts or the nearest part of any existing dwelling, school, church, or institution for human care, in any other district.

As shown in Note 7 on Sheet Z-1 of Exhibit "1", the proposed facility will meet the requirement of a 389 ft. distance from these districts, existing dwellings, schools, churches, or institutions for human care. The closest RT zoned property is 469.4 ft. to the west.

3. Subject to a minimum setback from all overhead transmission lines of a distance equaling two times the height of the tower and equipment.

As shown in Note 9 on sheet Z-1 of Exhibit "1", there are no overhead transmission lines within two times the height of the tower.

4. Subject to a height not to exceed 200 feet. Measurement of tower height shall include the tower structure itself, the base pad, and any other equipment attached thereto which extends more than twenty (20) feet over the top of the tower structure itself. The tower height shall be measured from grade.

The proposed monopole-style communications facility will not exceed 200 feet. As shown on Sheet Z-4 of Exhibit "1" and the Design Drawings by Maryland Professional Engineer Robert E. Beacom attached hereto as Exhibit "3" and incorporated herein by reference, the proposed monopole has an elevation of 187 feet with a two-foot lightning rod for a total height of 189 feet.

5. Proposed towers shall meet the following separation requirements from existing towers or towers which have been issued a permit but are not yet constructed.
    a. Monopole towers shall be separated from all other towers, whether monopole, self-supporting lattice, or guyed, by a minimum of seven hundred and fifty (750) feet.

Please see the Alternative Candidate Analysis in Exhibit "2". There are no existing structures within a 750 ft. radius of the proposed facility. The closest facility is a 195 ft. American Towers self-support tower approximately 1.29 miles away to the southeast and will not cover the intended area for Verizon.

    b. Self-supporting lattice or guyed towers shall be separated from all other self- supporting or guyed towers by a minimum of fifteen hundred (1,500) feet.

The provision does not apply to this application as this is for a monopole-style communications facility.

c. Self-supporting lattice or guyed towers shall be separated from all monopole towers by a minimum of seven hundred and fifty (750) feet.

The provision does not apply to this application as this is for a monopole-style communications facility.

6. All towers shall be designed for co-location, which shall mean the ability of the structure to allow for the placement of comparable equipment for other carriers. An application for a tower shall be accompanied by an affidavit from the applicant stating that one ten (10) foot space on the proposed tower will be specifically reserved for use by the County, and that other spaces will be made available to other future users, when possible.

Please see the Affidavit from Christian Winkler of Arcola Towers attached hereto as Exhibit "4" and incorporated herein by reference. One ten (10) foot space on the proposed tower will be specifically reserved for use by Washington County, and other spaces will be made available to other future users, when possible. The proposed monopole-style facility has been designed for Verizon and collocation by at least three additional broadband carriers as shown in the Design Drawings in Exhibit "3". In addition, please see the ANSI/ Fall Zone letter by Maryland Professional Engineer Robert E. Beacom attached hereto as Exhibit "5" and incorporated herein by reference. Beacom certifies the fall zone at 187' and thus any collapse would be contained on the Miller property.

7. Fencing shall be provided around the base of the tower and any associated equipment buildings.

Please see Sheets Z-4 and Z-6 of Exhibit "1" for the fencing plan. The 50ft-by-50ft compound will be secured by an 8' chain-link fence topped with three strands of barbed wire for a total height of 9 feet.

8. All sites shall be identified by means of a sign no longer than two square feet affixed to the fence identifying the entity using the site and shall provide the telephone number of a contact person in the event of an emergency.

The Applicant accepts and acknowledges this provision. Please see Sheet Z-7 in Exhibit "1". Only the FCC required site identification and emergency signage will be deployed.

9. Towers not requiring FAA painting or marking shall have an exterior finish which enhances compatibility with adjacent land uses, as approved by the Planning Commission or Board of Zoning Appeals. Towers shall not be lighted unless specifically required by the FAA.

The Applicant accepts and acknowledges this provision. Please see the FAA Determination of No Hazard to Air Navigation and the Ken Patterson Airspace Report attached hereto as Exhibit "6" and incorporated herein by reference. Due to its de minimis height, the FAA will not require illumination.

10. In order to protect the natural skyline, towers should be sited within areas of mature vegetation and should be located down slope from ridge lines, and toward the interior of the parcel whenever possible. Placement should only be considered elsewhere on the property when valid technical dates supplied by the applicant indicates that there is no other suitable location.

The Applicant accepts and acknowledges this provision. Please see the Balloon Test and Photo Simulations attached hereto as Exhibit "7" and incorporated herein by reference.

11. Towers proposed to be located within the Appalachian Trail corridor special planning area as identified in the adopted Comprehensive Plan for the County, any "AO" Antietam Overlay zoning district or "HP" Historic Preservation zoning district shall utilize stealth technology as defined in Article 28A to minimize visual impact.

The Applicant accepts and acknowledges this provision. As shown in Note 6 on sheet Z-1 of Exhibit "1", the proposed facility is not to be located within the Appalachian Trail Corridor Special Planning Area as identified in the adopted Comprehensive Plan for the County, any "AO" Antietam Overlay zoning district or "HP" Historic Preservation zoning district.

12. (a) A Commercial Communication Tower that is out of service for a continuous six (6) month period will be deemed to have been abandoned. The Zoning Administrator may issue a Notice of Abandonment to the Owner of the Tower that is deemed to be abandoned. The Owner shall have the right to respond in writing to the Notice of Abandonment setting forth the reasons for operation difficulty and providing a reasonable timeframe for correction action, within thirty (30) days from the date of the Notice. The Administrator shall withdraw the Notice of Abandonment and notify the Owner that the Notice has been withdrawn if the Owner provides information that demonstrates the Tower has not been abandoned.

The Applicant accepts and acknowledges this provision. Please see the Tower Removal Letter by Ryan Foltz of Arcola Towers attached hereto as Exhibit "8" and incorporated herein by reference.

(b) If the Tower is determined to be abandoned, the Owner of the Tower shall remove the Tower, and all related equipment at the Owner's sole expensed within three (3) months of the Date of Notice of Abandonment. If the Owner fails to remove the Tower and related equipment, the Administrator may pursue legal action to have the Tower removed at the Owner's expense.

The Applicant accepts and acknowledges this provision. Please see the Tower Removal Letter in Exhibit "8".

B. Additional Provisions for Towers Permitted by Special Exception

In addition to the limitations, guides and standards enumerated in Section 25.6, the Board of Zoning Appeals shall consider the following provisions when considering a request for a special exception for a commercial communications tower.

1. In those cases where a proposed tower is part of a grid or network, the applicant shall provide a map indicating the location of any existing or proposed towers in the grid or network within Washington County and within one (1) mile of the County boundary.

   Please see the Inventory Letter by Ryan Foltz of Arcola Towers and the RF Justification Letter by Shawn Boykins of Verizon Wireless and proposed coverage studies attached hereto as Exhibit "9" and incorporated herein by reference.

2. The tower shall be compatible with and shall not adversely impact the character and integrity of surrounding properties. Consideration shall be given to the view shed associated with scenic and historic areas and to the use of stealth technology to minimize the visibility of the proposed tower.

   Due to its strategic placement on the Miller property, the proposed facility shall not adversely impact the character and integrity of surrounding properties.

3. The applicant shall submit a visual analysis which may include, photo simulation, field mock-up, elevations or other visual or graphic illustrations to determine visual impact. Consideration shall be given to views from public areas as well as from private residences. The analysis shall assess the cumulative impacts of the proposed facility and other existing and foreseeable towers in the area and shall identify and include all feasible mitigation measures.

   Please see the Balloon Test and Photo Simulations in Exhibit "7".

4. The Board may include conditions on the site where the tower is to be located if such conditions are necessary to preserve the character and integrity of the area affected by the proposed tower and mitigate any adverse impacts which arise in connection with approval of the special exception.

   The Applicant accepts and acknowledges this provision.

In addition, please find the following: Survey by Maryland Professional Land Surveyor Fitzroy Jerry Bertrand attached hereto as Exhibit "10" and incorporated by reference; the Recorded Deed attached hereto as Exhibit "11" and incorporated herein by reference; and the Owners Representative Affidavit attached hereto as Exhibit "12" and incorporated herein by reference.

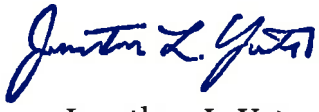
We would respectfully request to be placed on the agenda of the Washington County Board of Appeals April 15, 2026, scheduled meeting.

If you have any questions or concerns, please do not hesitate to contact me at 843-414-9754, or via email at jly@hellmanyates.com.

Thank you for all your help with this.

With warmest regards, I am

Yours very truly,



Jonathan L. Yates

Exhibit “1”

ARCOLA TOWERS
MD-003 WHITEHALL
10944 WHITE HALL ROAD
SMITHSBURG, MD 21783

187' MONOPOLE

entrex communication services, inc.
6100 EXECUTIVE BLVD.
SUITE 430
ROCKVILLE, MD 20852
PHONE: (202) 408-0960

arcola TOWERS
112 N. WASHINGTON ST
SUITE 201
MIDDLEBURG, VA 20117
PHONE: (571) 895 3990

SITE INFORMATION

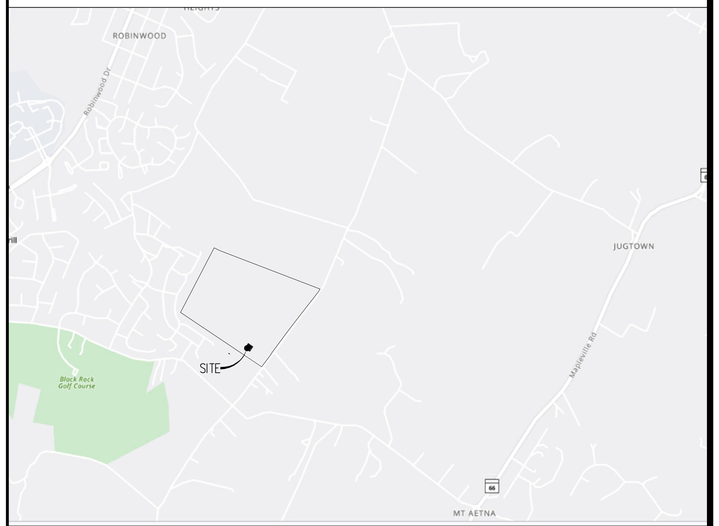
SCOPE OF WORK:

1. INSTALL EROSION AND SEDIMENT CONTROLS MEASURES
2. CLEAR AND GRADE SITE TO FINAL SUBGRADE ELEVATION
3. INSTALL MONOPOLE FOUNDATION AND MONOPOLE
4. INSTALL GROUNDING, TELCO HANDHOLES, UTILITY WIREWAY
5. INSTALL ELECTRICAL AND TELEPHONE CONDUITS AND HANDHOLES
6. INSTALL FENCE AND SITE IMPROVEMENTS

PROPERTY INFORMATION:

JURISDICTION:	WASHINGTON COUNTY, MD
ZONING:	A(R)
TAX ACCOUNT NUMBER:	18-013932
PARCEL AREA:	154.0000 ACRES
PARCEL OWNER:	MILLER, JAMES H & ELAINE K
OWNER ADDRESS:	12290 SCOTT RD, WAYNESBORO, PA 17268
STRUCTURE TYPE:	187' MONOPOLE + 2' LIGHTNING ROD
GROUND ELEVATION:	605.0' NAVD88
LATITUDE:	N 39° 36' 51.686" (N 39.614357°)
LONGITUDE:	W 77° 38' 45.768" (W 77.646047°)

VICINITY MAP



SCALE: 1" = 2,000'

MAGNETIC NORTH 10°.11' W — TRUE NORTH

SHEET INDEX

ARCOLA TOWERS
WHITEHALL
10944 WHITE HALL RD
SMITHSBURG, MD 21783

SEAL:

STATE OF MARYLAND
CAMILLE F. SHABSHAB
No. 17641
PROFESSIONAL ENGINEER

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 17641, Expiration Date: 12/27/2025

SUBMITTALS

DATE	DESCRIPTION	REV.
09-09-25	ZONING REVIEW	A
09-24-25	ZONING REVIEW	B
10-08-25	ZONING REVIEW	C
10-14-25	ZONING	1

PROJECT TEAM

APPLICANT:	ARCOLA TOWERS 112 W WASHINGTON STREET, SUITE 201 MIDDLEBURG, VA 20117 (571) 895 3990
ARCHITECT/ENGINEER:	ENTREX COMMUNICATION SERVICES, INC. 6100 EXECUTIVE BLVD, SUITE 430 ROCKVILLE, MD 20852 CAMILLE SHABSHAB (202) 408-0960

CODE COMPLIANCE

ALL WORK AND MATERIALS SHALL BE PERFORMED AND INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THE LATEST EDITIONS OF THE FOLLOWING CODES.

- 2021 INTERNATIONAL BUILDING CODE
- 2021 INTERNATIONAL EXISTING BUILDING CODE
- 2023 NATIONAL ELECTRICAL CODE
- 2018 NFPA 101 LIFE SAFETY CODE
- AMERICAN CONCRETE INSTITUTE
- AMERICAN INSTITUTE OF STEEL CONSTRUCTION
- ANSI/TIA-222-H

DRAWING APPROVALS

	SIGNATURE	DATE
OWNER REPRESENTATIVE		
SITE AQUISITION		
CONSTRUCTION MANAGER		
ZONING		
RF ENGINEER		

CALL
UTILITIES NOTIFICATION
MISS UTILITY
1-800-257-7777
3 WORKING DAYS PRIOR TO DIGGING

J:\Template Drawing\@11.dft

PROJECT NO: 1164.015
DESIGNER: R.S.
ENGINEER: C.S.

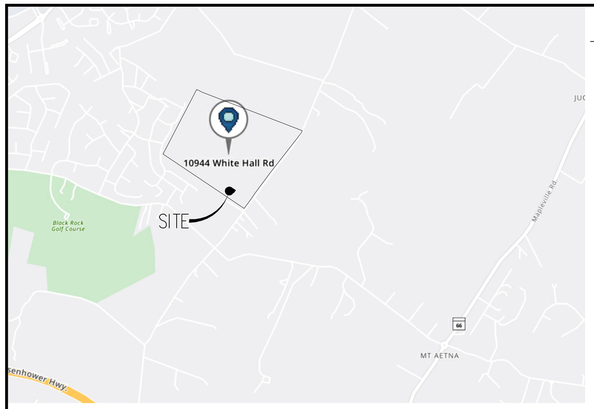
THESE DRAWINGS ARE FORMATTED TO BE FULL-SIZE AT 24"X36"
0 1/2 1
GRAPHIC SCALE IN INCHES

SHEET TITLE:

TITLE SHEET

SHEET NUMBER:

T-1



VICINITY MAP

SCALE: 1" = 2,000'-0"

TRUE NORTH

ZONING DRAWING NOTES

1. SITE NAME: ARCOLA TOWERS SITE – WHITEHALL

2. THIS IS NOT A BOUNDARY SURVEY AND IS NOT TO BE USED FOR THE TRANSFER OF PROPERTY.

3. THE SUBJECT PARCEL INFORMATION;

   OWNER: MILLER, JAMES H. & ELAINE K.
   PREMISES ADDRESS: 10944 WHITE HALL ROAD
   SMITHSBURG, MD 21783
   MAILING ADDRESS: 12290 SCOTT ROAD
   WAYNESBORO, PA 17268

   JURISDICTION: WASHINGTON COUNTY

   ACCOUNT NO.: 18-013932

   PARCEL: 0322, DEED REF: 01652/00685

   MAP 0050 GRID 0024

   AREA: 154 ACRES

4. THE LOCATION OF THE PROPOSED MONOPOLE IS AS FOLLOWS; THE VALUES LISTED BELOW ARE WITHIN ±50' HORIZONTAL AND ±20' VERTICAL.
   LATITUDE: ± N 39° 36' 51.686"
   LONGITUDE: ± W 77° 38' 45.768"
   ELEVATION: 605.0' NAVD88 AT MONOPOLE BASE

5. THERE IS NO FLOODPLAIN ON THE PROPERTY THAT IS THE SUBJECT OF THIS APPLICATION. THE CURRENT FLOOD INSURANCE RATE MAP (FIRM) OF WASHINGTON COUNTY, MD, COMMUNITY PANEL NUMBER FOR THE PROPERTY THAT IS THE SUBJECT OF THIS APPLICATION IS 24043C0310D, EFFECTIVE AUGUST 15, 2017.

6. THIS SITE IS NOT LOCATED WITHIN THE APPALACHIAN TRAIL CARTER SPECIAL PLANNING AREA, THE ANTIETAM OVERLAY ZONING DISTRICT OR THE HISTORIC PRESERVATION ZONING DISTRICT

7. THE SITE IS LOCATED MORE THAN 389' FROM ANY EXISTING OFFSITE DWELLING, SCHOOL, CHURCH, OR INSTITUTION FOR HUMAN CARE.

8. THE CLOSEST EXISTING TOWER IS A 205' AMERICAN TOWERS SELF-SUPPORT TOWER, LOCATED APPROXIMATELY 1.75 MI SOUTH.

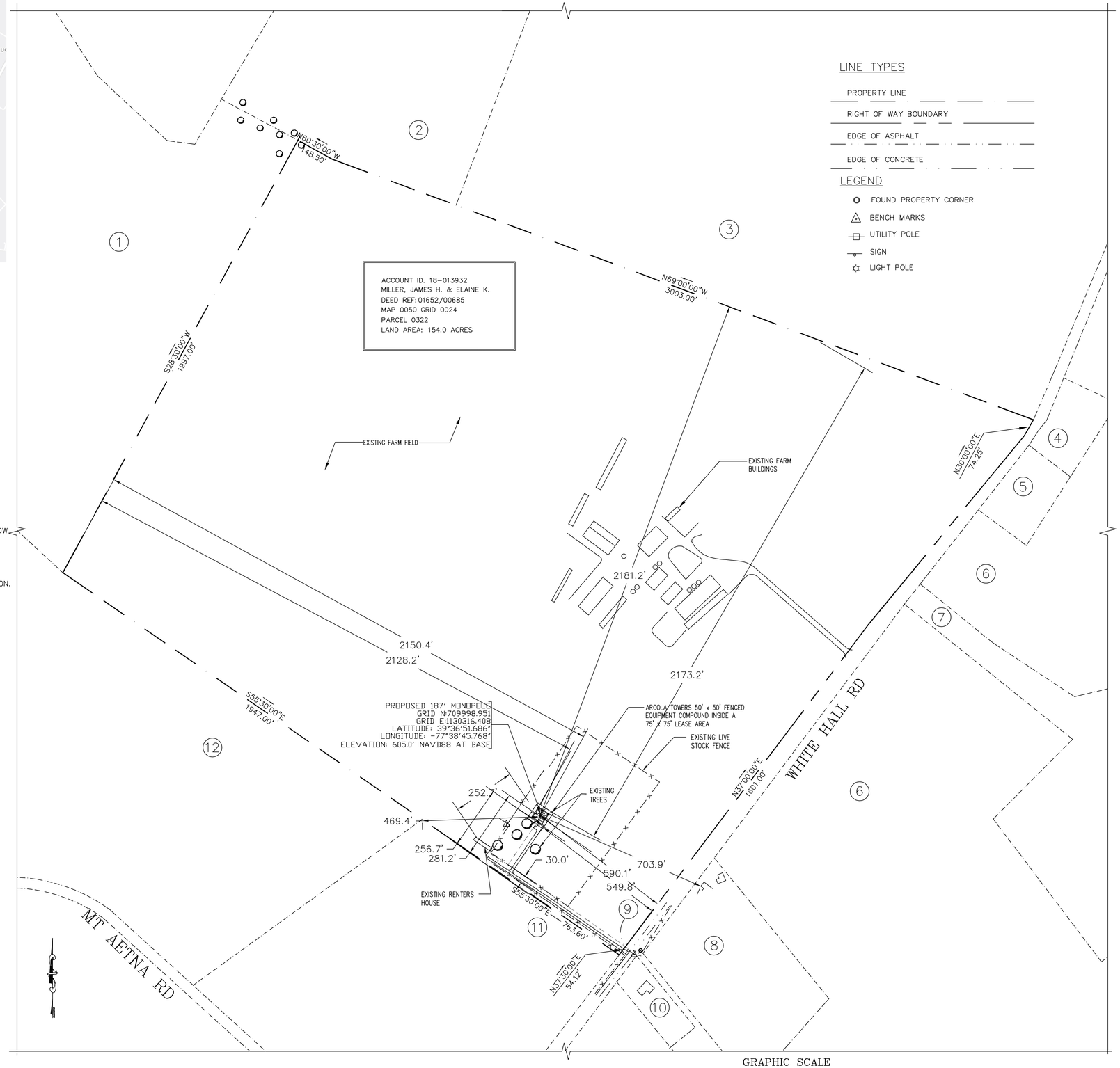
9. THERE ARE NO OVERHEAD TRANSMISSION LINES WITHIN TWO TIMES THE HEIGHT OF THE TOWER.

10. NO WETLANDS HAVE BEEN DEFINED AND ANY AREAS SHOWN AS MARSH, PONDS OR DITCHES ARE DONE SO FROM VISIBLE SURFACE FEATURES AND IN NO WAY CONSTITUTE A DEFINED WETLAND.

11. EXISTING HEAVY MATURE VEGETATION WILL BE UTILIZED IN LIEU OF NEW LANDSCAPING.

MONOPOLE SETBACKS		
	REQUIRED	PROPOSED
FRONT YARD (SOUTHEAST)	187'	590.1'
REAR YARD (NORTHWEST)	187'	2150.4'
SIDE YARD (SOUTHWEST)	187'	281.2'
SIDE YARD (NORTHEAST)	187'	2181.2'
CLOSEST OFF SITE RESIDENCE	187'	703.9'

COMPOUND SETBACKS		
	REQUIRED	PROPOSED
FRONT YARD (SOUTHEAST)	50'	549.8'
REAR YARD (NORTHWEST)	50'	21282'
SIDE YARD (SOUTHWEST)	50'	256.7'
SIDE YARD (NORTHEAST)	50'	2173.2'



GRAPHIC SCALE

(IN FEET)
1 inch = 200 ft.

SITE PLAN
SCALE: 1"= 200'

1 Z-1

TRUE NORTH

entrex communication services, inc.
6100 EXECUTIVE BLVD.
SUITE 430
ROCKVILLE, MD 20852
PHONE: (202) 408-0960

arcola TOWERS
112 N. WASHINGTON ST
SUITE 201
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SUBMITTALS

DATE	DESCRIPTION	REV.
09-09-25	ZONING REVIEW	A
09-24-25	ZONING REVIEW	B
10-08-25	ZONING REVIEW	C
10-14-25	ZONING	1

PROJECT NO: 1164.015
DESIGNER: R.S.
ENGINEER: C.S.

THESE DRAWINGS ARE FORMATTED TO BE FULL-SIZE AT 24"X36"
0 1/2 1
GRAPHIC SCALE IN INCHES

SHEET TITLE:

SITE PLAN

SHEET NUMBER:

Z-1

SUBJECT PARCEL
ACCOUNT ID: 18–013932
N/F
MILLER, JAMES H. & ELAINE K.
PARCEL ADDRESS: 10944 WHITE HALL RD
SMITHSBURG MD 21738
MAILING ADDRESS: 12290 SCOTT ROAD
WAYNESBORO, PA 17268
MAP/GRID/PARCEL: 0050/0024/0322
DEED BOOK 01652 PAGE 00685
USE: AGRICULTURAL
ZONING: A(R)
AREA: 154.00 AC

1.
ACCOUNT ID: 18–019493
N/F
DRB GROUP MID–ATLANTIC LLC
PARCEL ADDRESS: 11301 BOVEY LN
SMITHSBURG MD 21783
MAILING ADDRESS: 2099 GAITHER RD STE 600
ROCKVILLE, MD 20850
MAP/GRID/PARCEL: 0050/0017/0309
DEED BOOK 06871 PAGE 00421
USE: RESIDENTIAL
ZONING: RT
AREA: 153.45 AC

2.
ACCOUNT ID: 18–002264
N/F
TOOTHMAN RONALD G & TOOTHMAN COLLEEN M
PARCEL ADDRESS: 11350 KIEFFER FUNK RD
SMITHSBURG MD 21783
MAILING ADDRESS: P.O.BOX 185
CHEWSVILLE, MD 21721
MAP/GRID/PARCEL: 0051/0018/0307
DEED BOOK 000787 PAGE 00975
USE: AGRICULTURAL
ZONING: A(R)
AREA: 22.4 AC

3.
ACCOUNT ID:18–009315
N/F
MARTIN NATHAN R & MARTIN REBECCA
PARCEL ADDRESS: 11219 KIEFFER FUNK RD
SMITHSBURG MD 21783
MAILING ADDRESS: 11131 WHITE HALL RD
SMITHSBURG MD 21783
MAP/GRID/PARCEL: 0051/0018/0306
DEED BOOK 02887 PAGE 00635
USE: AGRICULTURAL
ZONING: A(R)
AREA: 145.63 AC

4.
ACCOUNT ID:18–042347
N/F
RIDENOUR HAROLD E JR L/E & RIDENOUR PEGGY J L/E
PARCEL ADDRESS: 11047 WHITE HALL RD
SMITHSBURG MD 21783
MAILING ADDRESS: 11047 WHITE HALL RD
SMITHSBURG MD 21783
MAP/GRID/PARCEL: 0051/0019/0718
DEED BOOK 07072 PAGE 00360
USE: RESIDENTIAL
ZONING: A(R)
AREA: 1.69 AC

5.
ACCOUNT ID:18–017121
N/F
RIDENOUR DAVID MATTHEW & RIDENOUR BETHANY DIANE
PARCEL ADDRESS: 11035 WHITE HALL RD
SMITHSBURG MD 21783
MAILING ADDRESS: 11035 WHITE HALL RD
SMITHSBURG MD 21783
MAP/GRID/PARCEL: 0050/0024/0323
DEED BOOK 06201 PAGE 00420
USE: RESIDENTIAL
ZONING: A(R)
AREA: 1.77 AC

6.
ACCOUNT ID:18–018195
N/F
SCRIVENER THOMAS F
PARCEL ADDRESS: 10901 WHITE HALL RD
SMITHSBURG MD 21783
MAILING ADDRESS: 10901 WHITE HALL RD
SMITHSBURG MD 21783
MAP/GRID/PARCEL: 0051/0019/0061
DEED BOOK 00509 PAGE 00499
USE: AGRICULTURAL
ONING: A(R)
AREA: 120.51 AC

7.
ACCOUNT ID:18–040344
N/F
SCRIVENNER JOHN T & SCRIVENER LISA M
PARCEL ADDRESS: 11005 WHITE HALL RD
SMITHSBURG, MD 21783
MAILING ADDRESS: 11005 WHITE HALL RD
SMITHSBURG, MD 21783
MAP/GRID/PARCEL: 0050/0024/1699
DEED BOOK 01225 PAGE 00203
USE: AGRICULTURAL
ZONING: A(R)
AREA: 15.7 AC

8.
ACCOUNT ID: 18–014831
N/F
NEEDY PAUL E
PARCEL ADDRESS: 10863 WHITE HALL RD
SMITHSBURG MD 21783
MAILING ADDRESS: 10863 WHITE HALL RD
SMITHSBURG, MD 21783
MAP/GRID/PARCEL: 0050/0024/0324
DEED BOOK 03779 PAGE 00209
USE: AGRICULTURAL
ZONING: A(R)
AREA: 8.41 AC

9.
ACCOUNT ID: 18–040387
N/F
FERGUSON MICHAEL T &
FERGUSON LISA ANN
PARCEL ADDRESS: 10853 WHITE HALL RD
SMITHSBURG MD 21783
MAILING ADDRESS: 10849 WHITE HALL RD
SMITHSBURG, MD 21783
MAP/GRID/PARCEL: 0050/0024/1703
DEED BOOK 01414 PAGE 00755
USE: RESIDENTIAL
ZONING: A(R)
AREA: 1.48 AC

10.
ACCOUNT ID: 18–028328
N/F
FERGUSON MICHAEL T &
FERGUSON LISA A
PARCEL ADDRESS: 10849 WHITE HALL RD
SMITHSBURG MD 21783
MAILING ADDRESS: 10849 WHITE HALL RD
SMITHSBURG MD 21783
MAP/GRID/PARCEL: 0050/0024/1649
DEED BOOK 01072 PAGE 01021
USE: RESIDENTIAL
ZONING: A(R)
AREA: 40,336 SF

11.
ACCOUNT ID: 16–008664
N/F
PRYOR DAVID ET AL &
PRYOR KENNETH
PARCEL ADDRESS: MT AETNA RD
HAGERSTOWN, MD 21742
MAILING ADDRESS: 20615 MT AETNA
HAGERSTOWN, MD 21742
MAP/GRID/PARCEL: 0050/0023/0001
DEED BOOK 06275 PAGE 00017
USE: AGRICULTURAL
ZONING: A(R)
AREA: 70.29 AC

12.
ACCOUNT ID: 18–019507
N/F
DRB GROUP MID–ATLANTIC LLC
PARCEL ADDRESS: MT AETNA RD
HAGERSTOWN MD 21742
MAILING ADDRESS: 2099 GAITHER RD STE 600
ROCKVILLE, MD 20850
MAP/GRID/PARCEL: 0050/0023/0321
DEED BOOK 07544 PAGE 00155
USE: RESIDENTIAL
ZONING: RT
AREA: 63.97 AC

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10–14–25	ZONING	1

PROJECT NO: 1164.015
DESIGNER: R.S.
ENGINEER: C.S.
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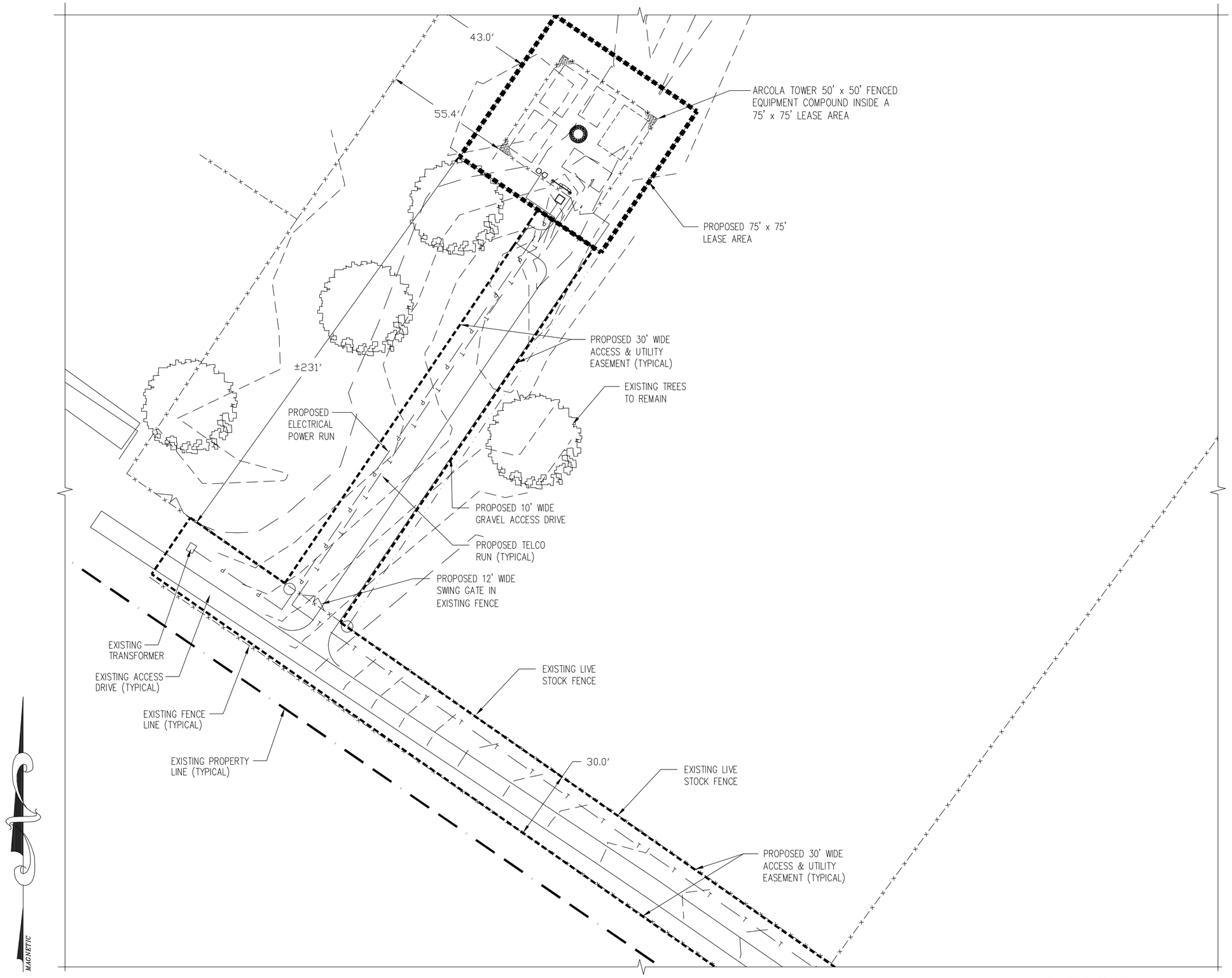
0 1/2 1
GRAPHIC SCALE IN INCHES
SHEET TITLE:

ADJOINERS

SHEET NUMBER:

Z-2



PARTIAL SITE PLAN
SCALE: 1"= 20'

1 / Z-3

TRUE NORTH

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PROJECT NO: 1164.015
DESIGNER: R.S.
ENGINEER: C.S.

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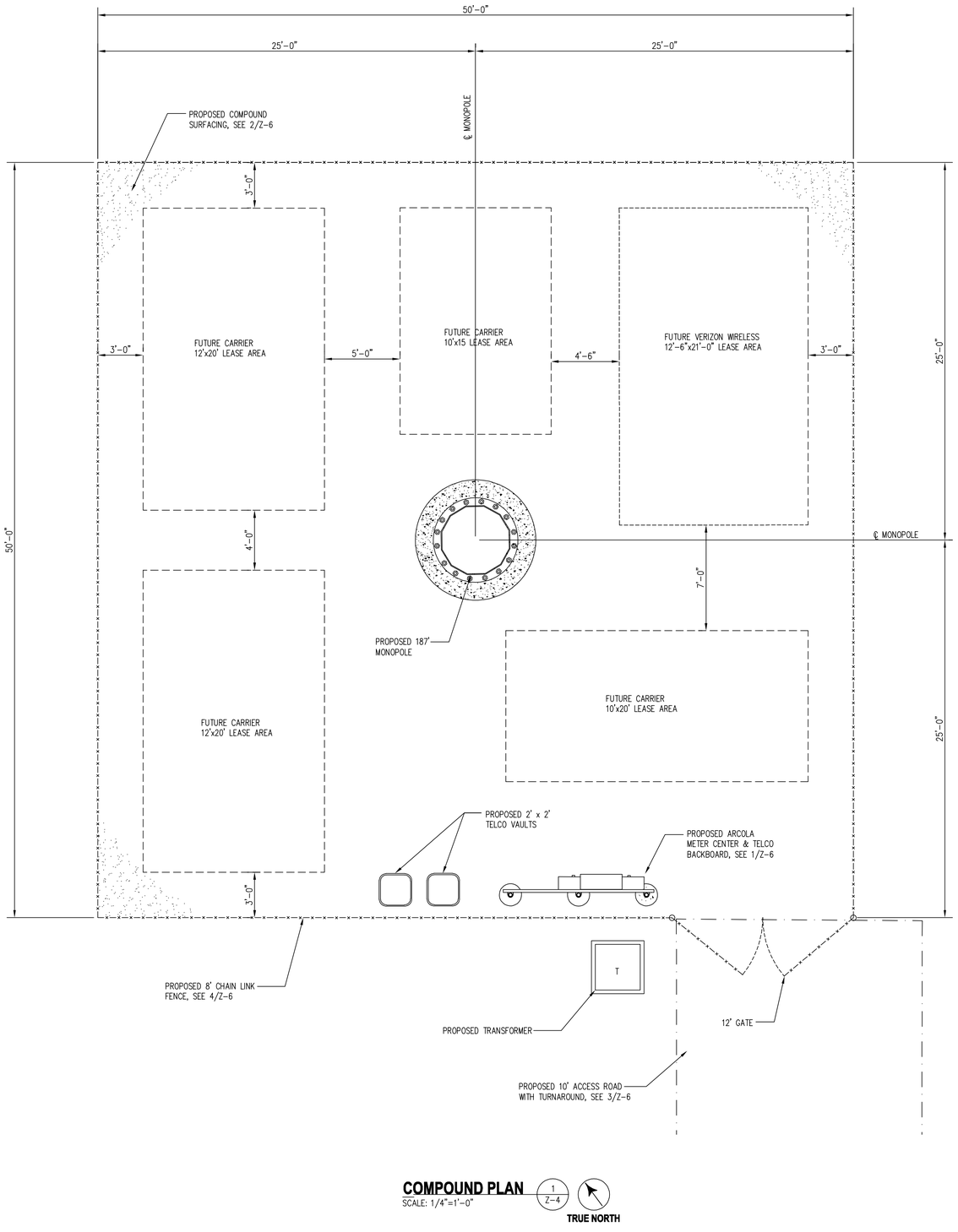
0 1/2 1
GRAPHIC SCALE IN INCHES

SHEET TITLE:

PARTIAL SITE PLAN

SHEET NUMBER:

Z-3

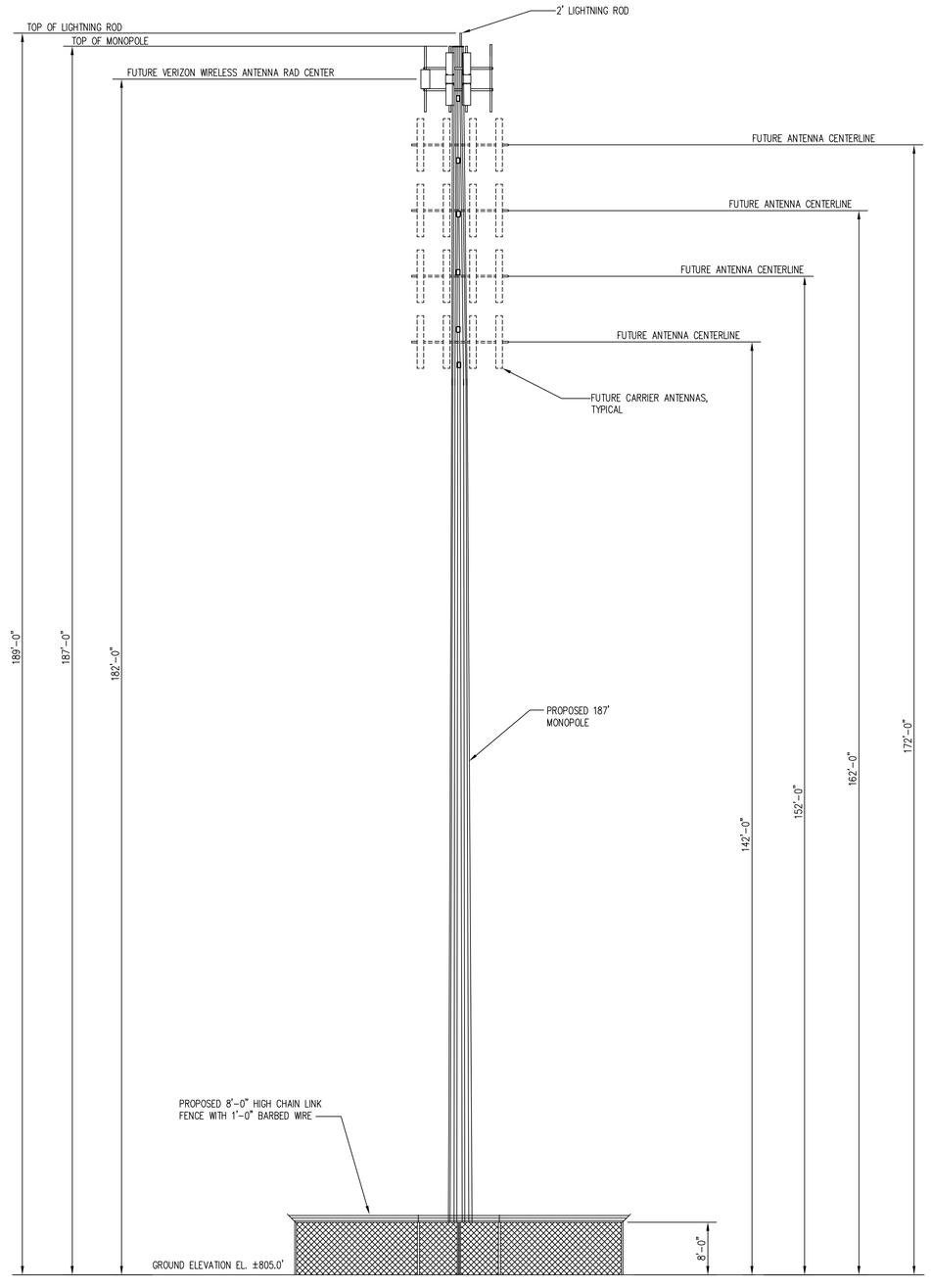


COMPOUND PLAN (1 / Z-4)
SCALE: 1/4"=1'-0"

TRUE NORTH

DESIGN NOTES

1. PROPOSED ANTENNA, ANTENNAS ARRAYS, CABLES, AND OTHER MONOPOLE ATTACHMENTS WILL BE PAINTED GRAY TO BETTER BLEND WITH GALVANIZED METAL MONOPOLE AND FURTHER MINIMIZE THE VISUAL IMPACT.
2. THERE IS NO ILLUMINATION PROPOSED FOR THE MONOPOLE.



MONOPOLE ELEVATION (2 / Z-4)
SCALE: 3/32"=1'-0"

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10-14-25	ZONING	1

PROJECT NO:	1164.015
DESIGNER:	R.S.
ENGINEER:	C.S.

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0 1/2 1
GRAPHIC SCALE IN INCHES

SHEET TITLE:

COMPOUND PLAN & MONOPOLE ELEVATION

SHEET NUMBER:

Z-4

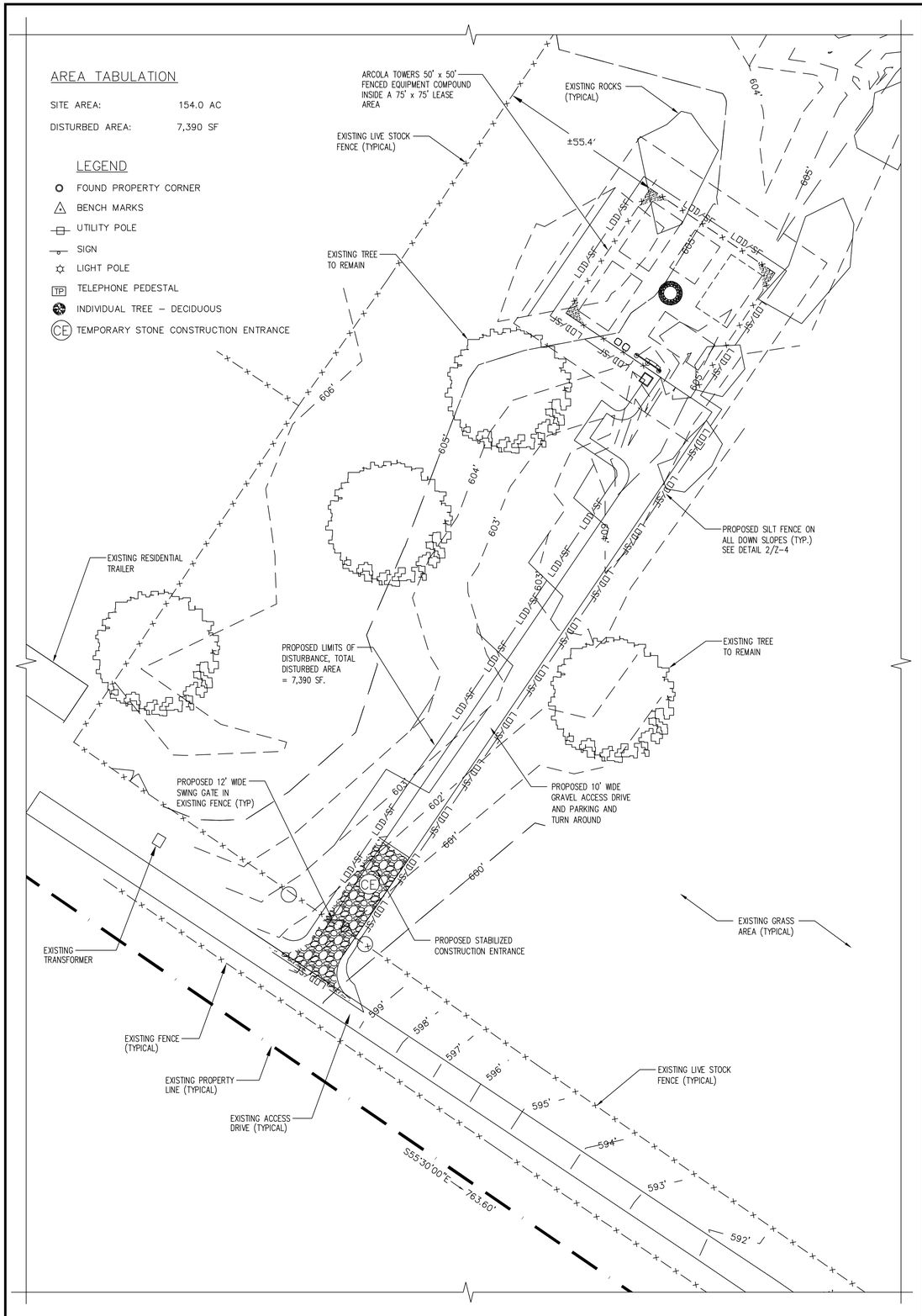
AREA TABULATION

SITE AREA: 154.0 AC

DISTURBED AREA: 7,390 SF

LEGEND

- FOUND PROPERTY CORNER
- BENCH MARKS
- UTILITY POLE
- SIGN
- LIGHT POLE
- TP — TELEPHONE PEDESTAL
- INDIVIDUAL TREE – DECIDUOUS
- CE — TEMPORARY STONE CONSTRUCTION ENTRANCE



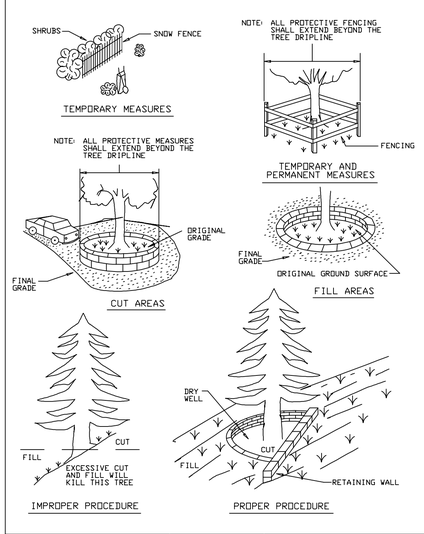
EROSION AND SEDIMENT CONTROL PLAN

SCALE: 1" = 20'

1 / Z-5

TRUE NORTH

DETAIL 31 - TREE PROTECTION



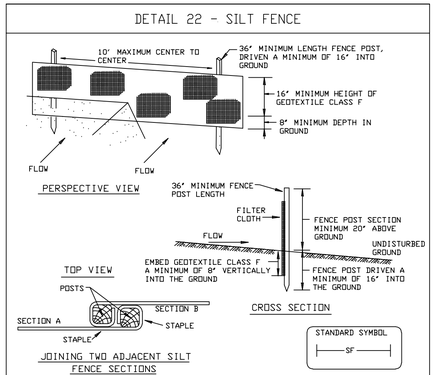
U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE	PAGE G - 23 - 3	MARYLAND DEPARTMENT OF ENVIRONMENT WATER MANAGEMENT ADMINISTRATION

TREE PROTECTION FENCE DETAIL

SCALE: N.T.S.

2 / Z-5

DETAIL 22 - SILT FENCE



Construction Specifications

1. Fence posts shall be a minimum of 36" long driven 16" minimum into the ground. Wood posts shall be 11/2" x 11/2" square (minimum) cut, or 13/4" diameter (minimum) round and shall be of sound quality hardwood. Steel posts will be standard T or U section weighting not less than 1.00 pond per linear foot.

2. Geotextile shall be fastened securely to each fence post with wire ties or staples at top and mid-section and shall meet the following requirements for Geotextile Class F:

Tensile Strength	50 lbs/in (min.)	Test: MSMT 509
Tensile Modulus	20 lbs/in (min.)	Test: MSMT 509
Flow Rate	0.3 gal/ft²/ minute (max.)	Test: MSMT 322
Filtering Efficiency	75% (min.)	Test: MSMT 322

3. Where ends of geotextile fabric come together, they shall be overlapped, folded and stapled to prevent sediment bypass.

4. Silt fence shall be inspected after each rainfall event and maintained when bulges occur or when sediment accumulation reached 50% of the fabric height.

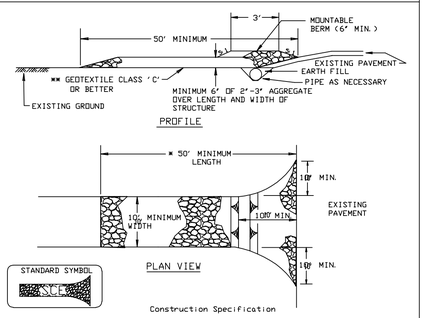
U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE	PAGE E - 15 - 3	MARYLAND DEPARTMENT OF ENVIRONMENT WATER MANAGEMENT ADMINISTRATION

SILT FENCE DETAIL

SCALE: N.T.S.

3 / Z-5

DETAIL 24 - STABILIZED CONSTRUCTION ENTRANCE



Construction Specification

1. Length - minimum of 50' (*30' for single residence lot).

2. Width - 10' minimum, should be flared at the existing road to provide a turning radius.

3. Geotextile Fabric (Filter cloth) shall be placed over the existing ground prior to placing stone. **The plan approval authority may not require single family residences to use geotextile.

4. Stone - crushed aggregate (2" to 3") or reclaimed or recycled concrete equivalent shall be placed at least 6" deep over the length and width of the entrance.

5. Surface Water - all surface water flowing to or diverted toward construction entrances shall be piped through the entrance, maintaining positive drainage. Pipe installed through the stabilized construction entrance shall be protected with a mountable berm with 5:1 slopes and a minimum of 6" of stone over the pipe. Pipe has to be sized according to the drainage. When the SCE is located at a high spot and has no drainage to convey a pipe will not be necessary. Pipe should be sized according to the amount of runoff to be conveyed. A 6" minimum will be required.

6. Location - A stabilized construction entrance shall be located at every point where construction traffic enters or leaves a construction site. Vehicles leaving the site must travel over the entire length of the stabilized construction entrance.

U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE	PAGE F - 17 - 3	MARYLAND DEPARTMENT OF ENVIRONMENT WATER MANAGEMENT ADMINISTRATION

STABILIZED CONSTRUCTION ENTRANCE DETAIL

SCALE: N.T.S.

4 / Z-5

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PHONE: (571) 895 3990

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SEAL:

STATE OF MARYLAND — CAMILLE F. STANDAHAR — No. 17641 — PROFESSIONAL ENGINEER

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10-14-25	ZONING	1

PROJECT NO: 1164.015

DESIGNER: R.S.

ENGINEER: C.S.

THESE DRAWINGS ARE FORMATTED TO BE FULL-SIZE AT 24"X36"

0 1/2 1

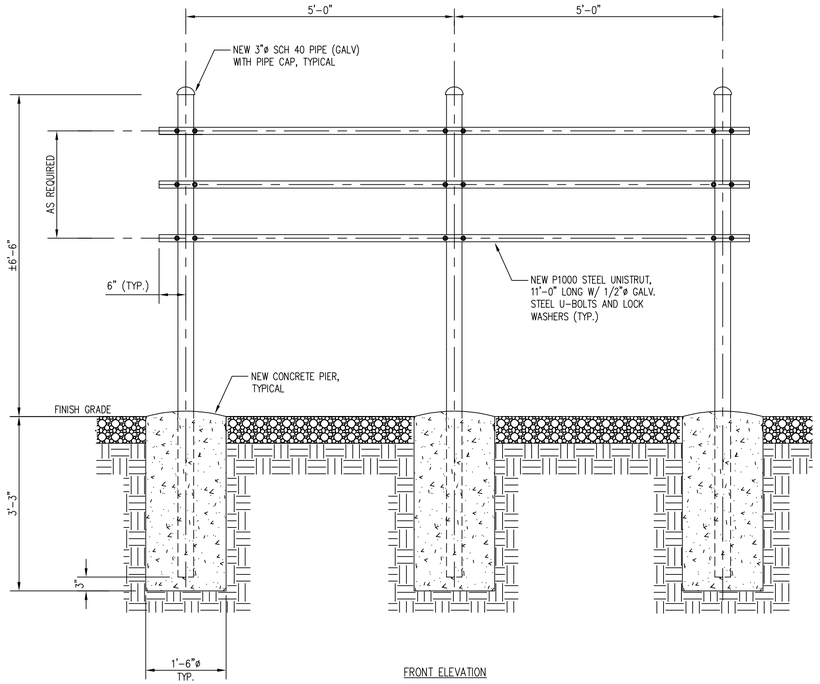
GRAPHIC SCALE IN INCHES

SHEET TITLE:

EROSION AND SEDIMENT CONTROL PLAN

SHEET NUMBER:

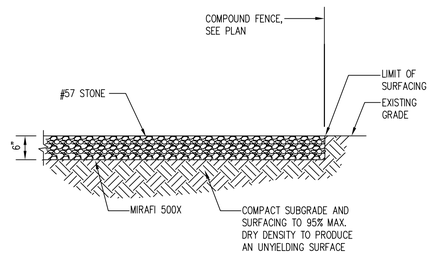
Z-5



UTILITY FRAME DETAIL

SCALE: 3/4"=1'-0"

1 / Z-6

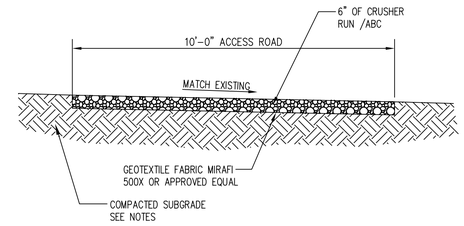


NOTE:
COMPOUND AREA SHALL BE CLEARED AND GRUBBED. REMOVE UNSUITABLE LOOSE OR SOFT SOIL, ORGANIC MATERIAL OR RUBBLE TO FIRM GRADE. FILL UNDERCUT AND COMPACT UP TO 6" BELOW FINISH GRADE. PLACE A MIRAFI 500X SOIL STABILIZATION FABRIC ON SUBGRADE. FILL WITH 6" OF AASHTO 57 STONE TO FINISH GRADE.

COMPOUND SURFACING DETAIL

SCALE: 1"=1'-0"

2 / Z-6



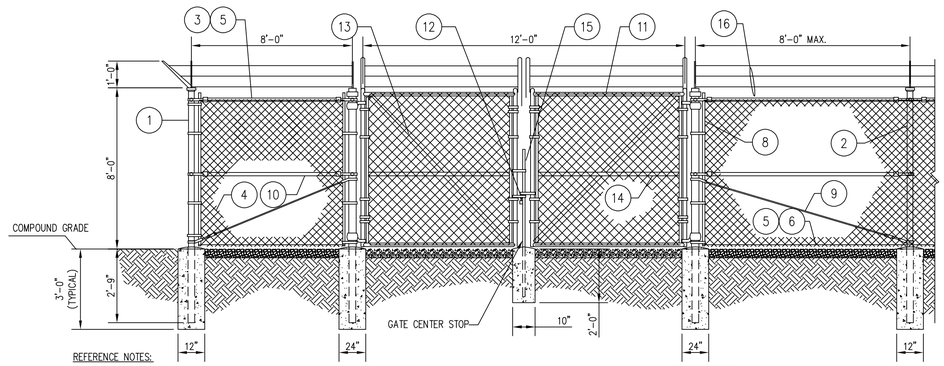
ACCESS ROAD:
FILL, UNDERCUT AND COMPACT TEMPORARY ACCESS ROAD UP TO 6" BELOW FINISH GRADE. PLACE MIRAFI, 500X STABILIZATION FABRIC ON SUBGRADE. PULL TIGHT AND STAKE IN PLACE. PLACE AND COMPACT 6" DEPTH OF CRUSHER RUN/ABC TO FINISH GRADE

COMPACTION NOTES:
1. SURFACE COURSE AND BASE COURSE SHALL BE COMPACTED TO 98% MAX. DRY DENSITY STANDARD PROCTOR.
2. SUBGRADE SOIL SHALL BE COMPACTED TO 95% MAX. DRY DENSITY STANDARD PROCTOR.

GRAVEL ACCESS ROAD DETAIL

SCALE: N.T.S.

3 / Z-6



REFERENCE NOTES:

1. GATE POST: 6" NOMINAL SCHEDULE 40 PIPE. CORNER, END OR PULL POST: 4" NOMINAL SCHEDULE 40 PIPE.
2. LINE POST: 3" SCHEDULE 40 PIPE, PER ASTM-F1083. LINE POSTS SHALL BE EQUALLY SPACED AT MAXIMUM 6'-0" O.C.
3. TOP RAIL & BRACE RAIL: 1 1/2" PIPE, PER ASTM-F1083.
4. FABRIC: 9 GA CORE WIRE SIZE 2" MESH, CONFORMING TO ASTM-A392.
5. TIE WIRE: MINIMUM 11 GA GALVANIZED STEEL AT POSTS AND RAILS A SINGLE WRAP OF FABRIC TIE AND AT TENSION WIRE BY HOG RINGS SPACED MAX. 24" INTERVALS.
6. TENSION WIRE: 9 GA. GALVANIZED STEEL.
7. NOT USED
8. STRETCHER BAR.
9. 3/8" DIAGONAL ROD WITH GALVANIZED STEEL TURNBUCKLE OR DIAGONAL THREADED ROD.
10. FENCE CORNER POST BRACE: 1 5/8" DIA. EACH CORNER EACH WAY.
11. GATE FRAME: 1 1/2" PIPE, PER ASTM-F1083.
12. NOT USED
13. NOT USED
14. GATE FRAME BRACE: 1 5/8" DIAMETER.
15. CENTER GATE STOP
16. BARBED WIRE: DOUBLE STRAND 12-1/2" O.D. TWISTED WIRE TO MATCH WITH FABRIC 14 GA, 4 PT. BARBS SPACED ON APPROXIMATELY 5" CENTERS.

GENERAL NOTES:

1. INSTALL FENCING PER ASTM F-567
2. INSTALL SWING GATES PER ASTM F-900
3. FENCE PIPE AND COMPONENTS SHALL BE GALVANIZED.
4. GATE FRAMES SHALL BE WELDED. WELDING SHALL BE COATED WITH (3) COATS OF COLD GALV. (OR EQUAL).
5. POSTS SHALL HAVE END-CAPS.
6. GATES SHALL HAVE LOCKING HARDWARE.
7. PROVIDE GATE STOPS TO SECURE GATES IN OPEN POSITION.

GALVANIZED STEEL FENCE AND GATE DETAIL

SCALE: NTS

4 / Z-6

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ENGINEER: C.S.

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0 1/2 1
GRAPHIC SCALE IN INCHES

SHEET TITLE:

SITE DETAILS

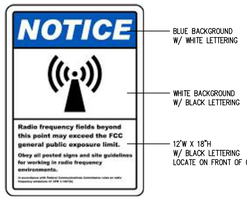
SHEET NUMBER:

Z-6

SITE SIGNAGE DETAILS

WHITE BACK GROUND
W/ BLACK LETTERING
(SITE ID SIGN SHALL NOT EXCEED 6 SF)

MARKETING / ID SIGN (1 / Z-7)
SCALE: N.T.S.

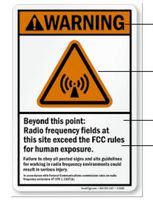


BLUE BACKGROUND
W/ WHITE LETTERING

WHITE BACKGROUND
W/ BLACK LETTERING

12"W X 18"H
W/ BLACK LETTERING
LOCATE ON FRONT OF GATE

RF NOTICE SIGN (3 / Z-7)
SCALE: N.T.S.



ORANGE BACKGROUND
W/ BLACK LETTERING

ORANGE BACKGROUND
W/ BLACK LETTERING

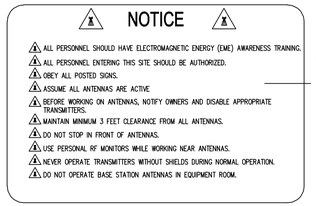
WHITE BACKGROUND
W/ BLACK LETTERING

12"W X 18"H
LOCATE AT BASE OF TOWER

RF WARNING SIGN (5 / Z-7)
SCALE: N.T.S.

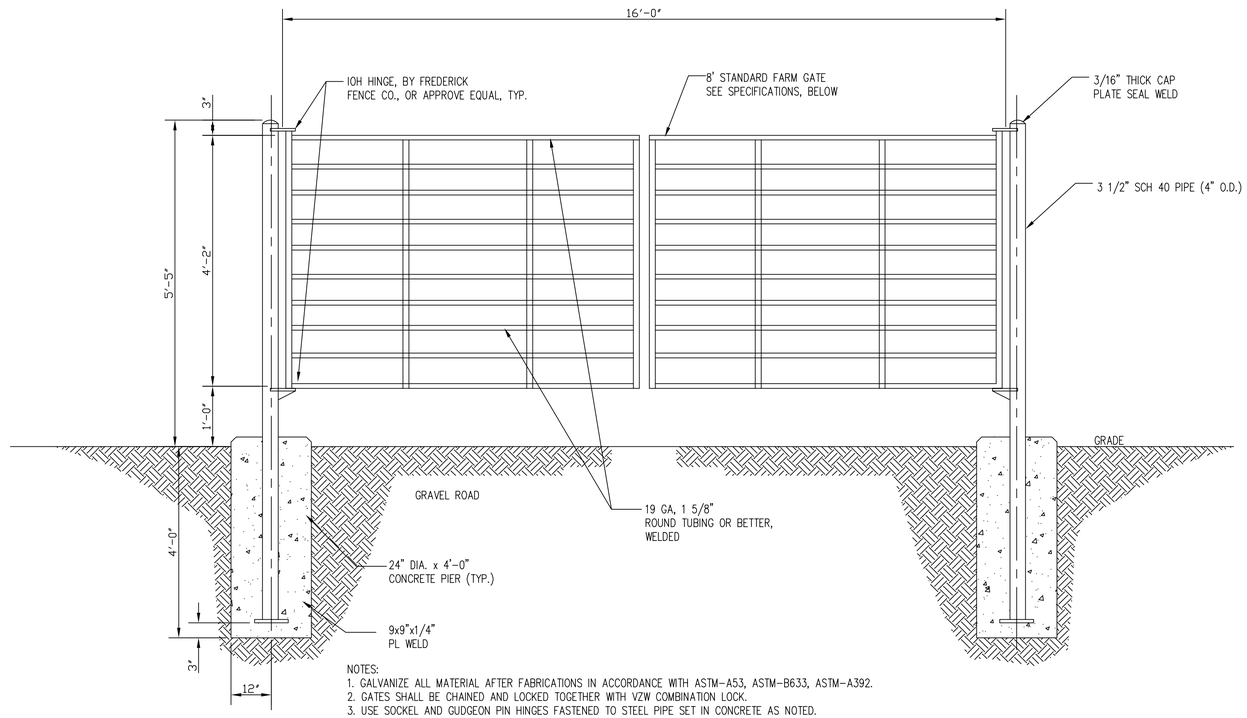
WHITE BACKGROUND
W/ RED LETTERING
18"H X 24"W

NO TRESPASSING SIGN (2 / Z-7)
SCALE: N.T.S.



ORANGE BACKGROUND
W/ BLACK LETTERING
12"W X 18"H

RF NOTICE SIGN 2 (4 / Z-7)
SCALE: N.T.S.



NOTES:
1. GALVANIZE ALL MATERIAL AFTER FABRICATIONS IN ACCORDANCE WITH ASTM-A53, ASTM-B633, ASTM-A392.
2. GATES SHALL BE CHAINED AND LOCKED TOGETHER WITH VZW COMBINATION LOCK.
3. USE SOCKEL AND GUDGEON PIN HINGES FASTENED TO STEEL PIPE SET IN CONCRETE AS NOTED.
4. WELDED STEEL PROOF COIL CHAIN 1/4" GRADE 80 CUT TO LENGTH.

GATE DETAIL (5 / Z-7)
SCALE: NTS

entrex
communication services, inc.
6100 EXECUTIVE BLVD.
SUITE 430
ROCKVILLE, MD 20852
PHONE: (202) 408-0960

arcola TOWERS
112 N. WASHINGTON ST
SUITE 201
MIDDLEBURG, VA 20117
PHONE: (571) 895 3990

ARCOLA TOWERS
WHITEHALL
10944 WHITE HALL RD
SMITHSBURG, MD 21783

SEAL:
STATE OF MARYLAND · CAMILLE F. SHABAB · No. 17641 · PROFESSIONAL ENGINEER

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 17641, Expiration Date: 12/27/2025

SUBMITTALS

DATE	DESCRIPTION	REV.
09-09-25	ZONING REVIEW	A
09-24-25	ZONING REVIEW	B
10-08-25	ZONING REVIEW	C
10-14-25	ZONING	1

PROJECT NO: 1164.015
DESIGNER: R.S.
ENGINEER: C.S.

THESE DRAWINGS ARE FORMATTED TO BE FULL-SIZE AT 24"X36"
0 1/2 1
GRAPHIC SCALE IN INCHES

SHEET TITLE:

SIGNAGE DETAILS AND GATE DETAIL

SHEET NUMBER:

Z-7

Exhibit “2”

Arcola Towers: Whitehall

Alternative Candidate Analysis

Arcola Towers submits this document to address the alternative candidates it considered during its selection process. Verizon has a specific search ring in the Whitehall Road area in Washington County, Maryland, with a requested antenna centerline of 182 ft.

Existing Structures in the Search Ring

Arcola Towers was able to confirm that there are no existing structures within the search ring. There are no structures at 182 ft. in height within the specified search ring, nor within a reasonable distance outside of the search ring. Arcola Towers utilized existing databases (Tower Source, etc.) to determine the existence of nearby towers while confirming that information via driving in and around the search ring area.

Existing Structures outside of the Search Ring

Arcola Towers then located the nearest existing tower locations as shown on the attached map in a 1-mile radius:

This map illustrates a 1-mile radius from the proposed search ring center, and as shown on the map, there are no existing towers. The closest tower is 1.29 miles to the Southeast of the search ring and will not cover the intended area.

Conclusion

As noted above, there are no existing structures within the search area, and as noted above, the closest existing tower to the Search Ring Center is a 195' self-support American Tower site that is 1.29 miles away and too far to cover Verizon's intended coverage area. Arcola has leased a property from a willing landlord whose property will be developed in accordance with Washington County regulations.

I certify that the foregoing is true and correct:

Ryan Foltz

Ryan Foltz on behalf of Arcola Towers

Exhibit “3”

sabre

Structural Design Report

187' Monopole

Site: MD-003 Whitehall, MD

Prepared for: ARCOLA TOWERS

by: Sabre Industries ™

Job Number: 26-2535-TLJ

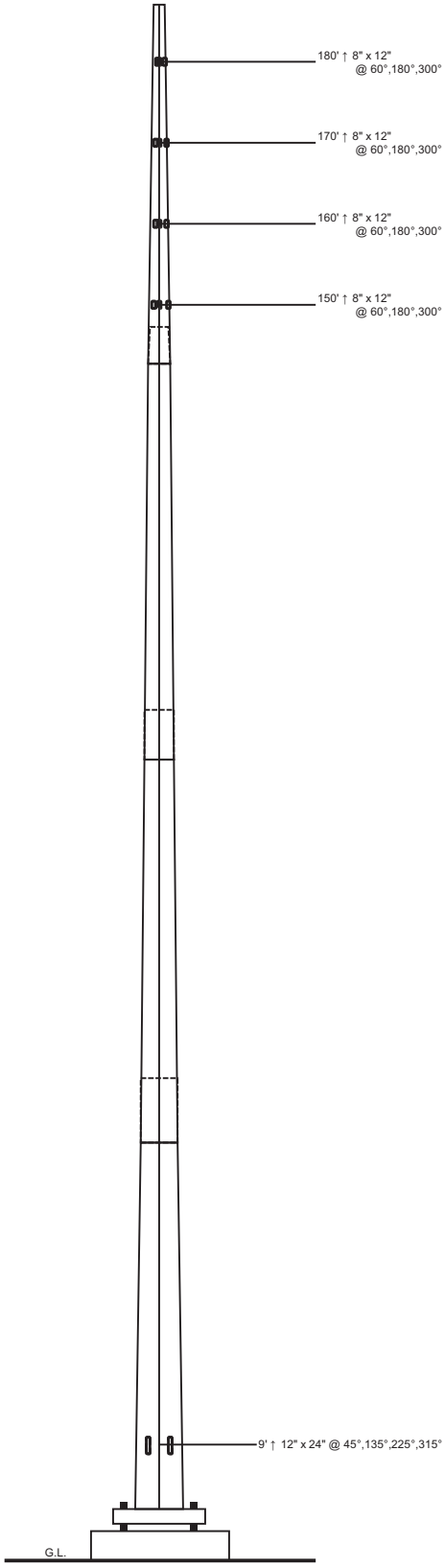
October 24, 2025

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 40946, Expiration Date: 8/9/2027

10/24/25

Digitally Signed By Robert Beacom
DN: C=US,SERIALNUMBER=MAS20250205984402,ST=Texas,L=Alvarado,2.5.4.97=NTRUS\+DE-4349737,O=SABRE INDUSTRIES\, INC.,CN=Robert Beacom Date: 2025.10.24 13:22:58

Length (ft)	53'-3"	53'-6"	53'-6"	44'-6"						
Number Of Sides	18									
Thickness (in)	7/16"	3/8"	1/4"							
Lap Splice (ft)	8' - 0"	6' - 3"	A							
Top Diameter (in)	53.74"	41.98"	29.61"	19"						
Bottom Diameter (in)	68.53"	56.84"	44.46"	31.36"						
Taper (in/ft)	0.2777									
Grade	A572-65									
Weight (lbs)	18347	13132	8434	3722						
Overall Steel Height (ft)	186									



Designed Appurtenance Loading

Elev	Description	Tx-Line
187	(1) 2 sq. ft. EPA	
182	(1) 250 Sq.Ft. EPA (7000 lbs)	(6) 1 5/8"
172	(1) 200 sq. ft. EPA, 4,500 lb Weight	(6) 1 5/8"
162	(1) 175 Sq. Ft. EPA (4,500 lbs)	(6) 1 5/8"
152	(1) 150 sq. ft. EPA, 4,500 lb Weight	(6) 1 5/8"

Design Criteria - ANSI/TIA-222-H

Wind Speed (No Ice)	112 mph
Wind Speed (Ice)	40 mph
Design Ice Thickness	1.00 in
Risk Category	II
Exposure Category	C
Topographic Factor Procedure	Method 1 (Simplified)
Topographic Category	1
Ground Elevation	604 ft
Seismic Importance Factor, Ie	1.00
0.2-sec Spectral Response, Ss	0.124 g
1-sec Spectral Response, S1	0.042 g
Site Class	D (DEFAULT)
Seismic Design Category	B
Basic Seismic Force-Resisting System	Telecommunication Tower (Pole: Steel)

Limit State Load Combination Reactions

Load Combination	Axial (kips)	Shear (kips)	Moment (ft-k)	Deflection (ft)	Sway (deg)
1.2 D + 1.0 Wo	76.12	53.73	8054.65	17.04	10.96
0.9 D + 1.0 Wo	57.19	53.18	7848.28	16.57	10.64
1.2 D + 1.0 Di + 1.0 Wi	111.55	10.81	1676.58	3.71	2.38
1.2 D + 1.0 Ev + 1.0 Eh	77.75	1.91	316.36	0.71	0.45
0.9 D - 1.0 Ev + 1.0 Eh	55.39	1.9	310.55	0.69	0.44
1.0 D + 1.0 Wo (Service @ 60 mph)	63.52	13.73	2056.21	4.47	2.83

Base Plate Dimensions

Shape	Diameter	Thickness	Bolt Circle	Bolt Qty	Bolt Diameter
Round	81.5"	2.25"	75.75"	22	2.25"

Anchor Bolt Dimensions

Length	Diameter	Hole Diameter	Weight	Type	Finish
84"	2.25"	2.625"	2664.2	A615-75	Galv

Material List

Display	Value
A	4' - 6"

Notes

1) Antenna Feed Lines Run Inside Pole
2) All dimensions are above ground level, unless otherwise specified.
3) Weights shown are estimates. Final weights may vary.
4) Full Height Step Bolts
5) This tower design and, if applicable, the foundation design(s) shown on the following page(s) also meet or exceed the requirements of the 2021 International Building Code.
6) Tower Rating: 99.4%

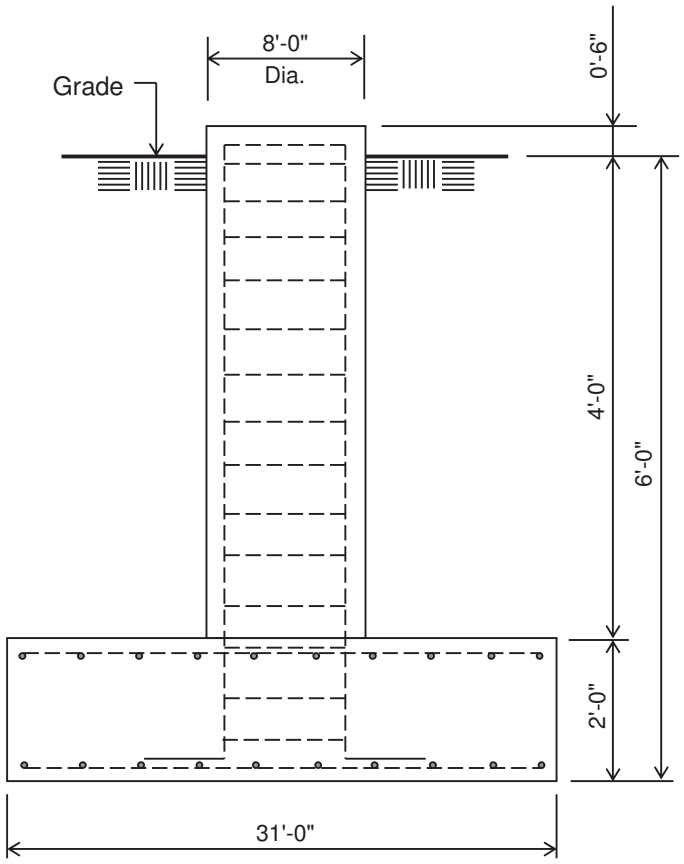
Sabre Industries
7101 Southbridge Drive
P.O. Box 658
Sioux City, IA 51102-0658
Phone: (712) 258-6690
Fax: (712) 279-0814

Job:	**26-2535-TLJ**
Customer:	ARCOLA TOWERS
Site Name:	MD-003 Whitehall, MD
Description:	187' Monopole
Date:	10/24/2025 By: TTW

No.: 26-2535-TLJ
Date: 10/24/25
By: TTW

Customer: ARCOLA TOWERS
Site: MD-003 Whitehall, MD
187' Monopole

PRELIMINARY -NOT FOR CONSTRUCTION-



ELEVATION VIEW
(79.56 Cu. Yds.)
(1 REQUIRED; NOT TO SCALE)

Notes:

1) Concrete shall have a minimum 28-day compressive strength of 4,500 psi, in accordance with ACI 318-14.

2) Rebar to conform to ASTM specification A615 Grade 60.

3) All rebar to have a minimum of 3" concrete cover.

4) All exposed concrete corners to be chamfered 3/4".

5) The foundation design is based on presumptive clay soil as defined in ANSI/TIA-222-H-2017. It is recommended that a soil analysis of the site be performed to verify the soil parameters used in the design.

6) 4 ft of soil cover is required over the entire area of the foundation slab.

7) The bottom anchor bolt template shall be positioned as closely as possible to the bottom of the anchor bolts.

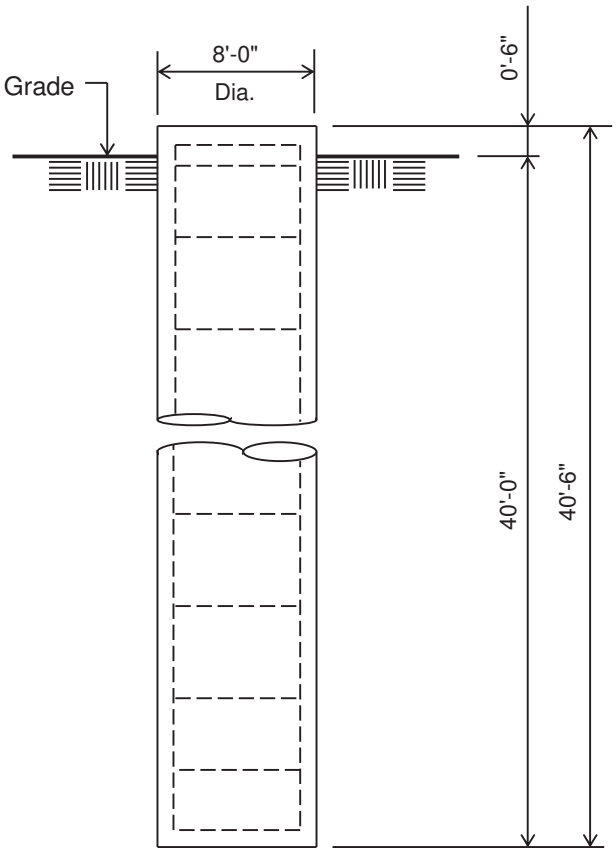
Rebar Schedule for Pad and Pier	
Pier	(54) #9 vertical rebar w/ hooks at bottom w/ #5 ties, (2) within top 5" of pier, then 4" C/C
Pad	(53) #10 horizontal rebar evenly spaced each way top and bottom (212 total)

7101 Southbridge Drive - P.O. Box 658 - Sioux City, IA 51102-0658 - Phone 712.258.6690 - Fax 712.279.0814

No.: 26-2535-TLJ
Date: 10/24/25
By: TTW

Customer: ARCOLA TOWERS
Site: MD-003 Whitehall, MD
187' Monopole

PRELIMINARY -NOT FOR CONSTRUCTION-



ELEVATION VIEW
(75.40 Cu. Yds.)
(1 REQUIRED; NOT TO SCALE)

Notes:

1) Concrete shall have a minimum 28-day compressive strength of 4,500 psi, in accordance with ACI 318-14.

2) Rebar to conform to ASTM specification A615 Grade 60.

3) All rebar to have a minimum of 3" concrete cover.

4) All exposed concrete corners to be chamfered 3/4".

5) The foundation design is based on presumptive clay soil as defined in ANSI/TIA-222-H-2017. It is recommended that a soil analysis of the site be performed to verify the soil parameters used in the design.

6) The bottom anchor bolt template shall be positioned as closely as possible to the bottom of the anchor bolts.

Rebar Schedule for Pier	
Pier	(54) #9 vertical rebar w/ #5 ties, (2) within top 5" of pier, then 7" C/C

7101 Southbridge Drive - P.O. Box 658 - Sioux City, IA 51102-0658 - Phone 712.258.6690 - Fax 712.279.0814

187' Monopole / MD-003 Whitehall, MD

* All pole diameters shown on the following pages are across corners.
See profile drawing for widths across flats.

POLE GEOMETRY

ELEV ft	SECTION NAME	No. SIDE	OUTSIDE DIAM in	THICK -NESS in	RESISTANCES **Pn kip	RESISTANCES **Mn ft-kip	SPLICE TYPE	OVERLAP LENGTH ft	OVERLAP RATIO	w/t
186.0										
			19.29	0.250	1105.4	424.0				
	A	18								12.2
			30.57	0.250	1660.2	1018.9				
146.0										
			30.57	0.250	1660.2	1018.9				
	A/B	18					SLIP	4.50	1.75	
			31.34	0.375	2696.2	1683.6				
141.5										
			31.34	0.375	2696.2	1683.6				
	B	18								13.6
			43.38	0.375	3577.9	3113.2				
98.7										
			43.38	0.375	3577.9	3113.2				
	B/C	18					SLIP	6.25	1.72	
			44.40	0.438	4418.9	3926.0				
92.5										
			44.40	0.438	4418.9	3926.0				
	C	18								16.8
			55.44	0.438	5180.1	5769.9				
53.2										
			55.44	0.438	5180.1	5769.9				
	C/D	18					SLIP	8.00	1.72	
			56.84	0.438	5266.2	6015.7				
45.2										
			56.84	0.438	5266.2	6015.7				
	D	18								21.7
			69.58	0.438	5950.1	8345.0				
0.0										

POLE ASSEMBLY

SECTION NAME	BASE ELEV ft	BOLTS AT BASE OF SECTION NUMBER	TYPE	DIAM in	STRENGTH ksi	THREADS IN SHEAR PLANE	CALC BASE ELEV ft
A	141.500	0	A325	0.00	92.0	0	141.500
B	92.500	0	A325	0.00	92.0	0	92.500
C	45.250	0	A325	0.00	92.0	0	45.250
D	0.000	0	A325	0.00	92.0	0	0.000

POLE SECTIONS

SECTION NAME	No.of SIDES	LENGTH ft	OUTSIDE.DIAMETER BOT * in	OUTSIDE.DIAMETER TOP * in	BEND RAD in	MAT-ERIAL ID	FLANGE.ID BOT	FLANGE.ID TOP	FLANGE.WELD ..GROUP.ID.. BOT	FLANGE.WELD ..GROUP.ID.. TOP
A	18	44.50	31.84	19.29	0.625	1	0	0	0	0
B	18	53.50	45.15	30.07	0.625	2	0	0	0	0
C	18	53.50	57.71	42.63	0.625	3	0	0	0	0
D	18	53.25	69.58	54.57	0.625	4	0	0	0	0

* - Diameter of circumscribed circle

MATERIAL TYPES

TYPE OF SHAPE	TYPE NO	NO OF ELEM.	ORIENT & deg	HEIGHT in	WIDTH in	.THICKNESS. WEB in	FLANGE in	IRREGULARITY .PROJECTION. % OF AREA	ORIENT deg
PL	1	1	0.0	31.84	0.25	0.250	0.250	0.00	0.0
PL	2	1	0.0	45.15	0.38	0.375	0.375	0.00	0.0
PL	3	1	0.0	57.71	0.44	0.438	0.438	0.00	0.0
PL	4	1	0.0	69.58	0.44	0.438	0.438	0.00	0.0

& - With respect to vertical

MATERIAL PROPERTIES

MATERIAL TYPE NO.	ELASTIC MODULUS ksi	UNIT WEIGHT pcf	.. STRENGTH .. Fu ksi	Fy ksi	THERMAL COEFFICIENT /deg
1	29000.0	490.0	80.0	65.0	0.00001170
2	29000.0	490.0	80.0	65.0	0.00001170
3	29000.0	490.0	80.0	65.0	0.00001170
4	29000.0	490.0	80.0	65.0	0.00001170

* Only 5 condition(s) shown in full

LOADING CONDITION A

112 mph wind with no ice. Wind Azimuth: 0• (1.2 D + 1.0 Wo)

LOADS ON POLE

LOAD TYPE	ELEV ft	APPLY..LOAD..AT RADIUS ft	AZI	LOAD AZI	.....FORCES..... HORIZ kip	DOWN kip	.....MOMENTS..... VERTICAL ft-kip	TORSNAL ft-kip
C	186.000	0.00	0.0	0.0	0.0948	0.0180	0.0000	0.0000
C	183.000	0.00	0.0	0.0	0.0207	0.0101	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.0000	1.3553	0.0000	0.0000
C	181.000	0.00	0.0	0.0	11.7840	8.4000	0.0000	0.0000
C	175.000	0.00	0.0	0.0	0.0341	0.0168	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.0000	1.2804	0.0000	0.0000
C	171.000	0.00	0.0	0.0	9.3157	5.4000	0.0000	0.0000
C	165.000	0.00	0.0	0.0	0.0337	0.0168	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.0000	1.2056	0.0000	0.0000
C	161.000	0.00	0.0	0.0	8.0491	5.4000	0.0000	0.0000
C	155.000	0.00	0.0	0.0	0.0333	0.0168	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.0000	1.1307	0.0000	0.0000
C	151.000	0.00	0.0	0.0	6.8073	5.4000	0.0000	0.0000
C	145.000	0.00	0.0	0.0	0.0328	0.0168	0.0000	0.0000
C	135.000	0.00	0.0	0.0	0.0323	0.0168	0.0000	0.0000
C	125.000	0.00	0.0	0.0	0.0318	0.0168	0.0000	0.0000
C	115.000	0.00	0.0	0.0	0.0312	0.0168	0.0000	0.0000
C	105.000	0.00	0.0	0.0	0.0306	0.0168	0.0000	0.0000
C	95.000	0.00	0.0	0.0	0.0300	0.0168	0.0000	0.0000
C	85.000	0.00	0.0	0.0	0.0293	0.0168	0.0000	0.0000
C	75.000	0.00	0.0	0.0	0.0286	0.0168	0.0000	0.0000
C	65.000	0.00	0.0	0.0	0.0277	0.0168	0.0000	0.0000
C	55.000	0.00	0.0	0.0	0.0267	0.0168	0.0000	0.0000
C	45.000	0.00	0.0	0.0	0.0256	0.0168	0.0000	0.0000
C	35.000	0.00	0.0	0.0	0.0243	0.0168	0.0000	0.0000
C	25.000	0.00	0.0	0.0	0.0227	0.0168	0.0000	0.0000
C	15.000	0.00	0.0	0.0	0.0204	0.0168	0.0000	0.0000
D	186.000	0.00	180.0	0.0	0.0500	0.0635	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0709	0.0941	0.0000	0.0000

D	146.000	0.00	180.0	0.0	0.0736	0.2441	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0736	0.2441	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0756	0.1526	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0936	0.2015	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0959	0.4506	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0959	0.4506	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0970	0.2501	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.1065	0.3026	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.1068	0.6220	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.1068	0.6220	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.1071	0.3195	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0977	0.3637	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0984	0.3725	0.0000	0.0000
D	0.000	0.00	180.0	0.0	0.1008	0.3814	0.0000	0.0000

LOADING CONDITION M

112 mph wind with no ice. Wind Azimuth: 0• (0.9 D + 1.0 Wo)

LOADS ON POLE

LOAD TYPE	ELEV	APPLY..LOAD..AT RADIUS	AZI	LOAD AZI	.....FORCES..... HORIZ	DOWN	.....MOMENTS..... VERTICAL	TORSNAL
	ft	ft			kip	kip	ft-kip	ft-kip
C	186.000	0.00	0.0	0.0	0.0948	0.0135	0.0000	0.0000
C	183.000	0.00	0.0	0.0	0.0207	0.0076	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.0000	1.0165	0.0000	0.0000
C	181.000	0.00	0.0	0.0	11.7840	6.3000	0.0000	0.0000
C	175.000	0.00	0.0	0.0	0.0341	0.0126	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.0000	0.9603	0.0000	0.0000
C	171.000	0.00	0.0	0.0	9.3157	4.0500	0.0000	0.0000
C	165.000	0.00	0.0	0.0	0.0337	0.0126	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.0000	0.9042	0.0000	0.0000
C	161.000	0.00	0.0	0.0	8.0491	4.0500	0.0000	0.0000
C	155.000	0.00	0.0	0.0	0.0333	0.0126	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.0000	0.8480	0.0000	0.0000
C	151.000	0.00	0.0	0.0	6.8073	4.0500	0.0000	0.0000
C	145.000	0.00	0.0	0.0	0.0328	0.0126	0.0000	0.0000
C	135.000	0.00	0.0	0.0	0.0323	0.0126	0.0000	0.0000
C	125.000	0.00	0.0	0.0	0.0318	0.0126	0.0000	0.0000
C	115.000	0.00	0.0	0.0	0.0312	0.0126	0.0000	0.0000
C	105.000	0.00	0.0	0.0	0.0306	0.0126	0.0000	0.0000
C	95.000	0.00	0.0	0.0	0.0300	0.0126	0.0000	0.0000
C	85.000	0.00	0.0	0.0	0.0293	0.0126	0.0000	0.0000
C	75.000	0.00	0.0	0.0	0.0286	0.0126	0.0000	0.0000
C	65.000	0.00	0.0	0.0	0.0277	0.0126	0.0000	0.0000
C	55.000	0.00	0.0	0.0	0.0267	0.0126	0.0000	0.0000
C	45.000	0.00	0.0	0.0	0.0256	0.0126	0.0000	0.0000
C	35.000	0.00	0.0	0.0	0.0243	0.0126	0.0000	0.0000
C	25.000	0.00	0.0	0.0	0.0227	0.0126	0.0000	0.0000
C	15.000	0.00	0.0	0.0	0.0204	0.0126	0.0000	0.0000
D	186.000	0.00	180.0	0.0	0.0500	0.0476	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0709	0.0705	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0736	0.1831	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0736	0.1831	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0756	0.1144	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0936	0.1511	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0959	0.3379	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0959	0.3379	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0970	0.1876	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.1065	0.2269	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.1068	0.4665	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.1068	0.4665	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.1071	0.2396	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0977	0.2728	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0984	0.2794	0.0000	0.0000
D	0.000	0.00	180.0	0.0	0.1008	0.2860	0.0000	0.0000

LOADING CONDITION Y

40 mph wind with 1 ice. Wind Azimuth: 0• (1.2 D + 1.0 Di + 1.0 Wi)

LOADS ON POLE
=============

LOAD TYPE	ELEV ft	APPLY..LOAD..AT RADIUS ft	AZI	LOAD AZI	.....FORCES..... HORIZ kip	DOWN kip	.....MOMENTS..... VERTICAL ft-kip	TORSNAL ft-kip
C	186.000	0.00	0.0	0.0	0.0220	0.0656	0.0000	0.0000
C	183.000	0.00	0.0	0.0	0.0150	0.0221	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.0000	1.3553	0.0000	0.0000
C	181.000	0.00	0.0	0.0	2.2162	16.7034	0.0000	0.0000
C	175.000	0.00	0.0	0.0	0.0246	0.0288	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.0000	1.2804	0.0000	0.0000
C	171.000	0.00	0.0	0.0	1.7488	10.7078	0.0000	0.0000
C	165.000	0.00	0.0	0.0	0.0242	0.0288	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.0000	1.2056	0.0000	0.0000
C	161.000	0.00	0.0	0.0	1.2674	10.6761	0.0000	0.0000
C	155.000	0.00	0.0	0.0	0.0238	0.0288	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.0000	1.1307	0.0000	0.0000
C	151.000	0.00	0.0	0.0	1.2729	10.6426	0.0000	0.0000
C	145.000	0.00	0.0	0.0	0.0233	0.0288	0.0000	0.0000
C	135.000	0.00	0.0	0.0	0.0228	0.0288	0.0000	0.0000
C	125.000	0.00	0.0	0.0	0.0223	0.0288	0.0000	0.0000
C	115.000	0.00	0.0	0.0	0.0218	0.0288	0.0000	0.0000
C	105.000	0.00	0.0	0.0	0.0212	0.0288	0.0000	0.0000
C	95.000	0.00	0.0	0.0	0.0206	0.0288	0.0000	0.0000
C	85.000	0.00	0.0	0.0	0.0200	0.0288	0.0000	0.0000
C	75.000	0.00	0.0	0.0	0.0192	0.0288	0.0000	0.0000
C	65.000	0.00	0.0	0.0	0.0185	0.0288	0.0000	0.0000
C	55.000	0.00	0.0	0.0	0.0176	0.0288	0.0000	0.0000
C	45.000	0.00	0.0	0.0	0.0166	0.0288	0.0000	0.0000
C	35.000	0.00	0.0	0.0	0.0154	0.0288	0.0000	0.0000
C	25.000	0.00	0.0	0.0	0.0140	0.0288	0.0000	0.0000
C	15.000	0.00	0.0	0.0	0.0121	0.0288	0.0000	0.0000
D	186.000	0.00	180.0	0.0	0.0124	0.0944	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0170	0.1381	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0176	0.2899	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0176	0.2899	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0180	0.1996	0.0000	0.0000
D	135.393	0.00	180.0	0.0	0.0180	0.1996	0.0000	0.0000
D	135.393	0.00	180.0	0.0	0.0187	0.2100	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0219	0.2613	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0224	0.5123	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0224	0.5123	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0227	0.3128	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.0246	0.3747	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.0247	0.6954	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.0247	0.6954	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.0247	0.3937	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0224	0.4395	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0225	0.4460	0.0000	0.0000
D	0.000	0.00	180.0	0.0	0.0230	0.4499	0.0000	0.0000

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LOADING CONDITION AK ==========

Seismic - Azimuth: 0• (1.2 D + 1.0 Ev + 1.0 Eh)

LOADS ON POLE
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LOAD TYPE	ELEV ft	APPLY..LOAD..AT RADIUS ft	AZI	LOAD AZI	.....FORCES..... HORIZ kip	DOWN kip	.....MOMENTS..... VERTICAL ft-kip	TORSNAL ft-kip
C	186.000	0.00	0.0	0.0	0.0010	0.0184	0.0000	0.0000
C	183.000	0.00	0.0	0.0	0.0006	0.0103	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.0728	1.3851	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.4511	8.5848	0.0000	0.0000
C	175.000	0.00	0.0	0.0	0.0008	0.0172	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.0614	1.3086	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.2588	5.5188	0.0000	0.0000
C	165.000	0.00	0.0	0.0	0.0007	0.0172	0.0000	0.0000
C	163.750	0.00	0.0	0.0	0.1576	3.6639	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.0512	1.2320	0.0000	0.0000

C	161.000	0.00	0.0	0.0	0.2294	5.5188	0.0000	0.0000
C	155.000	0.00	0.0	0.0	0.0007	0.0172	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.0423	1.1555	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.2018	5.5188	0.0000	0.0000
C	145.000	0.00	0.0	0.0	0.0006	0.0172	0.0000	0.0000
C	135.000	0.00	0.0	0.0	0.0005	0.0172	0.0000	0.0000
C	125.000	0.00	0.0	0.0	0.0004	0.0172	0.0000	0.0000
C	119.250	0.00	0.0	0.0	0.2216	9.7175	0.0000	0.0000
C	115.000	0.00	0.0	0.0	0.0004	0.0172	0.0000	0.0000
C	105.000	0.00	0.0	0.0	0.0003	0.0172	0.0000	0.0000
C	95.000	0.00	0.0	0.0	0.0002	0.0172	0.0000	0.0000
C	85.000	0.00	0.0	0.0	0.0002	0.0172	0.0000	0.0000
C	75.000	0.00	0.0	0.0	0.0002	0.0172	0.0000	0.0000
C	72.000	0.00	0.0	0.0	0.1259	15.1446	0.0000	0.0000
C	65.000	0.00	0.0	0.0	0.0001	0.0172	0.0000	0.0000
C	55.000	0.00	0.0	0.0	0.0001	0.0172	0.0000	0.0000
C	45.000	0.00	0.0	0.0	0.0001	0.0172	0.0000	0.0000
C	35.000	0.00	0.0	0.0	0.0000	0.0172	0.0000	0.0000
C	26.620	0.00	0.0	0.0	0.0212	18.6840	0.0000	0.0000
C	25.000	0.00	0.0	0.0	0.0000	0.0172	0.0000	0.0000
C	15.000	0.00	0.0	0.0	0.0000	0.0172	0.0000	0.0000
D	186.000	0.00	180.0	180.0	0.0000	0.0000	0.0000	0.0000
D	0.000	0.00	180.0	180.0	0.0000	0.0000	0.0000	0.0000

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LOADING CONDITION AL ==========

Seismic - Azimuth: 0• (0.9 D - 1.0 Ev + 1.0 Eh)

LOADS ON POLE

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LOAD TYPE	ELEV	APPLY..LOAD..AT RADIUS	AZI	LOAD AZI	.....FORCES..... HORIZ	DOWN	.....MOMENTS..... VERTICAL	TORSNAL
	ft	ft			kip	kip	ft-kip	ft-kip
C	186.000	0.00	0.0	0.0	0.0010	0.0131	0.0000	0.0000
C	183.000	0.00	0.0	0.0	0.0006	0.0074	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.0728	0.9867	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.4511	6.1152	0.0000	0.0000
C	175.000	0.00	0.0	0.0	0.0008	0.0122	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.0614	0.9321	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.2588	3.9312	0.0000	0.0000
C	165.000	0.00	0.0	0.0	0.0007	0.0122	0.0000	0.0000
C	163.750	0.00	0.0	0.0	0.1576	2.6098	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.0512	0.8776	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.2294	3.9312	0.0000	0.0000
C	155.000	0.00	0.0	0.0	0.0007	0.0122	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.0423	0.8231	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.2018	3.9312	0.0000	0.0000
C	145.000	0.00	0.0	0.0	0.0006	0.0122	0.0000	0.0000
C	135.000	0.00	0.0	0.0	0.0005	0.0122	0.0000	0.0000
C	125.000	0.00	0.0	0.0	0.0004	0.0122	0.0000	0.0000
C	119.250	0.00	0.0	0.0	0.2216	6.9220	0.0000	0.0000
C	115.000	0.00	0.0	0.0	0.0004	0.0122	0.0000	0.0000
C	105.000	0.00	0.0	0.0	0.0003	0.0122	0.0000	0.0000
C	95.000	0.00	0.0	0.0	0.0002	0.0122	0.0000	0.0000
C	85.000	0.00	0.0	0.0	0.0002	0.0122	0.0000	0.0000
C	75.000	0.00	0.0	0.0	0.0002	0.0122	0.0000	0.0000
C	72.000	0.00	0.0	0.0	0.1259	10.7879	0.0000	0.0000
C	65.000	0.00	0.0	0.0	0.0001	0.0122	0.0000	0.0000
C	55.000	0.00	0.0	0.0	0.0001	0.0122	0.0000	0.0000
C	45.000	0.00	0.0	0.0	0.0001	0.0122	0.0000	0.0000
C	35.000	0.00	0.0	0.0	0.0000	0.0122	0.0000	0.0000
C	26.620	0.00	0.0	0.0	0.0212	13.3091	0.0000	0.0000
C	25.000	0.00	0.0	0.0	0.0000	0.0122	0.0000	0.0000
C	15.000	0.00	0.0	0.0	0.0000	0.0122	0.0000	0.0000
D	186.000	0.00	180.0	180.0	0.0000	0.0000	0.0000	0.0000
D	0.000	0.00	180.0	180.0	0.0000	0.0000	0.0000	0.0000

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187' Monopole / MD-003 Whitehall, MD

MAXIMUM POLE DEFORMATIONS CALCULATED(w.r.t. wind direction)

MAST ELEV ft	DEFLECTIONS (ft) HORIZONTAL ALONG	HORIZONTAL ACROSS	DOWN	ROTATIONS (deg) TILT ALONG	TILT ACROSS	TWIST
186.0	17.04B	-0.09U	2.27K	10.96B	-0.04U	-0.01W
180.3	15.98B	-0.08U	2.06K	10.96B	-0.04U	-0.01W
174.6	14.93B	-0.08U	1.86K	10.88B	-0.04U	-0.01W
168.9	13.89B	-0.07U	1.67K	10.69B	-0.04U	-0.01W
163.1	12.87B	-0.07U	1.48K	10.40B	-0.04U	-0.01W
157.4	11.89B	-0.07U	1.30K	10.02B	-0.04U	-0.01W
151.7	10.94B	-0.06U	1.14K	9.55B	-0.04U	-0.01W
146.0	10.03B	-0.06U	1.00K	9.02B	-0.04U	-0.01W
141.5	9.35B	-0.05U	0.89K	8.71B	-0.04U	-0.01W
135.4	8.47B	-0.05U	0.76K	8.23B	-0.04U	-0.01W
129.3	7.63B	-0.05U	0.64K	7.75B	-0.04U	-0.01W
123.2	6.85B	-0.04U	0.54K	7.25B	-0.04U	0.00U
117.1	6.11B	-0.04U	0.45B	6.76B	-0.04U	0.00U
111.0	5.43B	-0.03U	0.37B	6.27B	-0.04U	0.00U
104.9	4.79B	-0.03U	0.31B	5.79B	-0.03U	0.00U
98.7	4.21B	-0.03U	0.25B	5.32B	-0.03U	0.00U
92.5	3.65B	-0.02U	0.20B	4.92B	-0.03U	0.00U
86.9	3.19B	-0.02U	0.16B	4.55B	-0.03U	0.00U
81.3	2.77B	-0.02U	0.13B	4.19B	-0.03U	0.00U
75.7	2.38B	-0.01U	0.10B	3.84B	-0.02U	0.00U
70.1	2.02B	-0.01U	0.08B	3.51B	-0.02U	0.00U
64.5	1.69B	-0.01U	0.06B	3.18B	-0.02U	0.00U
58.9	1.40B	-0.01U	0.05B	2.86B	-0.02U	0.00U
53.2	1.13B	-0.01U	0.03B	2.56B	-0.02U	0.00U
45.2	0.81B	-0.01U	0.02B	2.14B	-0.01U	0.00U
39.6	0.61B	0.00U	0.01B	1.84B	-0.01U	0.00U
33.9	0.44B	0.00U	0.01B	1.55B	-0.01U	0.00U
28.3	0.30B	0.00U	0.01B	1.27B	-0.01U	0.00U
22.6	0.19B	0.00U	0.00B	1.00B	-0.01U	0.00U
17.0	0.11B	0.00U	0.00B	0.74B	0.00U	0.00U
11.3	0.05B	0.00U	0.00B	0.48B	0.00U	0.00U

5.7	0.01B	0.00U	0.00Y	0.24B	0.00U	0.00U
0.0	0.00A	0.00A	0.00A	0.00A	0.00A	0.00A

MAXIMUM POLE FORCES CALCULATED(w.r.t. to wind direction)

MAST ELEV ft	TOTAL AXIAL kip	SHEAR.w.r.t.WIND.DIR ALONG kip	ACROSS kip	MOMENT.w.r.t.WIND.DIR ALONG ft-kip	ACROSS ft-kip	TORSION ft-kip
186.0	0.07 Y	0.10 K	0.01 B	-0.08 W	0.04 B	-0.01 B
	18.70 Y	12.19 K	0.01 B	-10.98 K	-0.04 B	-0.01 W
180.3	18.71 AB	12.27 O	-0.04 O	-11.03 I	-0.06 Q	-0.01 W
	19.33 AB	12.61 O	-0.04 O	-90.08 C	-0.20 H	-0.10 W
174.6	19.33 AG	12.62 E	0.04 E	-90.09 H	-0.23 H	-0.09 W
	31.95 AG	22.25 E	0.04 E	-193.59 E	0.35 O	-0.23 W
168.9	31.95 AD	22.27 E	-0.08 W	-193.57 E	-0.29 E	-0.22 W
	32.64 AD	22.64 E	-0.08 W	-335.69 E	-0.57 L	-0.45 W
163.1	32.65 AD	22.66 K	-0.09 W	-335.76 E	-0.51 L	-0.44 W
	45.23 AD	31.07 K	-0.09 W	-511.59 K	-0.87 L	-0.70 W
157.4	45.23 AJ	31.03 Q	0.18 O	-511.61 K	-0.95 L	-0.69 W
	45.99 AJ	31.44 Q	0.18 O	-709.10 K	-1.21 L	-0.92 W
151.7	45.99 AJ	31.51 R	-0.12 R	-709.28 K	-1.27 L	-0.92 W
	58.53 AJ	38.70 R	-0.12 R	-947.23 K	-1.47 L	-1.20 W
146.0	58.53 AJ	38.73 K	-0.16 Q	-947.10 K	-1.47 L	-1.20 W
	59.86 AJ	39.09 K	-0.16 Q	-1140.99 K	-2.17 L	-1.41 W
141.5	59.87 AI	39.12 K	-0.25 U	-1141.06 K	-2.07 O	-1.41 W
	61.09 AI	39.58 K	-0.25 U	-1406.53 K	-3.39 L	-1.65 W
135.4	61.09 AI	39.68 B	-0.25 U	-1406.62 K	-3.40 L	-1.67 W
	62.42 AI	40.19 B	-0.25 U	-1675.22 K	-4.41 L	-1.80 W
129.3	62.42 Y	40.21 K	-0.27 U	-1675.11 K	-4.36 L	-1.80 W
	63.81 Y	40.74 K	-0.27 U	-1946.70 K	-5.39 L	-1.86 W
123.2	63.81 Y	40.80 K	-0.35 U	-1946.43 K	-5.35 L	-1.87 W
	65.22 Y	41.31 K	-0.35 U	-2220.90 K	6.85 U	-1.97 W
117.1	65.22 Y	41.40 U	-0.47 U	-2220.76 K	6.91 U	-1.96 W
	66.72 Y	41.96 U	-0.47 U	-2498.03 K	9.72 U	-2.12 W
111.0	66.71 Y	42.00 U	-0.47 U	-2497.63 K	9.81 U	-2.14 W
	68.26 Y	42.58 U	-0.47 U	-2777.61 K	12.63 U	-2.19 W
104.9	68.26 Y	42.56 U	-0.52 U	-2777.29 K	12.48 U	-2.19 W
	69.83 Y	43.12 U	-0.52 U	-3059.00 K	15.63 U	-2.30 W
98.7	69.83 Y	43.10 B	-0.55 U	-3058.99 K	15.50 U	-2.30 W
	73.06 Y	43.73 B	-0.55 U	-3350.70 K	18.90 U	-2.39 U
92.5	73.06 Y	43.82 B	0.35 W	-3350.55 K	18.93 U	-2.39 U
	74.84 Y	44.37 B	0.35 W	-3615.94 B	20.48 U	-2.53 U

86.9						
	74.84 Y	44.26 B	−0.36 U	−3615.99 B	20.24 U	−2.51 U
	76.70 Y	44.84 B	−0.36 U	−3884.09 B	22.22 U	−2.67 U
81.3						
	76.70 Y	45.09 B	0.42 F	−3883.74 B	22.18 U	−2.68 U
	78.57 Y	45.65 B	0.42 F	−4155.45 B	24.35 U	−2.83 U
75.7						
	78.57 Y	45.57 B	−0.45 U	−4155.52 B	24.42 U	−2.83 U
	80.53 Y	46.17 B	−0.45 U	−4429.29 B	26.95 U	−3.00 U
70.1						
	80.53 Y	46.28 B	−0.39 U	−4429.33 B	26.94 U	−3.00 U
	82.53 Y	46.89 B	−0.39 U	−4706.01 B	29.09 U	−3.13 U
64.5						
	82.54 Y	47.10 B	0.35 F	−4705.76 B	29.02 U	−3.11 U
	84.56 Y	47.69 B	0.35 F	−4986.36 B	30.93 U	−3.21 U
58.9						
	84.56 Y	47.66 B	0.36 H	−4986.09 B	30.82 U	−3.22 U
	86.67 Y	48.28 B	0.36 H	−5268.72 B	32.67 U	−3.30 U
53.2						
	86.67 Y	48.22 B	0.37 H	−5268.68 B	32.70 U	−3.30 U
	92.23 Y	49.07 B	0.37 H	−5675.94 B	35.27 U	−3.41 U
45.2						
	92.23 Y	49.02 B	0.41 H	−5675.86 B	35.25 U	−3.41 U
	94.51 Y	49.65 B	0.41 H	−5966.60 B	37.06 U	−3.47 U
39.6						
	94.51 Y	49.64 B	0.41 H	−5966.59 B	37.11 U	−3.47 U
	96.83 Y	50.26 B	0.41 H	−6259.39 B	39.23 U	−3.53 U
33.9						
	96.83 Y	50.28 B	0.36 F	−6259.46 B	39.26 U	−3.53 U
	99.16 Y	50.87 B	0.36 F	−6554.23 B	40.92 U	−3.57 U
28.3						
	99.16 Y	50.89 B	0.33 H	−6554.28 B	40.92 U	−3.57 U
	101.57 Y	51.48 B	0.33 H	−6851.11 B	42.70 U	−3.61 U
22.6						
	101.57 Y	51.49 B	−0.34 T	−6851.10 B	42.70 U	−3.61 U
	103.99 Y	52.06 B	−0.34 T	−7149.76 B	44.61 U	−3.63 U
17.0						
	103.99 Y	52.01 B	0.34 H	−7149.77 B	44.61 U	−3.63 U
	106.48 Y	52.59 B	0.34 H	−7449.83 B	46.48 U	−3.65 U
11.3						
	106.48 Y	52.60 B	0.40 H	−7449.83 B	46.50 U	−3.65 U
	109.01 Y	53.16 B	0.40 H	−7751.50 B	48.43 U	−3.67 U
5.7						
	109.01 Y	53.16 B	0.39 H	−7751.50 B	48.43 U	−3.67 U
	111.55 Y	53.73 B	0.39 H	−8054.65 B	50.46 U	−3.67 U
base reaction	111.55 Y	−53.73 B	−0.39 H	8054.65 B	−50.46 U	3.67 U

COMPLIANCE WITH 4.8.2 & 4.5.4

ELEV ft	AXIAL	BENDING	SHEAR + TORSIONAL	TOTAL	SATISFIED	D/t(w/t)	MAX ALLOWED
186.00							
	0.00Y	0.00W	0.00K	0.00W	YES	12.17A	45.2
	0.02Y	0.02K	0.02K	0.03K	YES	13.29A	45.2
180.29							
	0.02AB	0.02I	0.02O	0.03K	YES	13.29A	45.2
	0.01AB	0.16C	0.02O	0.16C	YES	14.41A	45.2

174.57							
	0.01AG	0.16H	0.02E	0.16H	YES	14.41A	45.2
	0.02AG	0.29E	0.03E	0.30E	YES	15.52A	45.2
168.86							
	0.02AD	0.29E	0.03E	0.30E	YES	15.52A	45.2
	0.02AD	0.44E	0.03E	0.46E	YES	16.64A	45.2
163.14							
	0.02AD	0.44E	0.03K	0.46E	YES	16.64A	45.2
	0.03AD	0.61K	0.04K	0.63K	YES	17.76A	45.2
157.43							
	0.03AJ	0.61K	0.04Q	0.63K	YES	17.76A	45.2
	0.03AJ	0.76K	0.04Q	0.78K	YES	18.88A	45.2
151.71							
	0.03AJ	0.76K	0.04R	0.78K	YES	18.88A	45.2
	0.04AJ	0.93K	0.05R	0.95K	YES	20.00A	45.2
146.00							
	0.02AJ	0.59K	0.03K	0.61K	YES	13.22A	45.2
	0.02AJ	0.66K	0.03K	0.67K	YES	13.80A	45.2
141.50							
	0.02AI	0.68K	0.03K	0.69K	YES	13.57A	45.2
	0.02AI	0.75K	0.03K	0.76K	YES	14.37A	45.2
135.39							
	0.02AI	0.75K	0.03B	0.76K	YES	14.37A	45.2
	0.02AI	0.81K	0.03B	0.82K	YES	15.16A	45.2
129.29							
	0.02Y	0.81K	0.03K	0.82K	YES	15.16A	45.2
	0.02Y	0.85K	0.03K	0.86K	YES	15.96A	45.2
123.18							
	0.02Y	0.85K	0.03K	0.86K	YES	15.96A	45.2
	0.02Y	0.89K	0.03K	0.90K	YES	16.76A	45.2
117.07							
	0.02Y	0.89K	0.03U	0.90K	YES	16.76A	45.2
	0.02Y	0.93K	0.02U	0.94K	YES	17.56A	45.2
110.96							
	0.02Y	0.93K	0.02U	0.94K	YES	17.56A	45.2
	0.02Y	0.96K	0.02U	0.97K	YES	18.35A	45.2
104.86							
	0.02Y	0.96K	0.02U	0.97K	YES	18.35A	45.2
	0.02Y	0.98K	0.02U	0.99K	YES	19.15A	45.2
98.75							
	0.02Y	0.81K	0.02B	0.82K	YES	16.37A	45.2
	0.02Y	0.83K	0.02B	0.84K	YES	17.06A	45.2
92.50							
	0.02Y	0.85K	0.02B	0.86K	YES	16.76A	45.2
	0.02Y	0.87B	0.02B	0.88B	YES	17.39A	45.2
86.89							
	0.02Y	0.87B	0.02U	0.88B	YES	17.39A	45.2
	0.02Y	0.88B	0.02U	0.89B	YES	18.02A	45.2
81.29							
	0.02Y	0.88B	0.02B	0.89B	YES	18.02A	45.2
	0.02Y	0.89B	0.02B	0.90B	YES	18.65A	45.2
75.68							
	0.02Y	0.89B	0.02B	0.90B	YES	18.65A	45.2
	0.02Y	0.89B	0.02B	0.90B	YES	19.27A	45.2
70.07							
	0.02Y	0.89B	0.02B	0.90B	YES	19.27A	45.2
	0.02Y	0.90B	0.02B	0.91B	YES	19.90A	45.2
64.46							
	0.02Y	0.90B	0.02B	0.91B	YES	19.90A	45.2
	0.02Y	0.91B	0.02B	0.92B	YES	20.53A	45.2
58.86							

	0.02Y	0.91B	0.02B	0.92B	YES	20.53A	45.2
	0.02Y	0.91B	0.02B	0.92B	YES	21.16A	45.2
53.25	.....						
	0.02Y	0.91B	0.02B	0.92B	YES	21.16A	45.2
	0.02Y	0.92B	0.02B	0.93B	YES	22.05A	45.2
45.25	.....						
	0.02Y	0.94B	0.02B	0.96B	YES	21.70A	45.2
	0.02Y	0.95B	0.02B	0.96B	YES	22.33A	45.2
39.59	.....						
	0.02Y	0.95B	0.02B	0.96B	YES	22.33A	45.2
	0.02Y	0.95B	0.02B	0.96B	YES	22.96A	45.2
33.94	.....						
	0.02Y	0.95B	0.02B	0.96B	YES	22.96A	45.2
	0.02Y	0.95B	0.02B	0.97B	YES	23.60A	45.2
28.28	.....						
	0.02Y	0.95B	0.02B	0.97B	YES	23.60A	45.2
	0.02Y	0.96B	0.02B	0.97B	YES	24.23A	45.2
22.62	.....						
	0.02Y	0.96B	0.02B	0.97B	YES	24.23A	45.2
	0.02Y	0.96B	0.02B	0.97B	YES	24.86A	45.2
16.97	.....						
	0.02Y	0.96B	0.02B	0.97B	YES	24.86A	45.2
	0.02Y	0.96B	0.02B	0.97B	YES	25.50A	45.2
11.31	.....						
	0.02Y	0.96B	0.02B	0.97B	YES	25.50A	45.2
	0.02Y	0.96B	0.02B	0.98B	YES	26.13A	45.2
5.66	.....						
	0.02Y	0.96B	0.02B	0.98B	YES	26.13A	45.2
	0.02Y	0.97B	0.02B	0.98B	YES	26.76A	45.2
0.00	.....						

MAXIMUM LOADS ONTO FOUNDATION(w.r.t. wind direction)
==========

DOWN	SHEAR.w.r.t.WIND.DIR ALONG	ACROSS	MOMENT.w.r.t.WIND.DIR ALONG	ACROSS	TORSION
kip	kip	kip	ft-kip	ft-kip	ft-kip
111.55	53.73	0.39	-8054.65	50.46	-3.67
Y	B	H	B	U	U

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187' Monopole / MD-003 Whitehall, MD

****
********** Service Load Condition **********
****

* Only 1 condition(s) shown in full

LOADING CONDITION A ==========

60 mph wind with no ice. Wind Azimuth: 0• (1.0 D + 1.0 Wo)

LOADS ON POLE
==========

LOAD TYPE	ELEV ft	APPLY..LOAD..AT RADIUS ft	AZI	LOAD AZI	.....FORCES..... HORIZ kip	DOWN kip	.....MOMENTS..... VERTICAL ft-kip	TORSNAL ft-kip
C	186.000	0.00	0.0	0.0	0.0243	0.0150	0.0000	0.0000
C	183.000	0.00	0.0	0.0	0.0053	0.0084	0.0000	0.0000
C	181.000	0.00	0.0	0.0	0.0000	1.1294	0.0000	0.0000
C	181.000	0.00	0.0	0.0	3.0259	7.0000	0.0000	0.0000
C	175.000	0.00	0.0	0.0	0.0088	0.0140	0.0000	0.0000
C	171.000	0.00	0.0	0.0	0.0000	1.0670	0.0000	0.0000
C	171.000	0.00	0.0	0.0	2.3921	4.5000	0.0000	0.0000
C	165.000	0.00	0.0	0.0	0.0087	0.0140	0.0000	0.0000
C	161.000	0.00	0.0	0.0	0.0000	1.0046	0.0000	0.0000
C	161.000	0.00	0.0	0.0	2.0668	4.5000	0.0000	0.0000
C	155.000	0.00	0.0	0.0	0.0085	0.0140	0.0000	0.0000
C	151.000	0.00	0.0	0.0	0.0000	0.9422	0.0000	0.0000
C	151.000	0.00	0.0	0.0	1.7480	4.5000	0.0000	0.0000
C	145.000	0.00	0.0	0.0	0.0084	0.0140	0.0000	0.0000
C	135.000	0.00	0.0	0.0	0.0083	0.0140	0.0000	0.0000
C	125.000	0.00	0.0	0.0	0.0082	0.0140	0.0000	0.0000
C	115.000	0.00	0.0	0.0	0.0080	0.0140	0.0000	0.0000
C	105.000	0.00	0.0	0.0	0.0079	0.0140	0.0000	0.0000
C	95.000	0.00	0.0	0.0	0.0077	0.0140	0.0000	0.0000
C	85.000	0.00	0.0	0.0	0.0075	0.0140	0.0000	0.0000
C	75.000	0.00	0.0	0.0	0.0073	0.0140	0.0000	0.0000
C	65.000	0.00	0.0	0.0	0.0071	0.0140	0.0000	0.0000
C	55.000	0.00	0.0	0.0	0.0069	0.0140	0.0000	0.0000
C	45.000	0.00	0.0	0.0	0.0066	0.0140	0.0000	0.0000
C	35.000	0.00	0.0	0.0	0.0062	0.0140	0.0000	0.0000
C	25.000	0.00	0.0	0.0	0.0058	0.0140	0.0000	0.0000
C	15.000	0.00	0.0	0.0	0.0052	0.0140	0.0000	0.0000
D	186.000	0.00	180.0	0.0	0.0128	0.0529	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0182	0.0784	0.0000	0.0000
D	146.000	0.00	180.0	0.0	0.0189	0.2034	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0189	0.2034	0.0000	0.0000
D	141.500	0.00	180.0	0.0	0.0194	0.1271	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0240	0.1679	0.0000	0.0000
D	98.750	0.00	180.0	0.0	0.0246	0.3755	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0246	0.3755	0.0000	0.0000
D	92.500	0.00	180.0	0.0	0.0249	0.2084	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.0273	0.2522	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.0274	0.5184	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.0274	0.5184	0.0000	0.0000
D	45.250	0.00	180.0	0.0	0.0275	0.2662	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0251	0.3031	0.0000	0.0000
D	11.312	0.00	180.0	0.0	0.0253	0.3105	0.0000	0.0000
D	0.000	0.00	180.0	0.0	0.0259	0.3178	0.0000	0.0000

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MAXIMUM POLE DEFORMATIONS CALCULATED(w.r.t. wind direction)
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MAST ELEV ft	.....DEFLECTIONS (ft)..... HORIZONTAL ..... ALONG	ACROSS	DOWN	.....ROTATIONS (deg)..... TILT ..... ALONG	ACROSS	TWIST
186.0	4.47I	-0.02I	0.16I	2.83I	-0.01I	0.00K
180.3	4.18I	-0.02I	0.15I	2.83I	-0.01I	0.00K
174.6	3.90I	-0.02I	0.13I	2.81I	-0.01I	0.00K
168.9	3.62I	-0.02I	0.12I	2.76I	-0.01I	0.00K
163.1	3.35I	-0.01I	0.10I	2.69I	-0.01I	0.00K
157.4	3.09I	-0.01I	0.09I	2.59I	-0.01I	0.00K
151.7	2.84I	-0.01I	0.08I	2.46I	-0.01I	0.00K
146.0	2.60I	-0.01I	0.07I	2.32I	-0.01I	0.00K
141.5	2.42I	-0.01I	0.06I	2.24I	-0.01I	0.00K

135.4	2.19I	-0.01I	0.05I	2.12I	-0.01I	0.00K
129.3	1.97I	-0.01I	0.05I	1.99I	-0.01I	0.00K
123.2	1.77I	-0.01I	0.04I	1.86I	-0.01I	0.00K
117.1	1.57I	-0.01I	0.03I	1.73I	-0.01I	0.00K
111.0	1.40I	-0.01I	0.03I	1.61I	-0.01I	0.00K
104.9	1.23I	-0.01I	0.02I	1.48I	-0.01I	0.00K
98.7	1.08I	-0.01I	0.02I	1.36I	-0.01I	0.00K
92.5	0.94I	-0.01I	0.02I	1.26I	-0.01I	0.00K
86.9	0.82I	0.00I	0.01I	1.16I	-0.01I	0.00K
81.3	0.71I	0.00I	0.01I	1.07I	-0.01I	0.00K
75.7	0.61I	0.00I	0.01I	0.98I	-0.01I	0.00K
70.1	0.52I	0.00I	0.01I	0.90I	0.00I	0.00K
64.5	0.43I	0.00I	0.01I	0.81I	0.00I	0.00K
58.9	0.36I	0.00I	0.00I	0.73I	0.00I	0.00K
53.2	0.29I	0.00I	0.00I	0.65I	0.00I	0.00K
45.2	0.21I	0.00E	0.00I	0.55I	0.00I	0.00K
39.6	0.16I	0.00E	0.00I	0.47I	0.00I	0.00K
33.9	0.11I	0.00E	0.00I	0.40I	0.00E	0.00K
28.3	0.08I	0.00E	0.00I	0.32I	0.00E	0.00K
22.6	0.05I	0.00E	0.00I	0.26I	0.00E	0.00K
17.0	0.03I	0.00E	0.00I	0.19I	0.00E	0.00K
11.3	0.01I	0.00E	0.00I	0.12I	0.00E	0.00K
5.7	0.00I	0.00E	0.00I	0.06I	0.00E	0.00K
0.0	0.00A	0.00A	0.00A	0.00A	0.00A	0.00A

MAXIMUM POLE FORCES CALCULATED(w.r.t. to wind direction)

MAST ELEV ft	TOTAL AXIAL kip	SHEAR.w.r.t.WIND.DIR ALONG kip	SHEAR.w.r.t.WIND.DIR ACROSS kip	MOMENT.w.r.t.WIND.DIR ALONG ft-kip	MOMENT.w.r.t.WIND.DIR ACROSS ft-kip	TORSION ft-kip
186.0						
	0.02 B	0.03 I	0.00 L	0.02 I	-0.01 L	0.00 L
	8.47 B	3.14 I	0.00 L	-2.88 I	0.01 L	0.00 L
180.3						
	8.47 A	3.14 A	-0.01 F	-2.87 I	0.02 L	0.00 B
	8.81 A	3.22 A	-0.01 F	-23.38 A	-0.03 B	0.00 I
174.6						
	8.81 A	3.23 L	-0.01 H	-23.38 A	-0.04 B	0.00 I
	14.74 A	5.71 L	-0.01 H	-50.17 A	0.07 E	0.01 I
168.9						
	14.74 A	5.70 F	-0.01 E	-50.16 A	0.08 E	0.01 I
	15.13 A	5.80 F	-0.01 E	-86.98 A	0.17 E	0.01 I
163.1						
	15.13 A	5.82 A	-0.01 C	-86.99 A	0.18 E	0.01 I
	21.03 A	7.98 A	-0.01 C	-132.68 A	0.25 E	0.02 I
157.4						
	21.04 F	7.98 E	-0.02 E	-132.67 A	0.25 E	0.02 I

	21.47 F	8.08 E	−0.02 E	−183.81 A	0.35 E	0.03 I
151.7						
	21.47 B	8.08 A	−0.02 L	−183.82 A	0.34 E	0.03 I
	27.35 B	9.93 A	−0.02 L	−245.18 A	0.46 E	0.04 I
146.0						
	27.35 I	9.95 L	−0.06 K	−245.30 A	0.43 E	0.04 I
	28.28 I	10.05 L	−0.06 K	−294.75 A	0.55 E	0.04 I
141.5						
	28.28 I	10.03 I	0.03 H	−294.87 A	0.54 E	0.04 I
	29.07 I	10.15 I	0.03 H	−362.83 A	0.64 E	0.04 I
135.4						
	29.08 I	10.17 B	−0.04 K	−362.86 A	0.65 E	0.04 I
	29.92 I	10.30 B	−0.04 K	−431.65 A	0.80 K	−0.05 K
129.3						
	29.93 I	10.31 B	−0.04 K	−431.67 A	0.80 K	−0.05 K
	30.80 I	10.44 B	−0.04 K	−501.08 A	1.04 K	−0.06 K
123.2						
	30.80 I	10.46 A	−0.06 I	−501.12 A	1.03 K	−0.06 K
	31.70 I	10.59 A	−0.06 I	−571.32 A	1.32 K	−0.07 K
117.1						
	31.70 I	10.58 A	−0.05 I	−571.32 A	1.34 K	−0.07 K
	32.65 I	10.73 A	−0.05 I	−642.01 A	1.54 K	−0.08 K
111.0						
	32.65 I	10.79 I	−0.08 I	−642.07 A	1.54 K	−0.08 K
	33.64 I	10.94 I	−0.08 I	−713.36 I	1.99 I	−0.08 K
104.9						
	33.64 I	10.91 I	−0.09 I	−713.38 I	1.96 I	−0.08 K
	34.64 I	11.05 I	−0.09 I	−785.53 I	2.52 I	−0.09 K
98.7						
	34.64 I	11.12 I	−0.12 I	−785.54 I	2.50 I	−0.09 K
	37.00 I	11.28 I	−0.12 I	−860.59 I	3.27 I	−0.09 K
92.5						
	37.01 I	11.24 I	−0.11 I	−860.54 I	3.28 I	−0.09 K
	38.19 I	11.38 I	−0.11 I	−928.36 I	3.90 I	−0.09 K
86.9						
	38.19 I	11.38 I	−0.09 I	−928.35 I	3.88 I	−0.09 K
	39.43 I	11.53 I	−0.09 I	−996.78 I	4.41 I	−0.10 K
81.3						
	39.43 I	11.53 L	−0.08 E	−996.83 I	4.41 I	−0.10 K
	40.68 I	11.67 L	−0.08 E	−1065.42 I	4.82 I	−0.10 K
75.7						
	40.68 I	11.65 I	−0.09 E	−1065.39 I	4.81 I	−0.10 K
	41.99 I	11.80 I	−0.09 E	−1134.90 I	5.30 I	−0.10 K
70.1						
	41.99 I	11.81 L	−0.09 E	−1134.84 I	5.29 I	−0.10 K
	43.33 I	11.97 L	−0.09 E	−1204.99 I	5.75 I	−0.10 K
64.5						
	43.33 I	11.97 I	−0.14 E	−1204.94 I	5.77 I	−0.10 K
	44.69 I	12.12 I	−0.14 E	−1275.82 I	6.37 I	−0.11 K
58.9						
	44.69 I	12.13 I	−0.13 E	−1275.80 I	6.35 I	−0.10 K
	46.10 I	12.29 I	−0.13 E	−1347.32 I	7.03 I	−0.10 K
53.2						
	46.10 I	12.29 I	−0.15 E	−1347.29 I	7.04 I	−0.10 K
	50.25 I	12.51 I	−0.15 E	−1450.55 I	7.90 I	−0.10 K
45.2						
	50.25 I	12.52 I	−0.14 E	−1450.55 I	7.91 I	−0.10 K
	51.78 I	12.69 I	−0.14 E	−1524.45 I	8.55 I	−0.10 K
39.6						
	51.78 I	12.68 I	−0.13 E	−1524.44 I	8.55 I	−0.10 K
	53.36 I	12.84 I	−0.13 E	−1598.87 I	9.26 I	−0.10 K

33.9						
	53.36 I	12.82 I	−0.15 E	−1598.89 I	9.26 I	−0.10 K
	54.95 I	12.97 I	−0.15 E	−1673.79 I	9.77 I	−0.11 K
28.3						
	54.95 I	12.99 I	−0.14 E	−1673.78 I	9.78 I	−0.11 K
	56.59 I	13.14 I	−0.14 E	−1749.28 I	10.27 I	−0.11 K
22.6						
	56.59 I	13.14 I	−0.14 E	−1749.27 I	10.26 I	−0.11 K
	58.25 I	13.28 I	−0.14 E	−1825.27 I	10.79 E	−0.11 K
17.0						
	58.25 I	13.29 I	−0.15 E	−1825.26 I	10.79 E	−0.11 K
	59.96 I	13.43 I	−0.15 E	−1901.76 I	11.65 E	−0.11 K
11.3						
	59.96 I	13.45 I	−0.15 E	−1901.76 I	11.65 E	−0.11 K
	61.73 I	13.59 I	−0.15 E	−1978.76 I	12.51 E	−0.11 K
5.7						
	61.73 I	13.59 I	−0.15 E	−1978.76 I	12.51 E	−0.11 K
	63.52 I	13.73 I	−0.15 E	−2056.21 I	13.37 E	−0.11 K
base reaction	63.52 I	−13.73 I	0.15 E	2056.21 I	−13.37 E	0.11 K

COMPLIANCE WITH 4.8.2 & 4.5.4

ELEV ft	AXIAL	BENDING	SHEAR + TORSIONAL	TOTAL	SATISFIED	D/t(w/t)	MAX ALLOWED
186.00							
	0.00B	0.00I	0.00I	0.00I	YES	12.17A	45.2
	0.01B	0.01I	0.01I	0.01I	YES	13.29A	45.2
180.29							
	0.01A	0.01I	0.01A	0.01I	YES	13.29A	45.2
	0.01A	0.04A	0.00A	0.05A	YES	14.41A	45.2
174.57							
	0.01A	0.04A	0.00L	0.05A	YES	14.41A	45.2
	0.01A	0.08A	0.01L	0.09A	YES	15.52A	45.2
168.86							
	0.01A	0.08A	0.01F	0.09A	YES	15.52A	45.2
	0.01A	0.12A	0.01F	0.13A	YES	16.64A	45.2
163.14							
	0.01A	0.12A	0.01A	0.13A	YES	16.64A	45.2
	0.01A	0.16A	0.01A	0.17A	YES	17.76A	45.2
157.43							
	0.01F	0.16A	0.01E	0.17A	YES	17.76A	45.2
	0.01F	0.20A	0.01E	0.21A	YES	18.88A	45.2
151.71							
	0.01B	0.20A	0.01A	0.21A	YES	18.88A	45.2
	0.02B	0.24A	0.01A	0.26A	YES	20.00A	45.2
146.00							
	0.01I	0.15A	0.01L	0.16A	YES	13.22A	45.2
	0.01I	0.17A	0.01L	0.18A	YES	13.80A	45.2
141.50							
	0.01I	0.18A	0.01I	0.19A	YES	13.57A	45.2
	0.01I	0.19A	0.01I	0.20A	YES	14.37A	45.2
135.39							
	0.01I	0.19A	0.01B	0.20A	YES	14.37A	45.2
	0.01I	0.21A	0.01B	0.22A	YES	15.16A	45.2
129.29							
	0.01I	0.21A	0.01B	0.22A	YES	15.16A	45.2
	0.01I	0.22A	0.01B	0.23B	YES	15.96A	45.2

123.18							
	0.01I	0.22A	0.01A	0.23A	YES	15.96A	45.2
	0.01I	0.23A	0.01A	0.24A	YES	16.76A	45.2
117.07							
	0.01I	0.23A	0.01A	0.24A	YES	16.76A	45.2
	0.01I	0.24A	0.01A	0.25A	YES	17.56A	45.2
110.96							
	0.01I	0.24A	0.01I	0.25A	YES	17.56A	45.2
	0.01I	0.25I	0.01I	0.26I	YES	18.35A	45.2
104.86							
	0.01I	0.25I	0.01I	0.26I	YES	18.35A	45.2
	0.01I	0.25I	0.01I	0.26I	YES	19.15A	45.2
98.75							
	0.01I	0.21I	0.01I	0.22I	YES	16.37A	45.2
	0.01I	0.21I	0.01I	0.22I	YES	17.06A	45.2
92.50							
	0.01I	0.22I	0.01I	0.23I	YES	16.76A	45.2
	0.01I	0.22I	0.01I	0.23I	YES	17.39A	45.2
86.89							
	0.01I	0.22I	0.01I	0.23I	YES	17.39A	45.2
	0.01I	0.22I	0.00I	0.23I	YES	18.02A	45.2
81.29							
	0.01I	0.22I	0.00L	0.23I	YES	18.02A	45.2
	0.01I	0.23I	0.00L	0.24I	YES	18.65A	45.2
75.68							
	0.01I	0.23I	0.00I	0.24I	YES	18.65A	45.2
	0.01I	0.23I	0.00I	0.24I	YES	19.27A	45.2
70.07							
	0.01I	0.23I	0.00L	0.24I	YES	19.27A	45.2
	0.01I	0.23I	0.00L	0.24I	YES	19.90A	45.2
64.46							
	0.01I	0.23I	0.00I	0.24I	YES	19.90A	45.2
	0.01I	0.23I	0.00I	0.24I	YES	20.53A	45.2
58.86							
	0.01I	0.23I	0.00I	0.24I	YES	20.53A	45.2
	0.01I	0.23I	0.00I	0.24I	YES	21.16A	45.2
53.25							
	0.01I	0.23I	0.00I	0.24I	YES	21.16A	45.2
	0.01I	0.24I	0.00I	0.24I	YES	22.05A	45.2
45.25							
	0.01I	0.24I	0.00I	0.25I	YES	21.70A	45.2
	0.01I	0.24I	0.00I	0.25I	YES	22.33A	45.2
39.59							
	0.01I	0.24I	0.00I	0.25I	YES	22.33A	45.2
	0.01I	0.24I	0.00I	0.25I	YES	22.96A	45.2
33.94							
	0.01I	0.24I	0.00I	0.25I	YES	22.96A	45.2
	0.01I	0.24I	0.00I	0.25I	YES	23.60A	45.2
28.28							
	0.01I	0.24I	0.00I	0.25I	YES	23.60A	45.2
	0.01I	0.24I	0.00I	0.25I	YES	24.23A	45.2
22.62							
	0.01I	0.24I	0.00I	0.25I	YES	24.23A	45.2
	0.01I	0.24I	0.00I	0.25I	YES	24.86A	45.2
16.97							
	0.01I	0.24I	0.00I	0.25I	YES	24.86A	45.2
	0.01I	0.25I	0.00I	0.26I	YES	25.50A	45.2
11.31							
	0.01I	0.25I	0.00I	0.26I	YES	25.50A	45.2
	0.01I	0.25I	0.00I	0.26I	YES	26.13A	45.2
5.66							

```
           0.01I     0.25I     0.00I     0.26I     YES      26.13A       45.2

           0.01I     0.25I     0.00I     0.26I     YES      26.76A       45.2
  0.00  ..................................................................
```

MAXIMUM LOADS ONTO FOUNDATION(w.r.t. wind direction)

DOWN	SHEAR.w.r.t.WIND.DIR ALONG	SHEAR.w.r.t.WIND.DIR ACROSS	MOMENT.w.r.t.WIND.DIR ALONG	MOMENT.w.r.t.WIND.DIR ACROSS	TORSION
kip	kip	kip	ft-kip	ft-kip	ft-kip
63.52	13.73	-0.15	-2056.21	13.37	-0.11
I	I	E	I	E	K

Seismic Load Effects
Equivalent Lateral Force Procedure
ANSI/TIA-222-H

Parameters	
Risk Category	II
R	1.500
S_S	0.124
S_1	0.042
Site Class	D (default)
T_L (sec)	8.000
F_a	1.600
F_v	2.400
S_{MS}	0.198
S_{M1}	0.101
S_{DS}	0.132
S_{D1}	0.067
T_s	0.508
I_e	1.000
Ω	1.500
C_S	0.030
E (ksi)	29,000
I_{top} (in^4)	663
I_{bot} (in^4)	55,517
I_{avg} (in^4)	28,090
g (in/s^2)	386.4
W_t (kips)	63.399
W_u (kips)	20.515
W_L (kips)	42.884
L_p (in)	2232
f_1 (Hertz)	0.265
T (sec)	3.774
k_e	2.0000
V_s (kips)	1.902
Seismic Design Category	B

Vertical Distribution of Seismic Forces

Description	h_i (ft.)	w_i (kips)	W_u (kips)	$w_i h_i^{ke}$	F_{sz} or E_h (kips)	E_V (kips)	1.2 D + 1.0 E_V (kips)	0.9 D - 1.0 E_V (kips)
Antenna Load	186.00	0.0150	0.0150	518.9400	0.0010	0.0004	0.0184	0.0131
Step Bolts/Safety Climb Load	183.00	0.0084	0.0000	281.3076	0.0006	0.0002	0.0103	0.0074
Antenna Load	181.00	7.0000	7.0000	229,327.0000	0.4511	0.1848	8.5848	6.1152
Line Deadload	181.00	1.1294	0.0000	37,000.2734	0.0728	0.0298	1.3851	0.9867
Step Bolts/Safety Climb Load	175.00	0.0140	0.0000	428.7500	0.0008	0.0004	0.0172	0.0122
Antenna Load	171.00	4.5000	4.5000	131,584.5000	0.2588	0.1188	5.5188	3.9312
Line Deadload	171.00	1.0670	0.0000	31,200.1470	0.0614	0.0282	1.3086	0.9321
Step Bolts/Safety Climb Load	165.00	0.0140	0.0000	381.1500	0.0007	0.0004	0.0172	0.0122
Structure - Section 1	163.75	2.9875	0.0000	80,107.0117	0.1576	0.0789	3.6639	2.6098
Antenna Load	161.00	4.5000	4.5000	116,644.5000	0.2294	0.1188	5.5188	3.9312
Line Deadload	161.00	1.0046	0.0000	26,040.2366	0.0512	0.0265	1.2320	0.8776
Step Bolts/Safety Climb Load	155.00	0.0140	0.0000	336.3500	0.0007	0.0004	0.0172	0.0122
Antenna Load	151.00	4.5000	4.5000	102,604.5000	0.2018	0.1188	5.5188	3.9312
Line Deadload	151.00	0.9422	0.0000	21,483.1022	0.0423	0.0249	1.1555	0.8231
Step Bolts/Safety Climb Load	145.00	0.0140	0.0000	294.3500	0.0006	0.0004	0.0172	0.0122
Step Bolts/Safety Climb Load	135.00	0.0140	0.0000	255.1500	0.0005	0.0004	0.0172	0.0122
Step Bolts/Safety Climb Load	125.00	0.0140	0.0000	218.7500	0.0004	0.0004	0.0172	0.0122
Structure - Section 2	119.25	7.9236	0.0000	112,678.0490	0.2216	0.2092	9.7175	6.9220
Step Bolts/Safety Climb Load	115.00	0.0140	0.0000	185.1500	0.0004	0.0004	0.0172	0.0122
Step Bolts/Safety Climb Load	105.00	0.0140	0.0000	154.3500	0.0003	0.0004	0.0172	0.0122
Step Bolts/Safety Climb Load	95.00	0.0140	0.0000	126.3500	0.0002	0.0004	0.0172	0.0122
Step Bolts/Safety Climb Load	85.00	0.0140	0.0000	101.1500	0.0002	0.0004	0.0172	0.0122
Step Bolts/Safety Climb Load	75.00	0.0140	0.0000	78.7500	0.0002	0.0004	0.0172	0.0122
Structure - Section 3	72.00	12.3488	0.0000	64,016.1792	0.1259	0.3260	15.1446	10.7879
Step Bolts/Safety Climb Load	65.00	0.0140	0.0000	59.1500	0.0001	0.0004	0.0172	0.0122
Step Bolts/Safety Climb Load	55.00	0.0140	0.0000	42.3500	0.0001	0.0004	0.0172	0.0122
Step Bolts/Safety Climb Load	45.00	0.0140	0.0000	28.3500	0.0001	0.0004	0.0172	0.0122
Step Bolts/Safety Climb Load	35.00	0.0140	0.0000	17.1500	0.0000	0.0004	0.0172	0.0122
Structure - Section 4	26.62	15.2348	0.0000	10,795.7510	0.0212	0.4022	18.6840	13.3091
Step Bolts/Safety Climb Load	25.00	0.0140	0.0000	8.7500	0.0000	0.0004	0.0172	0.0122
Step Bolts/Safety Climb Load	15.00	0.0140	0.0000	3.1500	0.0000	0.0004	0.0172	0.0122
Σ		63.40	20.5150	967,000.65	1.90	1.67	77.75	55.39

SO#: 26-2535-TLJ
Site Name: MD-003 Whitehall, MD
Date: 10/24/2025

Round Flange Plate and Bolts per ANSI/TIA 222-H
Elevation = 186 feet

Pole Data

Diameter:	19	in
Thickness:	0.25	in
Yield (Fy):	65	ksi
# of Sides:	18	"0" IF Round
Strength (Fu):	80	ksi

Reactions

Moment, Mu:	424	ft-kips
Axial, Pu:	0	kips
Shear, Vu:	0	kips

Bolt Data

Quantity:	18	
Diameter:	1	in
Bolt Material:	A325	
Strength (Fu):	120	ksi
Yield (Fy):	92	ksi
BC Diam. (in):	22.25	BC Override:

Flange Bolt Results

Allowable Φ*Rnt:	54.54	kips
Adjusted Φ*Rnt (due to shear):	54.54	kips
Maximum Bolt Tension:	50.82	kips
Bolt Interaction Ratio:	**93.2%**	**Pass**

Plate Data

Diameter (in):	24.75	Dia. Override:
Thickness:	1.5	in
Center Hole Diam.:	13	in
Yield (Fy):	50	ksi
Single-Rod B-eff:	3.35	in
Drain Hole:	1	in. diameter
Drain Location:	8.5	in. center of pole to center of drain hole

Flange Plate Results

Compression Side Plate (Mu/Z):	26.7	ksi
Allowable Φ*Fy:	45.0	ksi
Compr. Plate Interaction Ratio:	**59.3%**	**Pass**

SO#: 26-2535-TLJ
Site Name: MD-003 Whitehall, MD
Date: 10/24/2025

Round Base Plate and Anchor Rods, per ANSI/TIA 222-H

Pole Data

Diameter:	68.530	in (flat to flat)
Thickness:	0.4375	in
Yield (Fy):	65	ksi
# of Sides:	18	"0" IF Round
Strength (Fu):	80	ksi

Reactions

Moment, Mu:	8054.65	ft-kips
Axial, Pu:	76.12	kips
Shear, Vu:	53.73	kips

Anchor Rod Data

Quantity:	22		
Diameter:	2.25	in	
Rod Material:	A615		
Strength (Fu):	100	ksi	
Yield (Fy):	75	ksi	
BC Diam. (in):	75.75	BC Override:	

Anchor Rod Results

(per 4.9.9)

Maximum Put:	229.40	Kips
Φt*Rnt:	243.75	Kips
Vu:	2.44	Kips
Φv*Rnv:	149.10	Kips
Tension Interaction Ratio:	0.89	
Maximum Puc:	235.46	Kips
Φc*Rnc:	268.39	Kips
Vu:	2.44	Kips
Φc*Rnvc:	120.77	Kips
Compression Interaction Ratio:	0.88	
Maximum Interaction Ratio:	**88.6%**	**Pass**

Plate Data

Diameter (in):	81.5	Dia. Override:	
Thickness:	2.25	in	
Yield (Fy):	50	ksi	
Eff Width/Rod:	9.89	in	
Drain Hole:	2.625	in. diameter	
Drain Location:	32.25	in. center of pole to center of drain hole	
Center Hole:	56.5	in. diameter	

Base Plate Results

Base Plate (Mu/Z):	43.8	ksi
Allowable Φ*Fy:	45.0	ksi (per AISC)
Base Plate Interaction Ratio:	**97.3%**	**Pass**

MAT FOUNDATION DESIGN BY SABRE INDUSTRIES

187' Monopole ARCOLA TOWERS MD-003 Whitehall, MD (26-2535-TLJ) 10/24/25 TTW

Overall Loads:			
Factored Moment (ft-kips)	8054.65		
Factored Axial (kips)	76.12		
Factored Shear (kips)	53.73		
Bearing Design Strength (ksf)	3.75	Max. Net Bearing Press. (ksf)	3.29
Water Table Below Grade (ft)	**999**		
Width of Mat (ft)	31	Allowable Bearing Pressure (ksf)	**2.50**
Thickness of Mat (ft)	2	Safety Factor	**2.00**
Depth to Bottom of Slab (ft)	6	Ultimate Bearing Pressure (ksf)	5.00
Quantity of Bolts in Bolt Circle	22	Bearing Φs	0.75
Bolt Circle Diameter (in)	75.75		
Effective Anchor Bolt Embedment (in)	66.5		
Diameter of Pier (ft)	8	Minimum Pier Diameter (ft)	8.00
Ht. of Pier Above Ground (ft)	0.5	Equivalent Square b (ft)	7.09
Ht. of Pier Below Ground (ft)	4	Square Pier? (Y/N)	**N**
Quantity of Bars in Mat	53		
Bar Diameter in Mat (in)	1.27		
Area of Bars in Mat (in^2)	67.14		
Spacing of Bars in Mat (in)	7.01	Recommended Spacing (in)	5 to 12
Quantity of Bars Pier	54		
Bar Diameter in Pier (in)	1.128		
Tie Bar Diameter in Pier (in)	0.625		
Spacing of Ties (in)	4		
Area of Bars in Pier (in^2)	53.96	Minimum Pier A_s (in^2)	36.19
Spacing of Bars in Pier (in)	5.10	Recommended Spacing (in)	5 to 12
f'c (ksi)	4.5		
fy (ksi)	60		
Unit Wt. of Soil (kcf)	**0.11**		
Unit Wt. of Concrete (kcf)	0.15		
Volume of Concrete (yd^3)	79.56		
Two-Way Shear Action:			
Average d (in)	19.73		
ϕv_c (ksi)	0.195	v_u (ksi)	0.146
$\phi v_c = \phi(2 + 4/\beta_c)f'_c{}^{1/2}$	0.302		
$\phi v_c = \phi(\alpha_s d/b_o+2)f'_c{}^{1/2}$	0.195	J (in^3)	1.528E+07
$\phi v_c = \phi 4f'_c{}^{1/2}$	0.201	c + d (in)	104.81
Shear perimeter, b_o (in)	419.23	$0.40M_{sc}$ (ft-kips)	3318.6
β_c	1		
One-Way Shear:			
ϕV_c (kips)	738.5	V_u (kips)	482.2
Stability:			
Overturning Design Strength (ft-k)	10970.1	Total Applied M (ft-k)	8403.9

Pier-Slab Transfer by Flexure:

b_{slab} (ft)	14.00		
ϕM_n (ft-kips)	4997.8	$0.60M_{sc}$ (ft-kips)	4977.9

Pier Design:

ϕV_n (kips)	1275.9	V_u (kips)	53.7
$\phi V_c=\phi 2(1+N_u/(2000A_g))f'_c{}^{1/2}b_wd$	745.8		
V_s (kips)	706.9	*** V_s max = 4 $f'_c{}^{1/2}b_wd$ (kips)	1978.3
Maximum Spacing (in)	7.62	(Only if Shear Ties are Required)	
Actual Hook Development (in)	18.46	Req'd Hook Development l_{dh} (in) - Tension	14.12
		Req'd Hook Development l_{dc} (in) - Compression	15.23

Flexure in Slab:

ϕM_n (ft-kips)	5533.2	M_u (ft-kips)	3974.7
a (in)	2.83		
Steel Ratio	0.00915		
β_1	0.825		
Maximum Steel Ratio (ρ_t)	0.0197		
Minimum Steel Ratio	0.0018		
Rebar Development in Pad (in)	135.00	Required Development in Pad (in)	34.08

Condition	1 is OK, 0 Fails
Maximum Soil Bearing Pressure	1
Pier Area of Steel	1
Pier Shear	1
Interaction Diagram	1
Two-Way Shear Action	1
One-Way Shear Action	1
Overturning	1
Flexure	1
Steel Ratio	1
Length of Development in Pad	1
Hook Development	1
Anchor Bolt Pullout	1
Anchor Bolt Punching Shear	1

```
================================================================================

                    LPile for Windows, Version 2019-11.009

               Analysis of Individual Piles and Drilled Shafts
               Subjected to Lateral Loading Using the p-y Method
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================================================================================

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--------------------------------------------------------------------------------
                            Files Used for Analysis
--------------------------------------------------------------------------------

Path to file locations:
\Program Files (x86)\Ensoft\Lpile2019\files\

Name of input data file:
26-2535-TLJ.lp11d

Name of output report file:
26-2535-TLJ.lp11o

Name of plot output file:
26-2535-TLJ.lp11p

Name of runtime message file:
26-2535-TLJ.lp11r


--------------------------------------------------------------------------------
                           Date and Time of Analysis
--------------------------------------------------------------------------------

               Date:  October 24, 2025            Time:   9:58:50


--------------------------------------------------------------------------------
                                  Problem Title
--------------------------------------------------------------------------------

Site         : MD-003 Whitehall, MD


Tower        : 187' Monopole


Prepared for : ARCOLA TOWERS


Job Number   : 26-2535-TLJ


Engineer     : TTW



--------------------------------------------------------------------------------
                         Program Options and Settings
--------------------------------------------------------------------------------

Computational Options:
```

```
 - Conventional Analysis
Engineering Units Used for Data Input and Computations:
 - US Customary System Units (pounds, feet, inches)

Analysis Control Options:
 - Maximum number of iterations allowed                =          999
 - Deflection tolerance for convergence                =   1.0000E-05 in
 - Maximum allowable deflection                        =     100.0000 in
 - Number of pile increments                           =          100

Loading Type and Number of Cycles of Loading:
 - Static loading specified

 - Use of p-y modification factors for p-y curves not selected
 - Analysis uses layering correction (Method of Georgiadis)
 - No distributed lateral loads are entered
 - Loading by lateral soil movements acting on pile not selected
 - Input of shear resistance at the pile tip not selected
 - Input of moment resistance at the pile tip not selected
 - Input of side resistance moment along pile not selected
 - Computation of pile-head foundation stiffness matrix not selected
 - Push-over analysis of pile not selected
 - Buckling analysis of pile not selected

Output Options:
 - Output files use decimal points to denote decimal symbols.
 - Report only summary tables of pile-head deflection, maximum bending moment,
   and maximum shear force in output report file.
 - No p-y curves to be computed and reported for user-specified depths
 - Print using wide report formats



--------------------------------------------------------------------------------
                     Pile Structural Properties and Geometry
--------------------------------------------------------------------------------

Number of pile sections defined                        =           1
Total length of pile                                   =      40.500 ft
Depth of ground surface below top of pile              =      0.5000 ft

Pile diameters used for p-y curve computations are defined using 2 points.

p-y curves are computed using pile diameter values interpolated with depth over
the length of the pile. A summary of values of pile diameter vs. depth follows.

             Depth Below          Pile
Point         Pile Head         Diameter
 No.             feet            inches
-----      -------------     -------------
  1             0.000           96.0000
  2            40.500           96.0000


Input Structural Properties for Pile Sections:
----------------------------------------------

Pile Section No. 1:

   Section 1 is a round drilled shaft, bored pile, or CIDH pile
   Length of section                                   =    40.500000 ft
   Shaft Diameter                                      =    96.000000 in
   Shear capacity of section                           =       0.0000 lbs



--------------------------------------------------------------------------------
                       Ground Slope and Pile Batter Angles
--------------------------------------------------------------------------------

Ground Slope Angle                                     =        0.000 degrees
                                                       =        0.000 radians

Pile Batter Angle                                      =        0.000 degrees
                                                       =        0.000 radians


--------------------------------------------------------------------------------
                          Soil and Rock Layering Information
```

The soil profile is modelled using 1 layers

Layer 1 is stiff clay without free water

Distance from top of pile to top of layer	=	0.500000 ft
Distance from top of pile to bottom of layer	=	60.500000 ft
Effective unit weight at top of layer	=	110.000000 pcf
Effective unit weight at bottom of layer	=	110.000000 pcf
Undrained cohesion at top of layer	=	1000.000000 psf
Undrained cohesion at bottom of layer	=	1000.000000 psf
Epsilon-50 at top of layer	=	0.010000
Epsilon-50 at bottom of layer	=	0.010000

(Depth of the lowest soil layer extends 20.000 ft below the pile tip)

Summary of Input Soil Properties

Layer Num.	Soil Type Name (p-y Curve Type)	Layer Depth ft	Effective Unit Wt. pcf	Cohesion psf	E50 or krm
1	Stiff Clay w/o Free Water	0.5000 60.5000	110.0000 110.0000	1000.0000 1000.0000	0.01000 0.01000

Static Loading Type

Static loading criteria were used when computing p-y curves for all analyses.

Pile-head Loading and Pile-head Fixity Conditions

Number of loads specified = 2

Load No.	Load Type	Condition 1	Condition 2	Axial Thrust Force, lbs	Compute Top y vs. Pile Length	Run Analysis
1	1	V = 71640. lbs	M = 128874400. in-lbs	101493.	No	Yes
2	1	V = 13730. lbs	M = 24674520. in-lbs	63520.	No	Yes

V = shear force applied normal to pile axis
M = bending moment applied to pile head
y = lateral deflection normal to pile axis
S = pile slope relative to original pile batter angle
R = rotational stiffness applied to pile head
Values of top y vs. pile lengths can be computed only for load types with
specified shear loading (Load Types 1, 2, and 3).
Thrust force is assumed to be acting axially for all pile batter angles.

Computations of Nominal Moment Capacity and Nonlinear Bending Stiffness

Axial thrust force values were determined from pile-head loading conditions

Number of Pile Sections Analyzed = 1

Pile Section No. 1:

Dimensions and Properties of Drilled Shaft (Bored Pile):

Length of Section	=	40.500000 ft
Shaft Diameter	=	96.000000 in
Concrete Cover Thickness (to edge of long. rebar)	=	3.625000 in
Number of Reinforcing Bars	=	54 bars
Yield Stress of Reinforcing Bars	=	60000. psi
Modulus of Elasticity of Reinforcing Bars	=	29000000. psi
Gross Area of Shaft	=	7238. sq. in.
Total Area of Reinforcing Steel	=	53.963715 sq. in.
Area Ratio of Steel Reinforcement	=	0.75 percent
Edge-to-Edge Bar Spacing	=	3.966766 in
Maximum Concrete Aggregate Size	=	0.750000 in
Ratio of Bar Spacing to Aggregate Size	=	5.29
Offset of Center of Rebar Cage from Center of Pile	=	0.0000 in

Axial Structural Capacities:

Nom. Axial Structural Capacity = 0.85 Fc Ac + Fy As	=	30717.639 kips
Tensile Load for Cracking of Concrete	=	-3343.589 kips
Nominal Axial Tensile Capacity	=	-3237.823 kips

Reinforcing Bar Dimensions and Positions Used in Computations:

Bar Number	Bar Diam. inches	Bar Area sq. in.	X inches	Y inches
1	1.128000	0.999328	43.811000	0.00000
2	1.128000	0.999328	43.514766	5.086147
3	1.128000	0.999328	42.630069	10.103512
4	1.128000	0.999328	41.168873	14.984244
5	1.128000	0.999328	39.150940	19.662341
6	1.128000	0.999328	36.603557	24.074538
7	1.128000	0.999328	33.561173	28.161168
8	1.128000	0.999328	30.064932	31.866967
9	1.128000	0.999328	26.162115	35.141819
10	1.128000	0.999328	21.905500	37.941439
11	1.128000	0.999328	17.352651	40.227966
12	1.128000	0.999328	12.565136	41.970479
13	1.128000	0.999328	7.607700	43.145412
14	1.128000	0.999328	2.547383	43.736879
15	1.128000	0.999328	-2.547383	43.736879
16	1.128000	0.999328	-7.607700	43.145412
17	1.128000	0.999328	-12.565136	41.970479
18	1.128000	0.999328	-17.352651	40.227966
19	1.128000	0.999328	-21.905500	37.941439
20	1.128000	0.999328	-26.162115	35.141819
21	1.128000	0.999328	-30.064932	31.866967
22	1.128000	0.999328	-33.561173	28.161168
23	1.128000	0.999328	-36.603557	24.074538
24	1.128000	0.999328	-39.150940	19.662341
25	1.128000	0.999328	-41.168873	14.984244
26	1.128000	0.999328	-42.630069	10.103512
27	1.128000	0.999328	-43.514766	5.086147
28	1.128000	0.999328	-43.811000	0.00000
29	1.128000	0.999328	-43.514766	-5.086147
30	1.128000	0.999328	-42.630069	-10.103512
31	1.128000	0.999328	-41.168873	-14.984244
32	1.128000	0.999328	-39.150940	-19.662341
33	1.128000	0.999328	-36.603557	-24.074538
34	1.128000	0.999328	-33.561173	-28.161168
35	1.128000	0.999328	-30.064932	-31.866967
36	1.128000	0.999328	-26.162115	-35.141819
37	1.128000	0.999328	-21.905500	-37.941439
38	1.128000	0.999328	-17.352651	-40.227966
39	1.128000	0.999328	-12.565136	-41.970479
40	1.128000	0.999328	-7.607700	-43.145412
41	1.128000	0.999328	-2.547383	-43.736879
42	1.128000	0.999328	2.547383	-43.736879
43	1.128000	0.999328	7.607700	-43.145412
44	1.128000	0.999328	12.565136	-41.970479
45	1.128000	0.999328	17.352651	-40.227966
46	1.128000	0.999328	21.905500	-37.941439
47	1.128000	0.999328	26.162115	-35.141819
48	1.128000	0.999328	30.064932	-31.866967
49	1.128000	0.999328	33.561173	-28.161168
50	1.128000	0.999328	36.603557	-24.074538
51	1.128000	0.999328	39.150940	-19.662341
52	1.128000	0.999328	41.168873	-14.984244
53	1.128000	0.999328	42.630069	-10.103512
54	1.128000	0.999328	43.514766	-5.086147

NOTE: The positions of the above rebars were computed by LPile

Minimum spacing between any two bars not equal to zero = 3.967 inches
between bars 46 and 47.

Ratio of bar spacing to maximum aggregate size = 5.29

Concrete Properties:

Compressive Strength of Concrete	=	4500. psi
Modulus of Elasticity of Concrete	=	3823676. psi
Modulus of Rupture of Concrete	=	-503.115295 psi
Compression Strain at Peak Stress	=	0.002001
Tensile Strain at Fracture of Concrete	=	-0.0001152
Maximum Coarse Aggregate Size	=	0.750000 in

Number of Axial Thrust Force Values Determined from Pile-head Loadings = 2

Number	Axial Thrust Force kips
1	63.520
2	101.493

Summary of Results for Nominal Moment Capacity for Section 1

Moment values interpolated at maximum compressive strain = 0.003
or maximum developed moment if pile fails at smaller strains.

Load No.	Axial Thrust kips	Nominal Mom. Cap. in-kip	Max. Comp. Strain
1	63.520	134176.704	0.00300000
2	101.493	135538.710	0.00300000

Note that the values of moment capacity in the table above are not
factored by a strength reduction factor (phi-factor).

In ACI 318, the value of the strength reduction factor depends on whether
the transverse reinforcing steel bars are tied hoops (0.65) or spirals (0.75).

The above values should be multiplied by the appropriate strength reduction
factor to compute ultimate moment capacity according to ACI 318,
or the value required by the design standard being followed.

The following table presents factored moment capacities and corresponding
bending stiffnesses computed for common resistance factor values used for
reinforced concrete sections.

Axial Load No.	Resist. Factor	Nominal Ax. Thrust kips	Nominal Moment Cap in-kips	Ult. (Fac) Ax. Thrust kips	Ult. (Fac) Moment Cap in-kips	Bend. Stiff. at Ult Mom kip-in^2
1	0.65	63.520000	134177.	41.288000	87215.	3.1892E+09
2	0.65	101.493333	135539.	65.970667	88100.	3.2257E+09
1	0.75	63.520000	134177.	47.640000	100633.	3.0731E+09
2	0.75	101.493333	135539.	76.120000	101654.	3.1092E+09
1	0.90	63.520000	134177.	57.168000	120759.	2.0005E+09
2	0.90	101.493333	135539.	91.344000	121985.	2.0258E+09

Summary of Pile-head Responses for Conventional Analyses

Definitions of Pile-head Loading Conditions:

Load Type 1: Load 1 = Shear, V, lbs, and Load 2 = Moment, M, in-lbs
Load Type 2: Load 1 = Shear, V, lbs, and Load 2 = Slope, S, radians
Load Type 3: Load 1 = Shear, V, lbs, and Load 2 = Rot. Stiffness, R, in-lbs/rad.

Load Type 4: Load 1 = Top Deflection, y, inches, and Load 2 = Moment, M, in-lbs
Load Type 5: Load 1 = Top Deflection, y, inches, and Load 2 = Slope, S, radians

Load Case No.	Load Type 1	Pile-head Load 1	Load Type 2	Pile-head Load 2	Axial Loading lbs	Pile-head Deflection inches	Pile-head Rotation radians	Max Shear in Pile lbs	Max Moment in Pile in-lbs
1	V, lb	71640.	M, in-lb	1.29E+08	101493.	18.7608	-0.07350	-572272.	1.31E+08
2	V, lb	13730.	M, in-lb	2.47E+07	63520.	0.05434	-3.59E-04	-100368.	2.50E+07

Maximum pile-head deflection = 18.7608261487 inches
Maximum pile-head rotation = -0.0735009674 radians = -4.211295 deg.

The analysis ended normally.

IBC 1807.3.2.1

Moment (ft·k)	8,054.65	
Shear (k)	53.73	
Caisson diameter (ft)	8	
Caisson height above ground (ft)	0.5	
Caisson height below ground (ft)	32	
Lateral soil pressure (lb/ft^2)	**300.00**	
Ground to application of force, h (ft)	150.41	
Applied lateral force, P (lb)	53,730	
Lateral soil bearing pressure, S_1 (lb/ft)	3,200.00	
Diameter, b (ft)	8	
A	4.91	$= (2.34P)/(S_1 b)$
Minimum depth of embedment, d (ft)	30.94	$= 0.5A[\ 1 + (\ 1 + (\ 4.36h / A\ )\ )^{1/2}\ ]$

Exhibit “4”

STATE OF MARYLAND)
) **AFFIDAVIT OF CHRISTIAN WINKLER**
COUNTY OF WASHINGTON)

I, Christian Winkler, being duly sworn, hereby state and affirm as follows:

1. This affidavit is based on my knowledge and I am competent to testify regarding those things about which I have knowledge.

2. I am the President of Arcola Towers.

3. Arcola Towers will be applying to Washington County for a proposed telecommunications facility at 10944 White Hall Road, Smithsburg, MD 21783, the property of James H. Miller and Elaine K. Miller, Account #18013932 for Verizon Wireless.

4. Pursuant to Section 4.22.6 of the Washington County Zoning Ordinance, I confirm and agree that one ten (10) foot space on the proposed tower will be specifically reserved for use by Washington County, and that other spaces will be made available to other future users, when possible.

Further affiant sayeth not.

Christian Winkler

SWORN to me this 5th day
of November 2025

Sandra K. Long

Notary Public for Commonwealth of Virginia

My Commission Expires 6-30-2026

SANDRA K LONG - NOTARY PUBLIC
REG# 317958
MY COMM. EXP. 6/30/2026
COMMONWEALTH OF VIRGINIA

Exhibit “5”

October 24, 2025

Ryan Foltz
Arcola Towers

RE: Proposed 187' Sabre Monopole for MD-003 Whitehall, MD

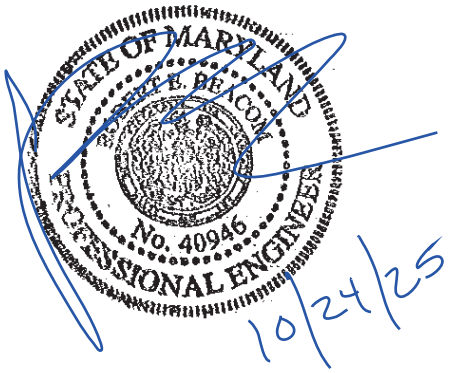
Dear Mr. Foltz,

Upon receipt of order, we propose to design and supply the above referenced Sabre monopole for an Ultimate Wind Speed of 112 mph without ice and 40 mph + 1" ice, Risk Category II, Exposure Category C, and Topographic Category 1, in accordance with the Telecommunications Industry Association Standard ANSI/TIA 222-H-2017 "Structural Standard for Antenna Supporting Structures and Antennas".

When designed according to this standard, the wind pressures and steel strength capacities include several safety factors. Therefore, it is highly unlikely that the monopole will fail structurally in a wind event where the design wind speed is exceeded within the range of the built-in safety factors.

Should the wind speed increase beyond the capacity of the built-in safety factors, to the point of failure of one or more structural elements, the most likely location of the failure would be within the monopole shaft, above the base plate. Assuming that the wind pressure profile is similar to that used to design the monopole, the monopole will buckle at the location of the highest combined stress ratio within the monopole shaft. This is likely to result in the portion of the monopole above leaning over and remaining in a permanently deformed condition. This would effectively result in a fall radius of 187' at ground level. *Please note that this letter only applies to the above referenced monopole designed and manufactured by Sabre Industries.*

Sincerely,



Robert E. Beacom, P.E., S.E.
Engineering Manager

Sabre Industries, Inc.

7101 Southbridge Drive
Sioux City, IA 51111

Exhibit “6”

Mail Processing Center
Federal Aviation Administration
Southwest Regional Office
Obstruction Evaluation Group
10101 Hillwood Parkway
Fort Worth, TX 76177

Aeronautical Study No.
2025-AEA-12000-OE

Issued Date: 01/21/2026

ARCOLA TOWERS
RYAN FOLTZ
112 W WASHINGTON ST STE 201
MIDDLEBURG, VA 20117

**** DETERMINATION OF NO HAZARD TO AIR NAVIGATION ****

The Federal Aviation Administration has conducted an aeronautical study under the provisions of 49 U.S.C., Section 44718 and if applicable Title 14 of the Code of Federal Regulations, part 77, concerning:

Structure: Monopole ARCOLA TOWERS - WHITEHALL
County, State: Washington, Maryland

Collected Point(s):

Label	Latitude	Longitude	SE	DET AGL	AMSL
pt-1	39-36-51.69N	77-38-45.77W	605 Ft	189 Ft	794 Ft

This aeronautical study revealed that the structure does not exceed obstruction standards and would not be a hazard to air navigation provided the following condition(s), if any, is(are) met:

Emissions from this site must be in compliance with the parameters set by collaboration between the FAA and telecommunications companies and reflected in the FAA 5G C band compatibility evaluation process (such as power, frequencies, and tilt angle). Operational use of this frequency band is not objectionable provided the Wireless Providers (WP) obtain and adhere to the parameters established by the FAA 5G C band compatibility evaluation process. **Failure to comply with this condition will void this determination of no hazard.**

See attachment for additional condition(s) or information.
Based on this evaluation, marking and lighting are not necessary for aviation safety. However, if marking/ lighting are accomplished on a voluntary basis, we recommend it be installed in accordance with FAA Advisory circular 70/7460-1 M Change 1.

This determination is based, in part, on the foregoing description which includes specific coordinates, heights, frequency(ies) and power. Any changes in coordinates, heights, and frequencies or use of greater power, except those frequencies specified in the Colo Void Clause Coalition; Antenna System Co-Location; Voluntary Best Practices, will void this determination. Any future construction or alteration, including increase to heights, power, or the addition of other transmitters, requires separate notice to the FAA.This determination includes all previously filed frequencies and power for this structure.

If construction or alteration is dismantled or destroyed, you must submit notice to the FAA within 5 days after the construction or alteration is dismantled or destroyed.

This determination does include temporary construction equipment such as cranes, derricks, etc., which may be used during actual construction of the structure. However, this equipment shall not exceed the overall heights as indicated above. Equipment which has a height greater than the studied structure requires separate notice to the FAA.

This determination concerns the effect of this structure on the safe and efficient use of navigable airspace by aircraft and does not relieve the sponsor of compliance responsibilities relating to any law, ordinance, or regulation of any Federal, State, or local government body.

A copy of this determination will be forwarded to the Federal Communications Commission (FCC) because the structure is subject to their licensing authority.

If we can be of further assistance, please contact our office at 1-817-222-5915, or Rodney.H-CTR.Love@faa.gov. On any future correspondence concerning this matter, please refer to Aeronautical Study Number 2025-AEA-12000-OE.

Signature Control No: 677612029-690948273 (DNE)
Rodney Love
Technician

Attachment(s)
Additional Information
Frequency Data
Map(s)

cc: FCC

BASIS FOR DECISION:

Part 77 authorizes the FAA to evaluate a structure or object's potential electromagnetic effects on air navigation, communication facilities, and other surveillance systems. It also authorizes study of impact on arrival, departure, and en route procedures for aircraft operating under visual or instrument flight rules, as well as the impact on airport traffic capacity at existing public use airports. Broadcast in the 3.7 to 3.98 GHz frequency (5G C band) currently causes errors in certain aircraft radio altimeters and the FAA has determined they cannot be relied upon to perform their intended function when experiencing interference from wireless broadband operations in the 5G C band. The FAA has adopted Airworthiness Directives for all transport and commuter category aircraft equipped with radio altimeters that prohibit certain operations when in the presence of 5G C band.

This determination of no hazard is based upon those mitigations implemented by the FAA and operators of transport and commuter category aircraft, and helicopters operating in the vicinity of your proposed location. It is also based on telecommunication industry and FAA collaboration on acceptable power levels and other parameters as reflected in the FAA 5G C band evaluation process.

The FAA 5G C band compatibility evaluation is a data analytics system used by FAA to evaluate operational hazards related to aircraft design. The FAA 5G C band compatibility evaluation process refers to the process in which the telecommunication companies and the FAA have set parameters, such as power output, locations, frequencies, and tilt angles for antenna that mitigate the hazard to aviation. As the telecommunication companies and FAA refine the tools and methodology, the allowable frequencies and power levels may change in the FAA 5G C band compatibility evaluation process. Therefore, your proposal will not have a substantial adverse effect on the safe and efficient use of the navigable airspace by aircraft provided the equipment and emissions are in compliance with the parameters established through the FAA 5G C band compatibility evaluation process.

Any future changes that are not consistent with the parameters listed in the FAA 5G C band compatibility evaluation process will void this determination of no hazard.

The FAA recognizes emissions in the 3.7-3.98 GHz band at this location will result in Electromagnetic Interference (EMI) as described in Airworthiness Directives (AD) 2021-23-12 and 2021-23-13. NAS services including airport and helicopter operations within a radius of 42 NM will be impacted by 5G RF emissions. Operational use of this frequency band is not objectionable provided the Wireless Providers obtain and adhere to the parameters established by the FAA 5G C band compatibility evaluation process.

Frequency Data for ASN 2025-AEA-12000-OE

LOW FREQUENCY	HIGH FREQUENCY	FREQUENCY UNIT	ERP	ERP UNIT
6	7	GHz	42	dBW
6	7	GHz	55	dBW
10	11.7	GHz	42	dBW
10	11.7	GHz	55	dBW
17.7	19.7	GHz	42	dBW
17.7	19.7	GHz	55	dBW
21.2	23.6	GHz	42	dBW
21.2	23.6	GHz	55	dBW
614	698	MHz	1000	W
614	698	MHz	2000	W
698	806	MHz	1000	W
806	824	MHz	500	W
806	901	MHz	500	W
824	849	MHz	500	W
851	866	MHz	500	W
869	894	MHz	500	W
896	901	MHz	500	W
901	902	MHz	7	W
929	932	MHz	3500	W
930	931	MHz	3500	W
931	932	MHz	3500	W
932	932.5	MHz	17	dBW
935	940	MHz	1000	W
940	941	MHz	3500	W
1670	1675	MHz	500	W
1710	1755	MHz	500	W
1850	1910	MHz	1640	W
1850	1990	MHz	1640	W
1930	1990	MHz	1640	W
1990	2025	MHz	500	W
2110	2200	MHz	500	W
2305	2360	MHz	2000	W
2305	2310	MHz	2000	W
2345	2360	MHz	2000	W
2496	2690	MHz	500	W
3700	3980	MHz	1640	W

Verified Map for ASN 2025-AEA-12000-OE

Ken Patterson
Airspace Consulting, Inc.
www.airspace-ken.com

Site ID: Whitehall 1164.015 December 1, 2025

To Whom It May Concern:

On December 1, 2025, I personally conducted an evaluation of a proposed telecommunications site for Arcola Towers. The study was to determine if the proposed structure would create any adverse effect on navigable airspace. The site is located near Smithsburg, Maryland at 39° 36' 51.69" North and 77° 38' 45.77" West (NAD 83). The site elevation is 605' above mean sea level (AMSL). The proposed structure height is 189' above ground level (AGL) or 794' AMSL. Part 77 of the Federal Air Regulations and Part 17 of the FCC Rules and Regulations were used as the primary reference for this evaluation.

The closest public use or DOD landing surface is Runway 02 at Hagerstown Regional Richard A. Henson Field. The distance to the runway is 6.58 nautical miles on a true bearing of 144.17° from the runway.

The proposed 189' AGL (794' AMSL) structure would not exceed any FAR Part 77 or FCC Part 17 notice requirement and, therefore, notice to the FAA is not required for this structure. If filed, the 189' AGL structure should be approved by the FAA.

Normally, structures that do not require notice to the FAA do not require marking and/or lighting. Private use landing facilities and AM broadcast stations are not a factor for this study.

For additional information or questions about this study, contact my office anytime.

Sincerely,

Ken Patterson

Ken Patterson

KP16118

150 Discovery Lake Dr, Fayetteville, GA 30215
(770) 461-0563 kpac0@bellsouth.net

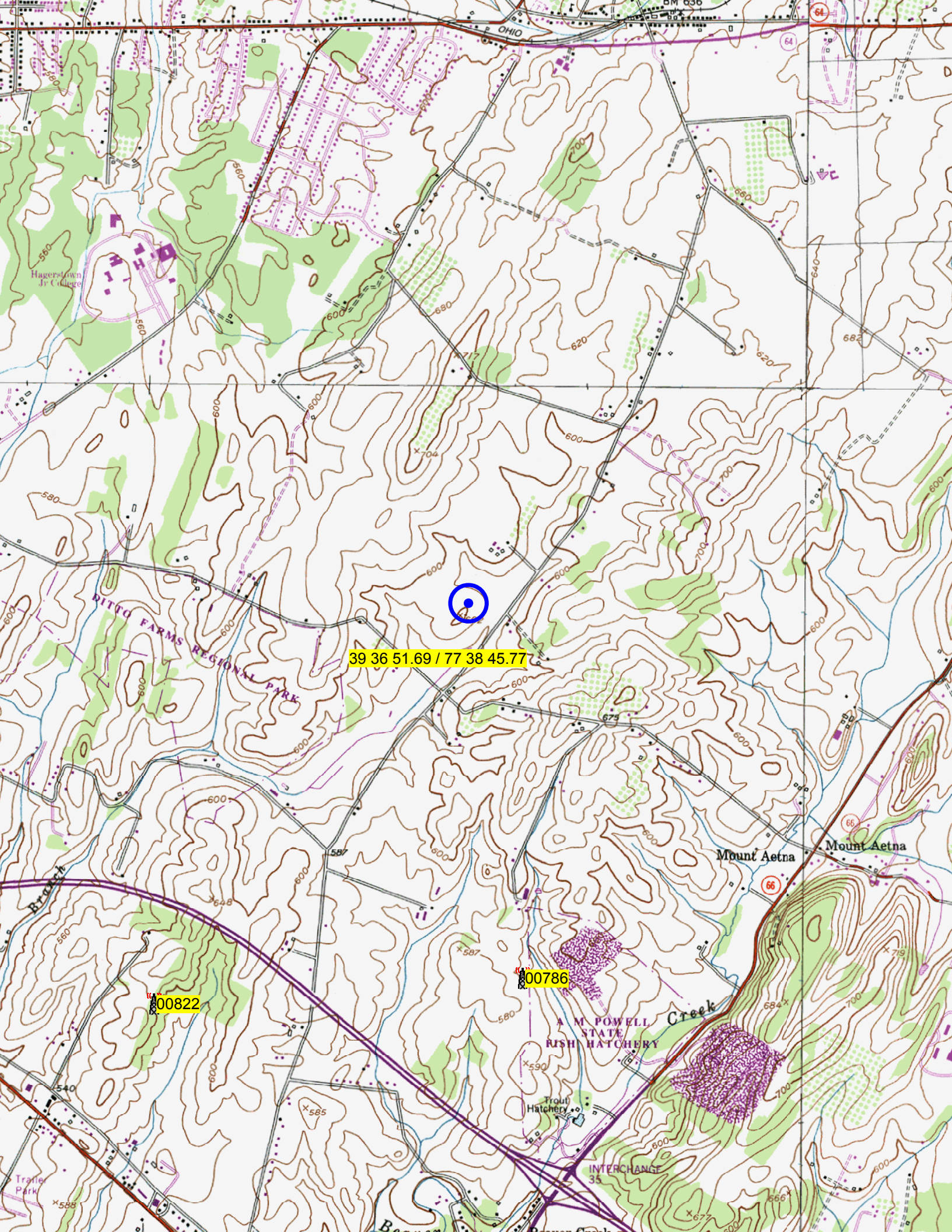
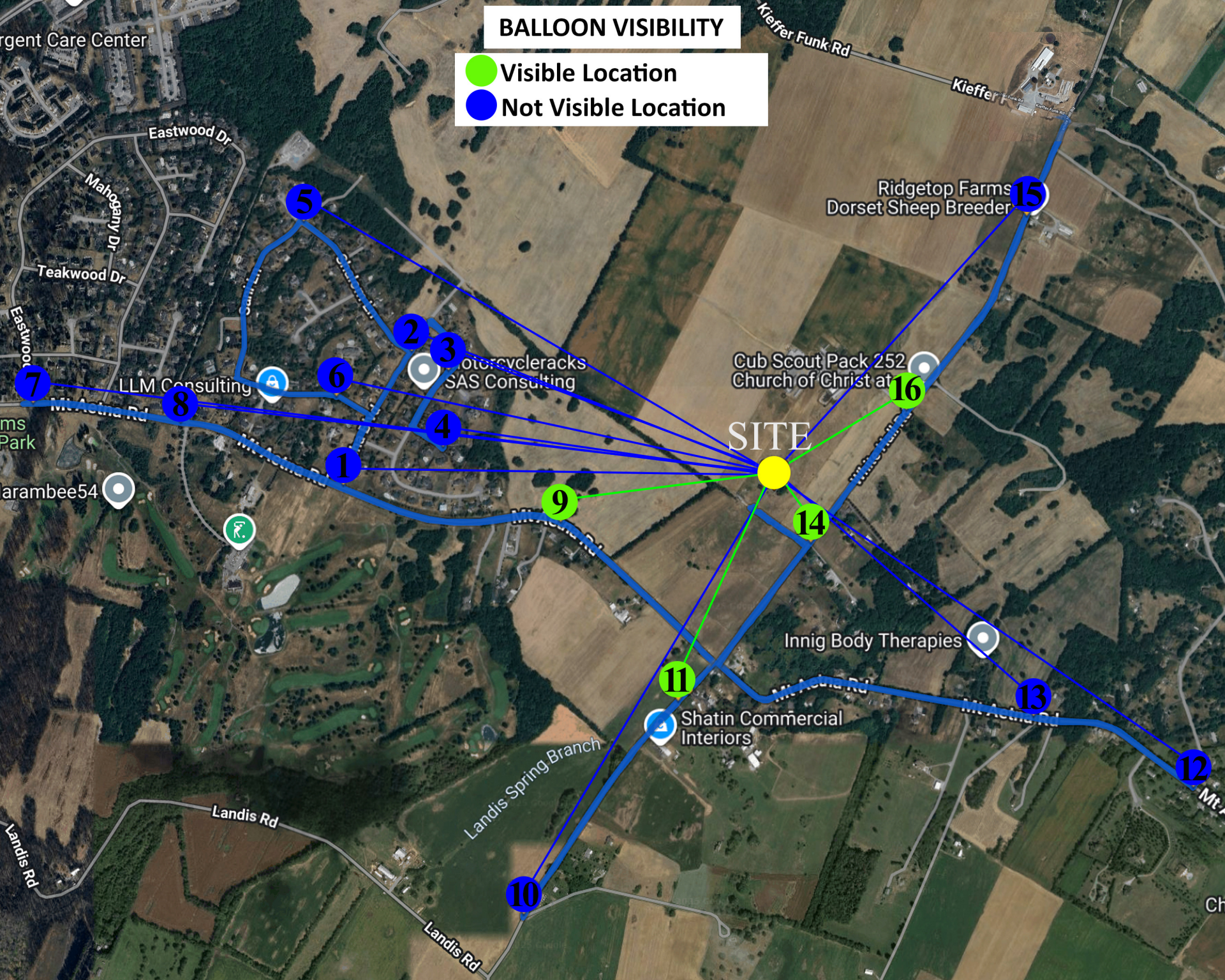

39 36 51.69 / 77 38 45.77
00822
00786
Mount Aetna
Mount Aetna
DITTO FARMS REGIONAL PARK
A M POWELL STATE FISH HATCHERY
Trout Hatchery
INTERCHANGE 35
Hagerstown Jr College
OHIO
Creek

Exhibit “7”


BALLOON VISIBILITY
Visible Location
Not Visible Location
SITE
1
2
3
4
5
6
7
8
9
10
11
12
13
14
15
16
Kieffer Funk Rd
Eastwood Dr
Mahogany Dr
Teakwood Dr
Ridgetop Farms
Dorset Sheep Breeder
Cub Scout Pack 252
Church of Christ at
LLM Consulting
SAS Consulting
Innig Body Therapies
Shatin Commercial
Interiors
Landis Spring Branch
Landis Rd
Mt Aetna Rd

WGS84 ±11ft 39°36'51.40"N, 77°39'33.68"W ft ±10ft 601 °,T ±18 NE75

24Oct25 08:39 Arcola Whitehall
20400–20508 Mt Aetna Rd, Hagerstown MD 21742, US 24-Oct-25 08:39:57

WGS84 ±20ft 39°37'3.12"N, 77°39'25.94"W ft ±10ft 621 °,T ±17 SE106

24Oct25 08:41 Arcola Whitehall
10941 Sasha Blvd, Hagerstown MD 21742, US ◷ 24-Oct-25 08:41:15

WGS84 ±7ft 39°37'1.37"N, 77°39'21.72"W ft ±10ft 633 °,T ±13 SE110

NOT VISIBLE

24Oct25 08:42 Arcola Whitehall
10964 Sassan Ln, Hagerstown MD 21742, US ◷ 24-Oct-25 08:42:37

WGS84 ±17ft 39°36'54.77"N, 77°39'22.36"W ft ±10ft 651 °,T ±89 S179

NOT VISIBLE

24Oct25 08:44 Arcola Whitehall
20509 Tehrani Ln, Hagerstown MD 21742, US ◷ 24-Oct-25 08:44:49

WGS84 ±7ft 39°37'14.42"N, 77°39'38.20"W ft ±10ft 603 °,T ±15 SE116

24Oct25 08:47 Arcola Whitehall
11205 Shalom Ln, Hagerstown MD 21742, US ◷ 24-Oct-25 08:47:34


WGS84
±7ft
39°36'59.17"N, 77°39'34.38"W
ft
±10ft
630
°,T
±13
SE106
NOT VISIBLE
24Oct25 08:49 Arcola Whitehall
20357 Ayoub Ln, Hagerstown MD 21742, US
24-Oct-25 08:49:37

WGS84 ±23ft 39°36'58.60"N, 77°40'8.78"W ft ±10ft 537 °,T ±11 E92

NOT VISIBLE

24Oct25 08:52 Arcola Whitehall
20100–20198 Mt Aetna Rd, Hagerstown MD 21742, US 24-Oct-25 08:52:46

WGS84 ±7ft 39°36'56.92"N, 77°39'52.01"W ft ±10ft 580 °,T ±43 S193

NOT VISIBLE

24Oct25 08:53 Arcola Whitehall
Hagerstown MD 21740, United States ◷ 24-Oct-25 08:53:44

PHOTO #9

WHITEHALL
PROPOSED MONOPOLE

WGS84 ±7ft 39°36'13.99"N, 77°39'13.28"W ft ±10ft 570 °,T ±14 NE33

NOT VISIBLE

24Oct25 08:58 Arcola Whitehall
10472–10552 White Hall Rd, Hagerstown MD 21740, US ◷ 24-Oct-25 08:58:06

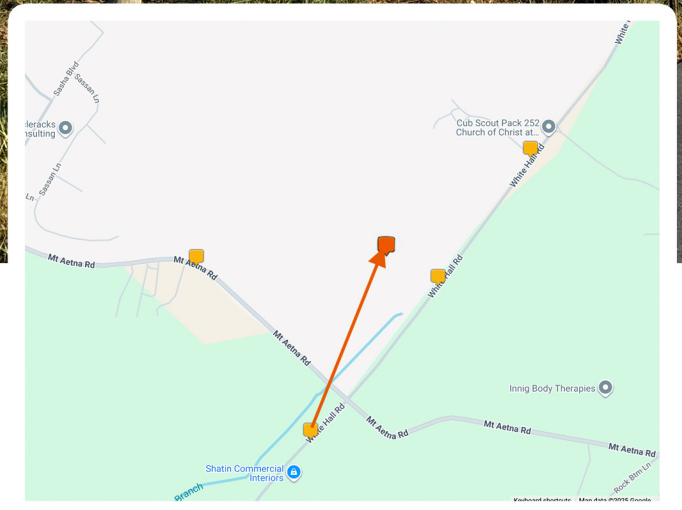


PHOTO #11

WHITEHALL
PROPOSED MONOPOLE

WGS84
±7ft
39°36'25.13"N, 77°37'57.58"W
ft
±10ft
623
°,T
±16
NW313

NOT VISIBLE

24Oct25 09:04 Arcola Whitehall
21205 Mt Aetna Rd, Hagerstown MD 21742, US ◷ 24-Oct-25 09:04:40

WGS84 ±28ft 39°36'31.31"N, 77°38'15.63"W ft ±10ft 680 °,T ±12 NW322

NOT VISIBLE

24Oct25 09:06 Arcola Whitehall
20970–21002 Mt Aetna Rd, Hagerstown MD 21783, US ◷ 24-Oct-25 09:06:25

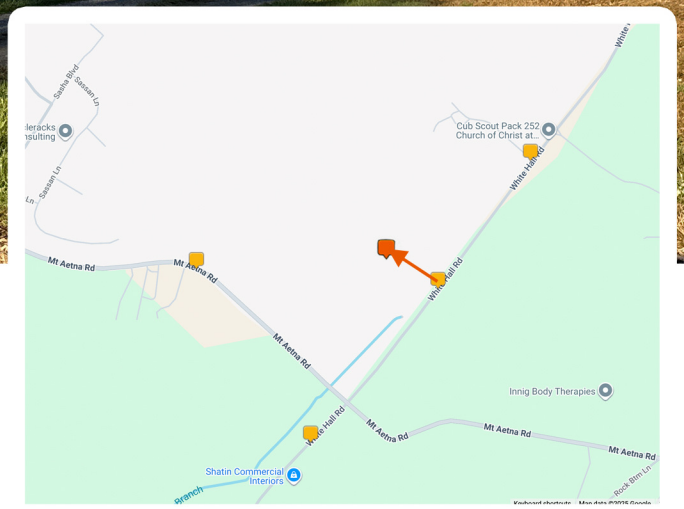


PHOTO #14

WHITEHALL

PROPOSED MONOPOLE

WGS84
±14ft
39°37'14.83"N, 77°38'16.43"W
ft
±10ft
606
°,T
±31
SW229

NOT VISIBLE

24Oct25 09:12 Arcola Whitehall
11034–11132 White Hall Rd, Smithsburg MD 21783, US ◷ 24-Oct-25 09:12:20

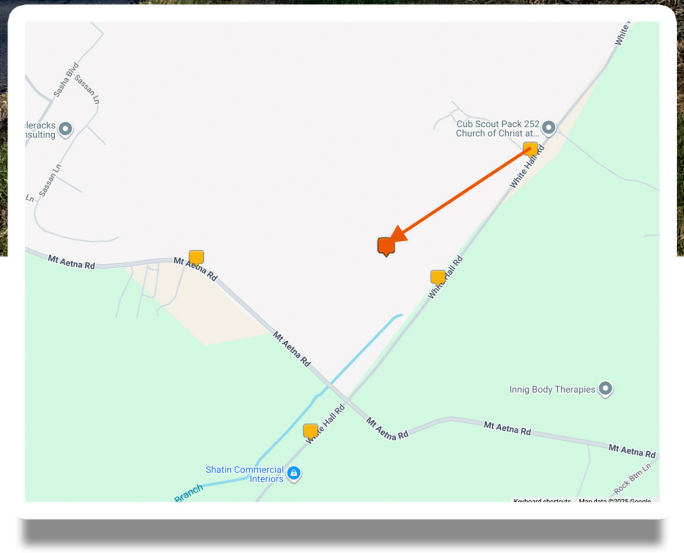


PHOTO #16

WHITEHALL
PROPOSED MONOPOLE

Exhibit “8”

December 2, 2025

Katie Rathvon
Zoning Coordinator
Washington County Planning & Zoning

80 West Baltimore Street,
Hagerstown, MD 21740

Re: Proposed Telecommunications Facility at 10944 White Hall Road, Smithsburg, MD 21783 by Milestone Towers for Verizon Wireless - Telecommunications Facility Application - Tower Removal Letter

Dear Ms. Rathvon,

Arcola Towers, its successors and assigns, provides this statement declaring itself, its successors and assigns of being responsible for compliance with Section 4.22.12 of the Washington County Zoning Ordinance, which requires the following:

1. A Commercial Communication tower that is out of service for a continuous six (6) month period will be deemed to have been abandoned. The Zoning Administrator may issue a Notice of Abandonment to the Owner of the tower that is deemed to be abandoned. The Owner shall have the right to respond in writing to the Notice of Abandonment setting forth the reasons for operation difficulty and providing a reasonable timeframe for correction action, within thirty (30) days from the date of the Notice. The Administrator shall withdraw the Notice of Abandonment and notify the Owner that the Notice has been withdrawn fi the Owner provides information that demonstrates the Tower has not been abandoned.
2. If the Tower is determined to be abandoned, the Owner of the Tower shall remove the Tower and all related equipment at the Owner's sole expensed within three (3) months of the Date of Notice of Abandonment. If the Owner fails to remove the Tower and related equipment, the Administrator may pursue legal action to have the Tower removed at the Owner's expense.

Please contact me should you have any questions.

Ryan Foltz

Ryan Foltz
Arcola Towers
Senior Development Director

Exhibit “9”

December 2, 2025

Katie Rathvon
Zoning Coordinator
80 West Baltimore Street,
Hagerstown, MD 21740

Re: Proposed 187-foot monopole-style wireless communication facility to be located at 10944 White Hall Road, Smithsburg, MD 21783 (Account ID # 18-013932) by Arcola Towers – Existing Inventory Letter

Dear Ms. Rathvon,

Arcola Towers' proposed wireless communication facility near located at 10944 White Hall Road, Smithsburg, MD 21783 will be designed and built in full compliance with Section 4.22(B)(1) of the Washington County Zoning Ordinance. Currently, Arcola Towers only owns and/or operates the following tower in the Washington County jurisdiction and within one (1) mile thereof:

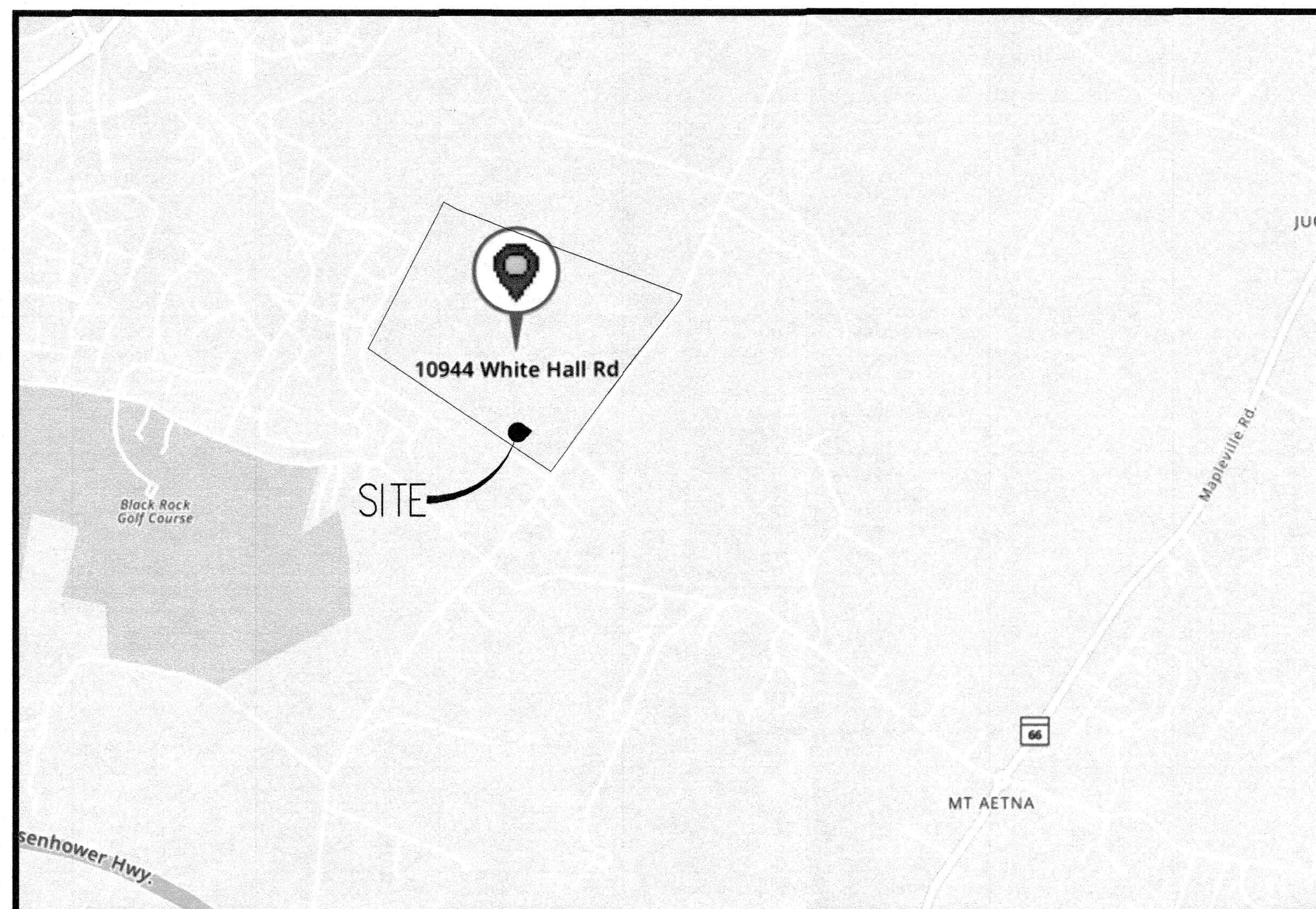
1.) **Paramount-** 140' Monopole – 19224 Longmeadow Road, Hagerstown, MD 21742: Account ID # 27-019196.

If you have further questions, please do not hesitate to contact me directly.

Ryan Foltz

Ryan Foltz
Arcola Towers
Senior Development Director

Exhibit “10”



VICINITY MAP

SCALE: 1" = 2,000'-0"

TRUE NORTH

ZONING DRAWING NOTES

1. SITE NAME: ARCOLA TOWERS SITE – WHITEHALL
2. THIS IS NOT A BOUNDARY SURVEY AND IS NOT TO BE USED FOR THE TRANSFER OF PROPERTY.
3. THE SUBJECT PARCEL INFORMATION;

   OWNER: MILLER, JAMES H. & ELAINE K.
   PREMISES ADDRESS: 10944 WHITE HALL ROAD, SMITHSBURG, MD 21783
   MAILING ADDRESS: 12290 SCOTT ROAD, WAYNESBORO, PA 17268

   JURISDICTION: WASHINGTON COUNTY

   ACCOUNT NO.: 18–013932

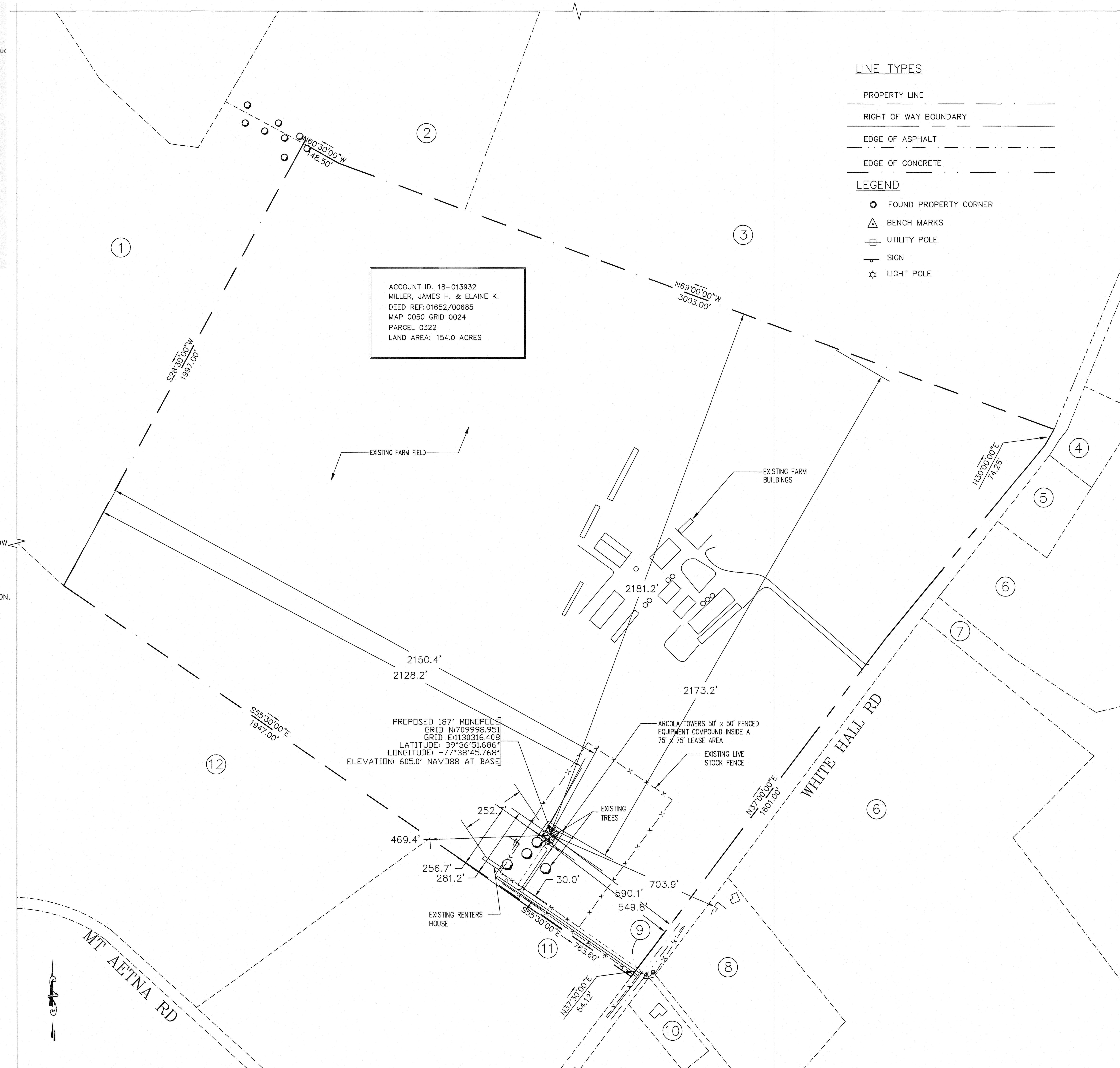
   PARCEL: 0322, DEED REF: 01652/00685

   MAP 0050 GRID 0024

   AREA: 154 ACRES
4. THE LOCATION OF THE PROPOSED MONOPOLE IS AS FOLLOWS; THE VALUES LISTED BELOW ARE WITHIN ±50' HORIZONTAL AND ±20' VERTICAL.
   LATITUDE: ± N 39° 36' 51.686"
   LONGITUDE: ± W 77° 38' 45.768"
   ELEVATION: 605.0' NAVD88 AT MONOPOLE BASE
5. THERE IS NO FLOODPLAIN ON THE PROPERTY THAT IS THE SUBJECT OF THIS APPLICATION. THE CURRENT FLOOD INSURANCE RATE MAP (FIRM) OF WASHINGTON COUNTY, MD, COMMUNITY PANEL NUMBER FOR THE PROPERTY THAT IS THE SUBJECT OF THIS APPLICATION IS 24043C0310D, EFFECTIVE AUGUST 15, 2017.
6. THIS SITE IS NOT LOCATED WITHIN THE APPALACHIAN TRAIL CARTER SPECIAL PLANNING AREA, THE ANTIETAM OVERLAY ZONING DISTRICT OR THE HISTORIC PRESERVATION ZONING DISTRICT
7. THE SITE IS LOCATED MORE THAN 389' FROM ANY EXISTING OFFSITE DWELLING, SCHOOL, CHURCH, OR INSTITUTION FOR HUMAN CARE.
8. THE CLOSEST EXISTING TOWER IS A 205' AMERICAN TOWERS SELF-SUPPORT TOWER, LOCATED APPROXIMATELY 1.75 MI SOUTH.
9. THERE ARE NO OVERHEAD TRANSMISSION LINES WITHIN TWO TIMES THE HEIGHT OF THE TOWER.
10. NO WETLANDS HAVE BEEN DEFINED AND ANY AREAS SHOWN AS MARSH, PONDS OR DITCHES ARE DONE SO FROM VISIBLE SURFACE FEATURES AND IN NO WAY CONSTITUTE A DEFINED WETLAND.
11. EXISTING HEAVY MATURE VEGETATION WILL BE UTILIZED IN LIEU OF NEW LANDSCAPING.

MONOPOLE SETBACKS	REQUIRED	PROPOSED
FRONT YARD (SOUTHEAST)	187'	590.1'
REAR YARD (NORTHWEST)	187'	2150.4'
SIDE YARD (SOUTHWEST)	187'	281.2'
SIDE YARD (NORTHEAST)	187'	2181.2'
CLOSEST OFF SITE RESIDENCE	187'	703.9'

COMPOUND SETBACKS	REQUIRED	PROPOSED
FRONT YARD (SOUTHEAST)	50'	549.8'
REAR YARD (NORTHWEST)	50'	21282'
SIDE YARD (SOUTHWEST)	50'	256.7'
SIDE YARD (NORTHEAST)	50'	2173.2'



SITE PLAN (1 / Z-1)

SCALE: 1"= 200'

TRUE NORTH

GRAPHIC SCALE

200 0 100 200 400 800

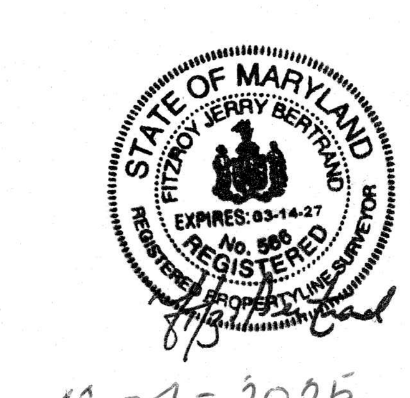
(IN FEET)
1 inch = 200 ft.

entrex communication services, inc
6100 EXECUTIVE BLVD.
SUITE 430
ROCKVILLE, MD 20852
PHONE: (202) 408-0960

arcola TOWERS
112 N. WASHINGTON ST
SUITE 201
MIDDLEBURG, VA 20117
PHONE: (571) 895 3990

ARCOLA TOWERS WHITEHALL
10944 WHITE HALL R
SMITHSBURG, MD 217

SEAL:



SUBMITTALS	
DATE	DESCRIPTION
09-09-25	ZONING REVIEW
09-24-25	ZONING REVIEW
10-08-25	ZONING REVIEW
10-14-25	ZONING

PROJECT NO: 1164.015
DESIGNER: R.S.
ENGINEER: C.S.

THESE DRAWINGS ARE FORMATTED TO BE FULL-SIZE AT 24"X36"
0 1/2 1
GRAPHIC SCALE IN INCHES

SHEET TITLE:

SITE PLAN

SHEET NUMBER:

Z-1

Exhibit “11”

01652 00685

CLERK OF THE CIRCUIT COURT
WASHINGTON COUNTY

TITLE NOT SEARCHED OR CERTIFIED

THIS DEED, Made this 1st day of May, 2001, by **Lewis M. Miller and Grace E. Miller**, hereinafter referred to as Grantors.

WITNESSETH: That for and in consideration of the sum of Four Hundred Fifty Thousand ($450,000.00) Dollars, cash in hand paid, and other good and valuable considerations, the receipt of which is hereby acknowledged, the said Grantors do hereby grant and convey unto **James H. Miller and Elaine K. Miller, husband and wife, as tenants by the entirety**, all that lot or parcel of land, together with the improvements thereon, if any, and all rights, ways, alleys, privileges and appurtenances thereunto belonging or in anywise appertaining, situate on the Western side of the White Hall Road, a short distance South of the Village of Chewsville, in Election District No. 18, in Washington County, Maryland, and being more particularly described as follows:

Being a part of a tract of land called "The Resurvey on the Old Fox Deceived:, and part of a tract of land called "The Resurvey on Carrs Queasy", lying contiguous and described by metes and bounds as follows: Beginning at a planted stone, said stone being the beginning of a deed from William J. Huyett to Jonas Q. Huffer, dated the 7th day of April, 1876, and recorded in Liber 74, folio 208, one of the Land Records of Washington County, Maryland, and running thence with the line of the aforesaid deed, South 28 ½ degrees West 121 perches to the end of a stone wall, thence South 55 ½ degrees East 118 perches to a stone at the end of the second line of a deed from Jacob Stotler, Sr. and wife to the said Jonas Q. Huffer dated the 20th day of May, A.D. 1881, and running thence with the third line of said deed, South 55 ½ degrees East 46.28 perches to the middle of the public road leading from Chewsville to Beaver Creek; thence along the middle of said road, and with the fourth line of said last mentioned deed, North 37 ½ degrees East 3.28 perches to the end of the fourth line of the first mentioned deed, thence along the middle of the said public road with the land of the first mentioned deed, North 37 degrees East 97 perches, North 39 ½ degrees East 60 perches, North 30 degrees East 4 ½ perches to a rock near the center of the aforesaid public road; thence leaving the said road, North 69 degrees West 182 perches, North 60 ½ degrees West 9 perches to the place of beginning; CONTAINING 154 acres and 8 perches of land, more or less.

Being all and the same real estate described and conveyed in a deed from Theophilus C. Herbst and Katherine M. Herbst, to Lewis M. Miller and Grace E. Miller, dated September 30, 1971 and recorded at Liber 531, folio 396, among the Land Records for Washington County, Maryland.

The above-described property is conveyed subject to all covenants, conditions, restrictions, easements and rights of way of record applicable thereto.

And the said Grantors do hereby covenant that they will warrant specially the property hereby conveyed, except as to the aforementioned covenants, conditions, restrictions, easements and rights of way of record, and that they will execute such other and further assurances as may be requisite and necessary.

WITNESS the hands and seals of the said Grantors.

Rest WAGE Rcpt # 40754
DJW TLC Blk # 1655
May 08, 2001 01:33 PM

IMP FD SURC $	5.00
RECORDING FEE	20.00
RECORDATION T	3,420.00
TR TAX STATE	2,250.00
TOTAL	5,695.00

01652 00686

CLERK OF THE CIRCUIT COURT
WASHINGTON COUNTY

WITNESS:

Diane L. Finney — Lewis M Miller (SEAL)
Lewis M. Miller

Diane L. Finney — Grace E. Miller (SEAL)
Grace E. Miller

STATE OF MARYLAND, COUNTY OF WASHINGTON, To-Wit:

I HEREBY CERTIFY, That on this 1st day of May, 2001, before me, the subscriber, a Notary Public in and for the State and County aforesaid, personally appeared Lewis M. Miller and Grace E. Miller, and each acknowledged the aforegoing deed to be their voluntary act and deed and that the actual consideration paid or to be paid for the aforegoing conveyance, including the amount of any mortgage or deed of trust assumed by the Grantees, is in the sum total of ($450,000.00) Dollars.

WITNESS my hand and Official Notarial Seal.

Diane L. Finney
Notary Public

My Commission Expires: 8/1/04

THIS IS TO CERTIFY, That the within instrument has been prepared by or under the supervision of an attorney admitted to practice law in the State of Maryland.

Kent N. Oliver, Attorney

~~AGRICULTURE TAX~~ $ Letter of Intent
ACREAGE 154 Agrel
CLERK Julie Grove

Mail to: Kent Oliver
P.O. Box 1269
Hagerstown MD 21741

RECEIVED FOR TRANSFER
State Department of
Assessments & Taxation
for Washington County
By Julie Grove Date 5/4/01

TAXES PAID May 8, 2001
TODD L. HERSHEY, TREASURER LJB

01652 00687

Signed Lof I

CLERK OF THE CIRCUIT COURT
WASHINGTON COUNTY

State of Maryland Land Instrument Intake Sheet

☐ Baltimore City ☑ County: Washington County

Information provided is for use of the Clerk's Office, State Department of Assessments and Taxation and County Finance Office only.

(Type or Print in Black Ink Only--All Copies Must Be Legible)

1 Type(s) of Instruments ([ ] Check Box if Addendum Intake Form is Attached)

1 ☐ Deed	2 ☑ Mortgage	☐ Other	☐ Other
☐ Deed of Trust	☐ Lease	☐ _____	☐ _____

2 Conveyance Type Check Box

☐ Improved Sale Arms-Length[1]	☐ Unimproved Sale Arms-Length[2]	☐ Multiple Accounts Arms-Length[3]	☐ Not an Arms-Length Sale[9]

3 Tax Exempt (if Applicable) Cite or Explain Authority

Recordation:
State Transfer:
County Transfer:

4 Consideration And Tax Calculations

Consideration Amount		Finance Office Use Only Transfer and Recordation Tax Consideration	
Purchase Price/Consideration	$ 450,000.00		
Any New Mortgage	$ 450,000.00	Transfer Tax Consideration	$
Balance of Existing Mortgage	$	X ( )% =	$
Other:	$	Less Exemption Amount -	$
		Total Transfer Tax =	$
Other:	$	Recordation Tax Consideration	$
		X ( ) per $500 =	$
Full Cash Value	$	TOTAL DUE	$

5 Fees

Amount of Fees	Doc. 1	Doc. 2	
Recording Charge	$ 20.00	$ 20.00	Agent:
Surcharge	$ 5.00	$ 5.00	Tax Bill:
State Recordation Tax	$ 3,420.00	$ 0	
State Transfer Tax	$ 2,250.00	$ 0	C.B. Credit
County Transfer Tax	$ 0	$ 0	
Other	$ 0	$ 0	Ag. Tax/Other:
Total Fees	$ 5,695.00	$ 25.00	

6 Description of Property

SDAT requires submission of all applicable information. A maximum of 40 characters will be indexed in accordance with the priority cited in Real Property Article Section 3-104(g)(3)(i).

District	Property Tax ID No.(1)	Grantor Liber/Folio	Map	Parcel No.	Var.LOG (5)
18	18-013932	531/396			

Subdivision Name	Lot(3a)	Block(3b)	Sect/AR(3c)	Plat Ref.	Sq.Ft./Acreage(4)

Location/Address of Property Being Conveyed (2)
154 acres, 10944 White Hall Road Smithsburg

Other Property Identifiers (if applicable) | Water Meter Account No.

Residential ☐ or Non-Residential ☐ | Fee Simple ☐ or Ground Rent ☐ Amount:

Partial Conveyance? [ ] Yes [☑] No | Description/Amt. of SqFt/Acreage Transferred:

If Partial Conveyance, List Improvements Conveyed:

7 Transferred From

Doc. 1 - Grantor(s) Name(s)	Doc. 2 - Grantor(s) Name(s)
Lewis M. Miller	James H. Miller
Grace E. Miller	Elaine K. Miller
Doc. 1 - Owner(s) of Record, if different from Grantor(s)	Doc. 2 - Owner(s) of Record, if Different from Grantor(s)

8 Transferred To

Doc. 1 - Grantee(s) Name(s)	Doc. 2 - Grantee(s) Name(s)
James H. Miller	
Elaine K. Miller	

New Owner's (Grantee) Mailing Address
10944 White Hall Road, Smithsburg MD 21783

9 Other Names to Be Indexed

Doc. 1 - Additional Names to be Indexed (Optional)	Doc. 2 - Additional Names to be Indexed (Optional)

10 Contact/Mail Information

Instrument Submitted By or Contact Person
Name: Kent N. Oliver
Firm: Miller, Oliver, Baker, Moylan & Stone
Address: 28 West Washington Street
Hagerstown, MD 21740
Phone: 301-790-1234

☐ Return to Contact Person
☐ Hold for Pickup
☑ Return Address Provided

11 IMPORTANT: BOTH THE ORIGINAL DEED AND A PHOTOCOPY MUST ACCOMPANY EACH TRANSFER

Assessment Information

☑ Yes ☐ No Will the property being conveyed be the grantee's principal residence?
☐ Yes ☑ No Does transfer include personal property? If yes, identify: _____
☐ Yes ☑ No Was property surveyed? If yes, attach copy of survey (If recorded, no copy required).

Assessment Use Only - Do Not Write Below This Line

☐ Terminal Verification ☐ Agricultural Verification ☐ Whole ☐ Part ☐ Tran. Process Verification

Transfer Number: Date Received: Deed Reference Assigned Property No.

Year	19	19	Geo.	Map	Sub	Block
Land			Zoning	Grid	Plat	Lot
Buildings			Use	Parcel	Section	Occ.Cd.
Total			Town Cd.	Ex. St.	Ex. Cd.	

Remarks:
Signed Lof I

Exhibit “12”

Washington County
MARYLAND

DIVISION OF
PERMITS AND INSPECTIONS

OWNER'S REPRESENTATIVE AFFIDAVIT

This is to certify that Jonathan L. Yates for Arcola Towers is authorized to make application for the work described as construction of a 187' monopole telecommunications facility, and located at 10944 White Hall Road, Smithsburg, MD 21783

The said work is authorized by James H. and Elaine K. Miller, the owner in fee, as required by the International Residential Code and the International Building Code.

This office does not enforce covenants or deed restrictions and it is the owner's, builder's and or developer's responsibility to make sure the covenants and/or deed restrictions are not being violated as a result of the issuance of this permit.

PROPERTY OWNER:

JAMES H MILLER
Name
12290 SCOTT RD
Address
WAYNESBORO PA 17268
City, State, Zip Code
James H Miller
Property Owner's Signature

AUTHORIZED REPRESENTATIVE:

Jonathan Yates
Name
105 Broad St. Third Floor
Address
Charleston, SC 29401
City, State, Zip Code
Jonathan L Yates
Authorized Representative's Signature

BOARD OF ZONING APPEALS

Annual Report Worksheet
Year 2025

The Board shall approve an Annual Report for the Reporting Year 2025 as required under Board of Appeals of Washington County Amended Rules of Procedure Meetings (2) a. In addition, this Annual Report shall be provided to the Washington County Commissioners.

Section I- Appeals for the Calendar Year of 2025.

Section II- Ordinance Sections Appealed.

Section III- Appeals Granted, Granted with Conditions, or Denied**.**

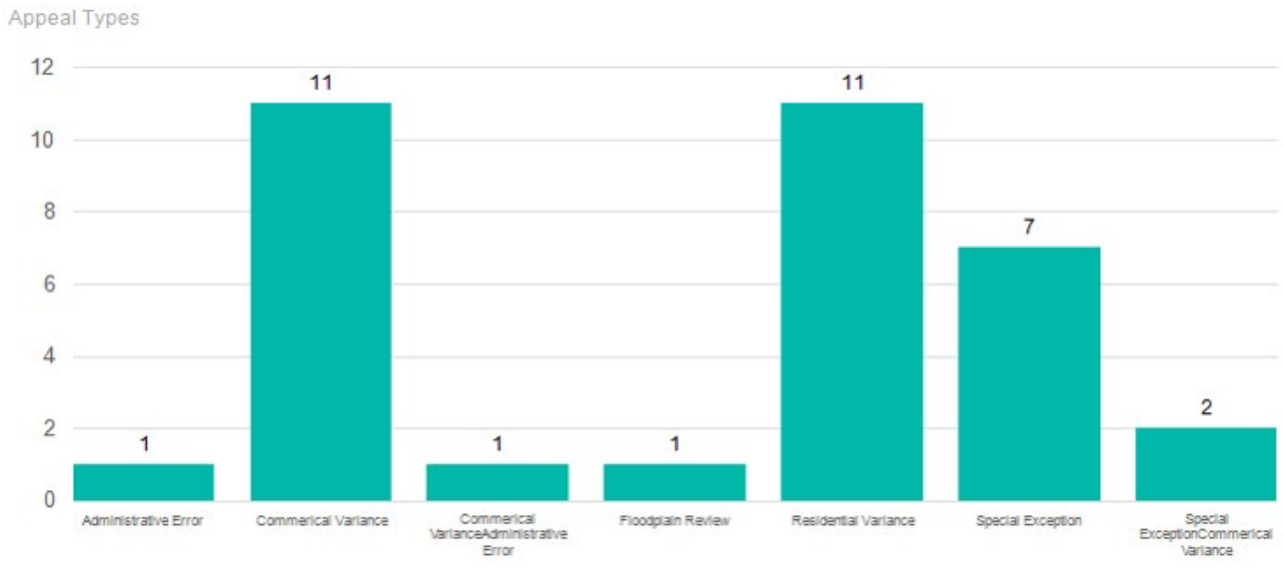
Section IV- Continuances, Postponements, Withdrawals, and Voids**.**

Section V- Appeals Filed to Circuit Court**.**

Section VI- Additional Items Completed by the Board.

Section I: Appeals for Calendar Year 2025

Total of 34 appeals were filed for 2025. Within each appeal there could be multiple request types i.e.: special exception with a variance(s).



Section II: Ordinance Sections Appealed

Ordinance Section	# of Appeals Filed
3.3 Land Use Table Special Exceptions	7
22.23 (e) Signage	5
9.5 (a) Residential Urban District Setback Requirements	4
22.12 Parking	4
22.94 (a) Animal Husbandry Setback Requirements	2
5A.5 Agricultural Rural Setback Requirements	2
8.5 (a) Residential Suburban District Setback Requirements	2
5D.3 Rural Village District Residential Uses Dimensional Requirements	2
Washington County Floodplain Ordinance	2
Other Sections of the Zoning Ordinance where only one appeal was filed for Section	7

Section III: Appeals Granted, Granted with Conditions, or Denied

(A) 28 Appeals were Granted by the Board with No Conditions.

(B) 4 Appeals were Denied by the Board.

Appeal	Appeal Type	Request
AP2025-007	Residential Variance	Variance from the required 8 ft. side yard setback to 1 ft. for proposed deck.
AP2025-008	Commercial Variance	Variance from the required 25 ft. setback from the road right-of-way to 22.34 ft. for installed freestanding sign.
AP2025-018	Residential Variance	Variance from the 100 ft. animal husbandry structure setback from all property lines to 7 ft. from the left property line, 40 ft. from the right property line, and 90 ft. from the rear property line.
AP2025-025	Administrative Error and Commercial Variance	Charging Admin. Error of the Zoning Administer stating the interpretation of Section 22.23(e) regarding “use on the Premises Signs” is incorrect for installation of a freestanding sign that would advertise the uses(s) or tenant(s) on adjacent parcels. Variance from the requirement for a property to have a lot frontage of at least 40 ft. in width to be reduced down to 25 ft. and a variance form the 25 ft. setback for the sign support structure from the road right-of-way to 10 ft. for proposed freestanding sign.

(C) 2 Appeals Granted with Conditions

Appeal	Appeal	Condition
AP2025-022	Variance is sought for the following ordinance sections: Section 5.4A to allow lowest floor to be constructed below Flood Elevation, Section 5.4.B.1 to allow floodproofing of a new non-residential building, Section 5.4.B.3.a to allow wet floodproofing in lieu of dry floodproofing for proposed public restroom for public park facility.	Condition: The structure will be elevated 1 ft.
AP2025-024	Special exception to establish a contractor equipment and storage yard.	Conditions: 1. Buffering to be installed and; 2. Downward/shielded lighting for property lines adjacent to residential properties.

(D) 6 Mixed Vote by the Board

Appeal	Vote Count	Appeal Summary
AP2025-004	3-2	Granted-Special exception for proposed 199 ft. monopole commercial communication tower and setbacks for tower and equipment.
AP2025-007	2-2	Denied-Variance from the required 8 ft. side yard setback to 1 ft. for proposed deck.
AP2025-008	3-2	Denied-Variance from the required 25 ft. setback from the road right-of-way to 22.34 ft. for installed freestanding sign.
AP2025-012	3-2	Granted-Variance from the 50 ft. side yard setback for proposed new diesel/gasoline fueling island with canopy.
AP2025-015	3-2	Granted-Variance from the requirement of the substantial improvement interpretation of the floodplain ordinance.
AP2025-025	4-1	Board Found No Error-Charging administrative error of Zoning Administer for interpretation on “use on the Premises Sings” and variance request for propose freestanding sign.

Section IV: Continuances, Postponements, Withdrawals, Reconsideration, and Voids

(A) 2 Appeals were Continued to Second Hearing

Appeal	Appeal Type
AP2025-010	Variance from sign structure and sign from the road right-of-way
AP2025-025	Charging administrative error of Zoning Administer for interpretation on "use on the Premises Sings" and variance request for propose freestanding sign.

(B) 1 Appeal Postponed to Different Hearing

Appeal	Who Made Request	Appeal Type
AP2025-032	Appellant	Variance from front and side yard setback for residential carport

(C) 1 Appeal Withdrawn

Appeal	Who Made Request	Appeal Type
AP2025-032	Appellant	Variance from front and side yard setback for residential carport

(D) No Appeals were Filed for Reconsideration

(E) No Appeals were Voided

Section V: Appeals Filed to Circuit Court

Appeal	Appeal Summary	Whom Filed to the Court	Status
AP2025-008	Variance from setback for installed freestanding sign	Appellant	Closed-Dismissal-Stipulation
AP2024-038	Residential Variance for Animal Husbandry Setback	Appellant	Closed-Dismissal Lack of Pros.
AP2024-020	Special Exception for Physicians' Office	Appellant	Closed-Dismissal by Appellant
AP2023-029	Special exception to establish a general retail/merchandise store	Appellant	Pending in Appellate Court

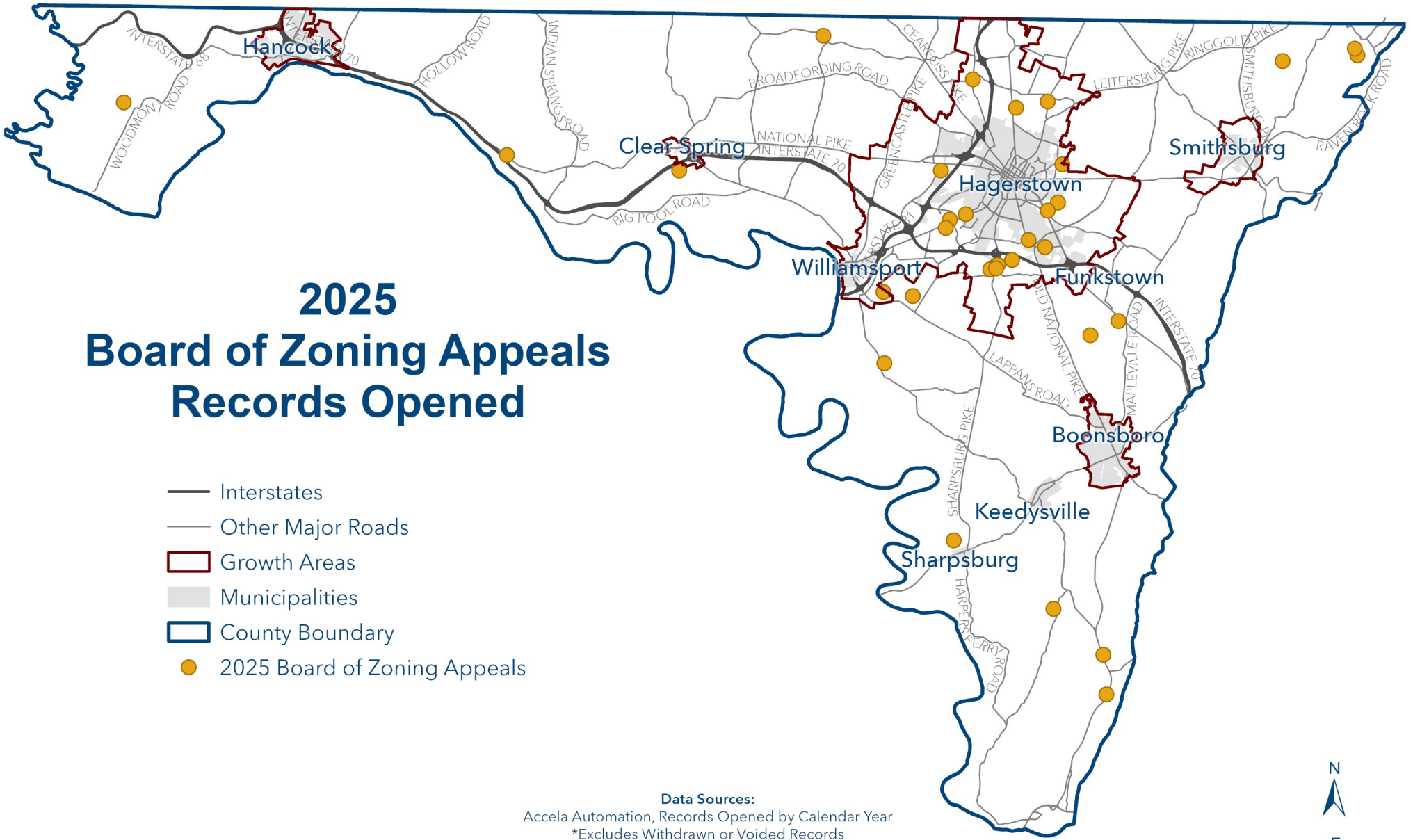
Section VI: Additional Items Completed by the Board

2024 Annual Report

All Opinions Received in a Timely Manner

Washington County, Maryland

Board of Zoning Appeals



2025 Board of Zoning Appeals Records Opened

- Interstates
- Other Major Roads
- Growth Areas
- Municipalities
- County Boundary
- 2025 Board of Zoning Appeals

Data Sources:
Accela Automation, Records Opened by Calendar Year
*Excludes Withdrawn or Voided Records

Prepared By:
Washington County Department of Planning and Zoning
Geographic Information Systems

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